

Functional Servicing and Stormwater Management Design Report – 81 Royal Road

City of Guelph, Ontario

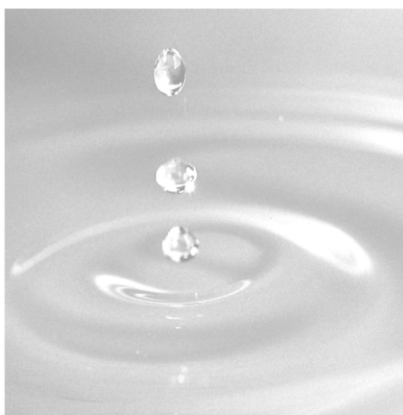
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December 13, 2024
Project No. 2406127



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Project Engineer

Table of Contents

Certification	iii
1. Introduction	1
1.1. Site Information	1
1.2. Site Grading	1
2. Site Services	2
2.1. Water Supply	2
2.2. Sanitary Service	2
2.3. Storm Service	2
3. Stormwater Management Design	3
3.1. Stormwater Management Criteria	4
3.2. Existing Conditions	4
3.3. Allowable Release Rates	4
3.4. Post-Development Condition Drainage Areas	5
3.5. Routing	7
4. Water Balance	9
5. Thermal Preventative Measures	10
6. Erosion and Sediment Control Plan	11
7. Maintenance Plan	12
8. Conclusions	13
List of Tables	
Table 2-1. Anticipated Water Demands from Site	2
Table 2-2. Anticipated Sanitary Design Flows to Royal Road	2
Table 3-1. City of Guelph – Chicago Storm Parameters	3
Table 3-2. City of Guelph – MIDUSS Horton Parameters	3
Table 3-3. Existing Condition Flow Rates	4
Table 3-4. Allowable Release Rates	4
Table 3-5. Quality Control Calculation	6
Table 3-6. Catchment 200 Rooftop Stage/Storage/Discharge Capacities	7
Table 3-7. Catchment 201 Underground Storage Stage/Storage/Discharge Capacities	7
Table 3-8. Summary of Post-Development Peak Flow Rates	8
Table 3-9. Comparison of Allowable Release Rates and Post-Development Flow Rates	8
Table 4-1. Summary of Recharge and Runoff Volumes	9

List of Figures

Figure No. 1 - Existing Conditions Drainage Area

Figure No. 2 - Post-Development Conditions Drainage Area

Appendices

Appendix A Sewer Design Sheets

Appendix B Stormwater Management Analysis

C.1. Existing Conditions Modelling

C.2. Post-Development Conditions Modelling

C.3. Oil and Grit Separator and Stormwater Tank Details

C.4. Water Balance Analysis

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Certification

PREPARED BY:

GEI Consultants Canada Ltd.



Patricia Wiebe, P.Eng.
Project Engineer

1. Introduction

This report documents the servicing design and the design of the stormwater management system for the proposed development at 81 Royal Road in the City of Guelph.

The Owner is required to have a Professional Engineer design a stormwater management system and have the said Engineer supervise and certify that the stormwater management system was installed in accordance with the approvals given under Section 41 of the Planning Act.

Van Harten completed the detailed site survey in May 29th, 2023. BJC Architects and Associates Inc. provided site layout (received December 12, 2024). The existing and proposed site details are shown on the Site Servicing and Grading Plans.

1.1. Site Information

The 1.83-hectare site is located at 81 Royal Road, in the City of Guelph. The site is bounded by existing industrial development to the north and east, Royal Road to the south and Woodlawn Road W to the west.

At this time, the intent of the Owner is to construct 30,000 sq. ft (2,787 m²), industrial building along with associated parking, driving, and landscaped areas on the currently vacant area.

Under existing conditions, runoff generated from the site is mostly conveyed via a swale on the adjacent property to the north to a catchbasin, ultimately discharging to the storm sewer on Woodlawn Road W. Following the development, stormwater runoff generated by the site will be attenuated on-site prior to discharging to the existing storm sewer on Woodlawn Road W.

1.2. Site Grading

As illustrated on the topographic survey completed for the proposed development, the site generally slopes towards the swale on the adjacent property in a southwest to northeast direction. The lowest elevation on the site of 341.00 (approximately) is in the northeast limits of the site boundary. The highest elevation on the site of 344.50 (approximately) is along the southeast limits of the site.

The grade and elevations of the site, along with the internal driving aisles, are controlled by the elevation of the Woodlawn Road W and Royal Road rights-of-way. The site will be graded to match the existing elevations along the property limits.

2. Site Services

2.1. Water Supply

Water supply is proposed to be provided via the extension of a 200mm diameter watermain from the existing 300mm diameter watermain on Royal Road.

There is one (1) existing nearby municipal fire hydrant on Royal Road alongside the proposed development.

Table 2-1. Anticipated Water Demands from Site

	Anticipated Water Demand
Proposed Average Water Demand	2.2 L/s
Proposed Peak Flow Rate (peaking factor of 4)	8.8 L/s

2.2. Sanitary Service

Sanitary service for the site will be provided by a 200mm diameter sanitary sewer extended southwest to the existing 300mm diameter sanitary sewer on Royal Road. The capacity of the existing 300mm diameter sanitary sewer on Royal Road is 0.081 m³/s (approximately), based on a grade of 0.64%.

The following tables summarize the flows discharging from the site to the municipal sanitary sewer. The sanitary sewer design sheet is found in Appendix A.

Table 2-2. Anticipated Sanitary Design Flows to Royal Road

	Anticipated Sanitary Design Flow
Proposed Average Sanitary Sewer Flow	0.0022 m ³ /s
Peak Sanitary Sewer Flow	0.0088 m ³ /s
Extraneous Flow (0.25 L/s/ha)	0.0005 m ³ /s
Total Sanitary Flow	0.0093 m ³ /s
Existing 300mm Diameter Sanitary Sewer Capacity	0.0810 m ³ /s

The peak sanitary sewer flow from the proposed development represents approximately 11.5% of the existing sanitary sewer capacity. Therefore, in our opinion the existing 300mm sanitary sewer on Royal Road has sufficient capacity to convey the anticipated design flows from the proposed development.

2.3. Storm Service

Storm services for the site will be provided via a 300mm diameter storm sewer discharging to the existing 600mm diameter storm sewer on Woodlawn Road West. Stormwater flows will be attenuated within the proposed underground stormwater management tank prior to discharge from the site.

Storm sewer design sheets are found in Appendix A.

3. Stormwater Management Design

The stormwater management design has been separated into the existing and approved site and the proposed development.

The City of Guelph Chicago Storm parameters used to model the design rainfall events for the site are summarized in the following Table 3-1. These parameters are consistent with the City of Guelph Stormwater Management Master Plan IDF Curves (dated August 2023). The Chicago Rainfall Distribution parameters and the total depth of rainfall for the analysis are as follows:

Table 3-1. City of Guelph – Chicago Storm Parameters

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
A	563.699	750.423	855.183	972.202	1054.539	1122.601
B	1.5	1.5	1.5	1.5	1.5	1.5
C	0.766	0.769	0.764	0.752	0.746	0.738
r	0.400	0.400	0.400	0.400	0.400	0.400
Duration (min.)	240	240	240	240	240	240
Total Depth (mm)	33.713	44.147	51.710	62.786	70.383	78.288

The Horton infiltration method was used in the MIDUSS model. The following parameters summarized in Table 3-2 were used according to the City of Guelph Standards:

Table 3-2. City of Guelph – MIDUSS Horton Parameters

	Impervious Areas	Pervious Areas
Maximum Infiltration (mm/hr)	0.0	75.0
Minimum Infiltration (mm/hr)	0.0	12.5
Lag Constant (hr)	0.0	0.25
Depression Storage (mm)	1.5	5.0

The hydrologic model MIDUSS was used to create runoff hydrographs and to route the flows through the storage structures.

3.1. Stormwater Management Criteria

Based on the City of Guelph Development Engineering Manual, the stormwater management criteria for the proposed development are as follows:

1. Maintain predevelopment recharge rate, volume and hydroperiods at post-development conditions
2. Provide a minimum of 5mm of volume control
3. Thermal preventive and mitigation measures for cool water habitat per Figure 4.1.
4. Enhanced level of water quality treatment
5. Control peak flow post to pre for all design events (2-100 year)
6. Control 90th percentile event or extended detention of the 4 hour, 25mm Chicago distribution rainfall event for 24 hours.

3.2. Existing Conditions

For analysis purposes, the site was modelled as one (1) drainage catchment under existing conditions. The existing condition drainage catchments are shown on Figure No. 1 and described below. The existing condition MIDUSS computer modeling is attached in Appendix C.

Catchment 100 (2.03-hectares, 0% Impervious) represents the site. Runoff generated from Catchment 100 is captured and conveyed by a swale on the adjacent property to the east, ultimately discharging to the existing catchbasin south of Woodlawn Road right-of-way.

In summary, the existing condition flow rates are as follows:

Table 3-3. Existing Condition Flow Rates

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Catchment 100 (uncontrolled)	0.030 m ³ /s	0.121 m ³ /s	0.226 m ³ /s	0.338 m ³ /s	0.414 m ³ /s	0.480 m ³ /s
Total	0.030 m³/s	0.121 m³/s	0.226 m³/s	0.338 m³/s	0.414 m³/s	0.480 m³/s

3.3. Allowable Release Rates

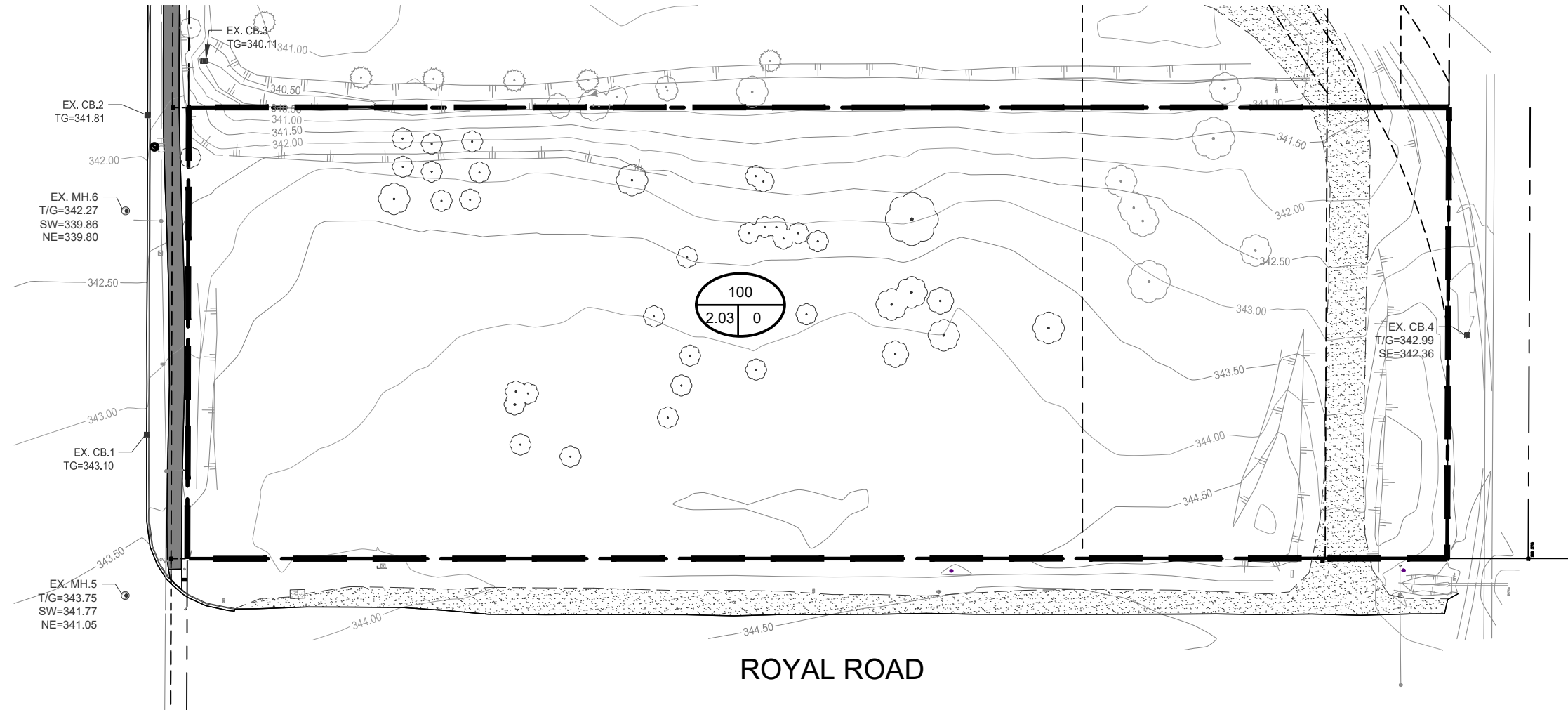
The post-development flows are to be attenuated to existing condition flow rates during all design storm events. Therefore, the allowable release rate under post-development conditions are as follows:

Table 3-4. Allowable Release Rates

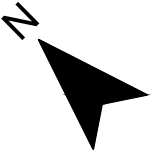
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Total Flows from Site to Storm Sewer on Woodlawn Road W	0.030 m ³ /s	0.121 m ³ /s	0.226 m ³ /s	0.338 m ³ /s	0.414 m ³ /s	0.480 m ³ /s

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WOODLAWN ROAD WEST

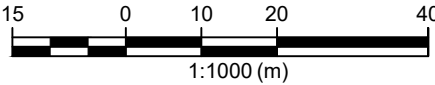


81 ROYAL ROAD
CITY OF GUELPH



LEGEND

- EX. STORM SEWER
- DRAINAGE AREA BOUNDARY
- CATCHMENT NUMBER
- % IMPERVIOUS
- CATCHMENT AREA IN HECTARES



EXISTING CONDITIONS
DRAINAGE AREA

Figure No. 1



3.4. Post-Development Condition Drainage Areas

For post-development analysis purposes, the proposed development was modelled as five (5) drainage catchments. The post-development drainage catchments are shown on Figure No. 2 and described below. The post-development MIDUSS computer modeling is attached in Appendix C.

Catchment 200 (0.28-hectares, 100% Impervious) represents the rooftop of the proposed building. The rooftop of the proposed building is flat and will be used to store and attenuate stormwater runoff. Runoff from Catchment 200 will be attenuated through the use of eight (8) roof drains with one (1) notch per drain and the average depth of water stored on the rooftop will not exceed the design criteria of 150 mm based on OBC. Please note that six (6) notches represent a fully open roof drain. Details of the roof drains have been appended. Runoff generated from Catchment 200 is directed to the proposed on-site storm sewers, and ultimately the storm sewers in the Woodlawn Road right-of-way.

Catchment 201 (0.75-hectares, 100% Impervious) represents the asphalt and landscape areas of the proposed development. Runoff generated from Catchment 201 will be captured by storm sewers and conveyed to the proposed underground stormwater management tank, discharging to the proposed on-site storm sewers, ultimately discharging to the storm sewers in the Woodlawn Road right-of-way.

Discharge from the underground stormwater management tank (22.4 m L x 12.8 m W x 1.32 m H tank using an EZStorm system) will be attenuated via a 90 mm diameter orifice plate prior to discharge to the existing storm sewer on Woodlawn Road W. The stormwater management tank will be constructed using an EZStorm system in order to maximize the storage volume and minimize the footprint of the tank. The stormwater management tank will provide a total of approximately 363.3 m³ of storage. Stormwater runoff exceeding the capacity of the stormwater management tank will discharge overland, ultimately discharging to the Woodlawn Road W right-of-way.

Catchment 202 (0.08-hectares, 0% Impervious) represents a small portion of the proposed development along the northwest limits of the site. Runoff generated from Catchment 202 sheetflows overland, uncontrolled, ultimately discharging to the Woodlawn Road West right-of-way.

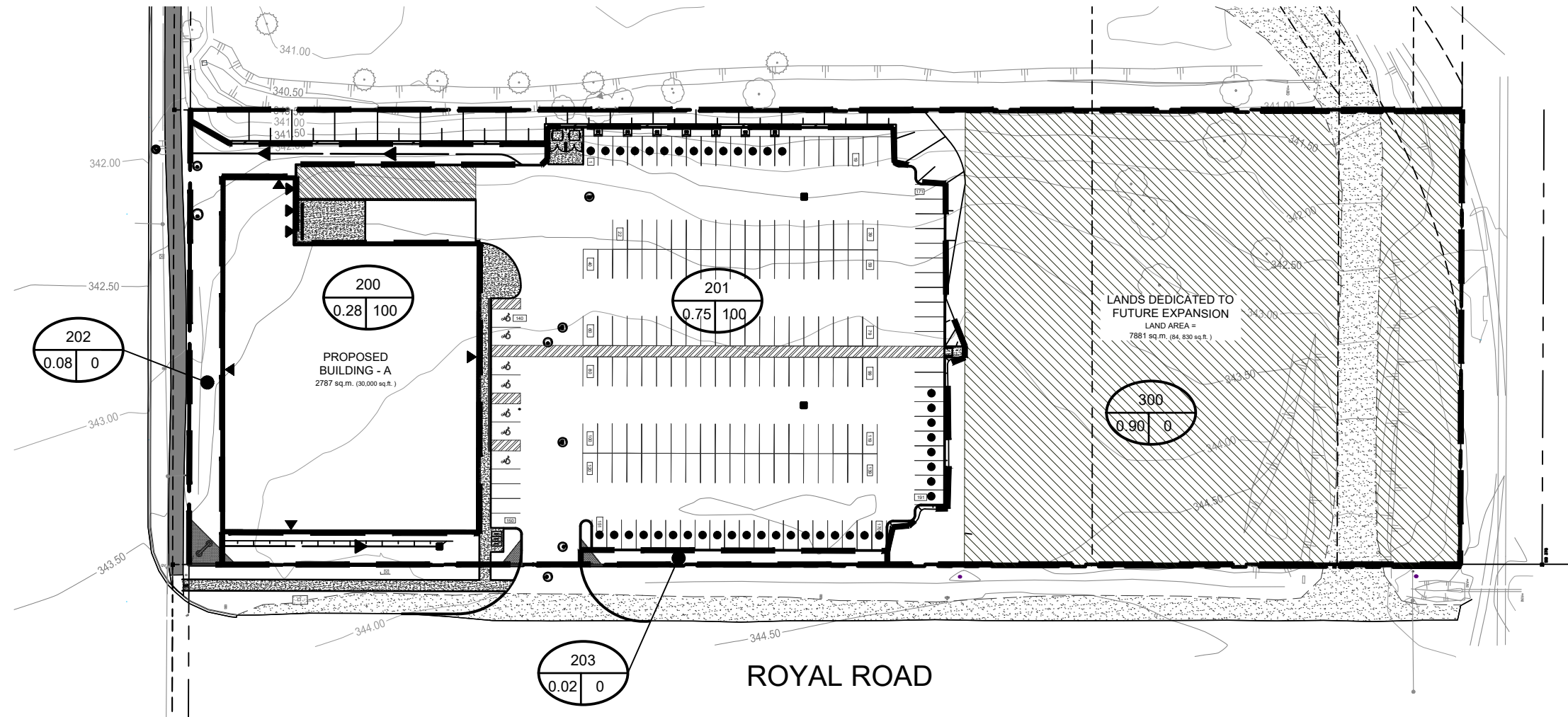
Catchment 203 (0.02-hectares, 0% Impervious) represents the landscaped area around the south limits of the proposed development. Runoff generated from Catchment 203 will sheetflow overland, discharging uncontrolled to the existing Royal Road right-of-way.

Catchment 300 (0.90-hectares, 0% Impervious) represents a portion of the site for future development along the east limits of the site. Runoff generated from Catchment 300 sheetflows overland north and east, uncontrolled, ultimately discharging to the catchbasin on the adjoining property and the storm sewers in the Woodlawn Road right-of-way.

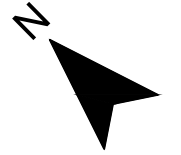
Quality Control for Catchments 200 and 201 will be provided by a treatment train of the pre-treatment row in the proposed underground EZStorm storage tank and further quality control for Catchments 200 and 201 will be provided by the proposed oil/grit separator unit (SDD3). We recognize that the City of Guelph Development Engineering Manual restricts the TSS removal from OGS Units to 50%. As such the TSS removal for the portion of the site being developed has been calculated as follows:

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WOODLAWN ROAD WEST

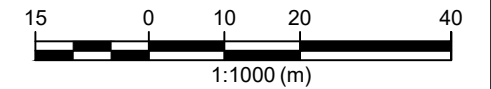


81 ROYAL ROAD
CITY OF GUELPH



LEGEND

- PROP. STORM SEWER
- DRAINAGE AREA BOUNDARY
- 201 CATCHMENT NUMBER
- 0.110 80 % IMPERVIOUS
- CATCHMENT AREA IN HECTARES



POST DEVELOPMENT
DRAINAGE AREA

Figure No. 2



2406127
AUGUST 2024
Scale: 1:1000 | NAD 1983 UTM Zone 17N

Table 3-5. Quality Control Calculation

Catchment	Area	Imp.	TSS Generated Annually	TSS Removal (%)	Annual TSS Removal
200	0.28 ha	100%	0 (rooftops considered clean runoff)	--	--
201	0.75 ha	100%	3.60 m ³	85% (70% from pretreatment row, then 50% from oil and grit separator)	3.06 m ³
202	0.08 ha	0%	0 (landscaped areas considered clean runoff)	--	--
203	0.02 ha	0%	0 (landscaped areas considered clean runoff)	--	--
300	0.90 ha	0%	0 (landscaped areas considered clean runoff)	--	--
Total	2.03 ha	52%	3.60 m³		3.06 m³

Therefore, the criteria of 80% TSS removal for the proposed development has been met. Details of the storage tank and the oil and grit separator are included in Appendix C.

3.5. Routing

The hydrologic model MIDUSS was used to create the 2-100-year design storm runoff hydrographs and to route the hydrographs. A copy of the modelling results of the post-development analysis is appended.

The results of the routing analysis are as follows:

Table 3-6. Catchment 200 Rooftop Stage/Storage/Discharge Capacities

	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Rooftop	0.000	0.0	0.000	---	---	---
2-Year	---	---	---	0.004	61.2	0.022
5-Year	---	---	---	0.006	80.8	0.029
10-Year	---	---	---	0.007	94.5	0.034
25-Year	---	---	---	0.008	114.0	0.041
50-Year	---	---	---	0.009	127.2	0.046
100-Year	---	---	---	0.010	140.8	0.051
Overflow	0.021	297.0	0.100	---	---	---

Table 3-7. Catchment 201 Underground Storage Stage/Storage/Discharge Capacities

	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Orifice Invert	0.000	0.0	340.420	---	---	---
2-Year	---	---	---	0.013	172.0	341.045
5-Year	---	---	---	0.016	234.7	341.273
10-Year	---	---	---	0.017	279.7	341.436
25-Year	---	---	---	0.019	345.6	341.675
Top of Storm Tank	0.020	363.3	341.740	---	---	---
CB T/G Overflow	0.029	366.7	343.210			
Weir	0.030	368.9	343.300	---	---	---
50-Year	---	---	---	0.032	368.9	343.301
100-Year	---	---	---	0.075	369.3	343.314
Overflow	0.506	372.7	343.450	---	---	---

The rooftop and underground storage attenuates the 2-year design storm event flows by approximately 4 hours. Extended detention of 24 hours is not feasible on this site given that infiltration is not a feasible option.

A summary of the post-development peak flow from the site for all design storm events are provided in the Table below.

Table 3-8. Summary of Post-Development Peak Flow Rates

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Catchment 200 and 201 (controlled)	0.013 m ³ /s	0.016 m ³ /s	0.017 m ³ /s	0.019 m ³ /s	0.032 m ³ /s	0.075 m ³ /s
Catchment 202 (uncontrolled)	0.004 m ³ /s	0.010 m ³ /s	0.016 m ³ /s	0.024 m ³ /s	0.030 m ³ /s	0.035 m ³ /s
Catchment 203 (uncontrolled)	0.001 m ³ /s	0.003 m ³ /s	0.005 m ³ /s	0.007 m ³ /s	0.009 m ³ /s	0.010 m ³ /s
Catchment 300 (uncontrolled)	0.013 m ³ /s	0.051 m ³ /s	0.090 m ³ /s	0.148 m ³ /s	0.182 m ³ /s	0.211 m ³ /s
Total	0.026 m³/s	0.067 m³/s	0.108 m³/s	0.170 m³/s	0.207 m³/s	0.239 m³/s

Table 3-9. Comparison of Allowable Release Rates and Post-Development Flow Rates

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Allowable Release Rate	0.030 m ³ /s	0.121 m ³ /s	0.226 m ³ /s	0.338 m ³ /s	0.414 m ³ /s	0.480 m ³ /s
Post-Development Flow Rate	0.026 m ³ /s	0.067 m ³ /s	0.108 m ³ /s	0.170 m ³ /s	0.207 m ³ /s	0.239 m ³ /s

Therefore, the post-development peak flow rates are attenuated to allowable release rates during all design storm events.

4. Water Balance

The precipitation and temperature values were taken from the City of Guelph Stormwater Management Master Plan (dated December 2022). The average annual precipitation is estimated to be 844.3 mm.

Therefore, based on the annual infiltration rates, the existing annual average recharge occurring within the 2.03-hectare catchment area is estimated to be 1,880.9 m³. Under post-development conditions, the annual average groundwater recharge is estimated to be 1,442.7 m³.

Under existing conditions, the annual average runoff from the area is estimated to be 4,388.7 m³. As a result of the proposed development the impervious area (rooftop and paved surfaces) of the site increases, the annual potential evapotranspiration for impervious surfaces decreases and the runoff from the site increases. The runoff from the site under post-development conditions is estimated to be 8,576.1m³ per year.

In summary, the estimated recharge and runoff volume for the site are as follows:

Table 4-1. Summary of Recharge and Runoff Volumes

	Existing Condition	Post-Development Condition	Percent Change
Total Estimated Recharge	1,880.9 m ³	1,442.7 m ³	-23%
Total Estimated Runoff	4,388.7 m ³	8,576.1 m ³	+95%

As noted above, the reduction in infiltration under post-development conditions is approximately 438.2m³ annually. Per the Ontario Sourcewater Information Atlas, the area of this site is in an Issue Contributing Area and not in a Significant Groundwater Recharge Area, and as such, the reduction in annual infiltration is not considered a significant change. Based on nearby sites, we anticipate the site will have tight existing subsurface soils and a shallow local groundwater table. As such, infiltration has not been included in the proposed design and the site cannot meet the 5mm volume control or maintain pre-development recharge rates. This assumption will be updated with the completion of the Geotechnical Investigation of the site. The water budget is summarized in Appendix C.

5. Thermal Preventative Measures

This site contributes to a storm sewer on the Woodlawn Road right-of-way. Figure 4.1 of the Guelph Stormwater Management Master Plan shows this storm sewer contributes to a watercourse with unknown temperature requirements. The proposed development uses underground stormwater attenuation for minor design storm events and therefore minimizes temperature increases.

6. Erosion and Sediment Control Plan

A silt fence is to be installed along the perimeter of the site. The silt fence serves to minimize the opportunity for sediment to leave the site. The on-site catch basins are to have silt sacs placed under the grate until all on-site construction has been completed.

Upon completion of the grading, any area not subject to active construction within 30 days will be topsoiled and hydroseeded as per OPSS. PROV 804.

Inspection and maintenance of all silt fencing will start after installation is complete. The silt fence will be inspected on a weekly basis during active construction or after a rainfall event of 13 mm or greater. Maintenance will be carried out, within 48 hours, on any part of the silt fence found to need repair.

Once construction and landscaping has been substantially completed, the silt fence will be removed, any accumulated sediment will be removed, and the landscaping will be completed.

7. Maintenance Plan

To ensure that the stormwater management system continues to function as designed and constructed, we recommend that the following inspections and maintenance activities be completed on an annual basis:

1. Is there any indication of a spill (i.e. frothy water, oily sheen on the water)? If yes, investigate, inform the appropriate agencies and complete the necessary clean-up and restoration.
2. Inspect all orifice plates, and flow control devices. Remove and dispose of any accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.).
3. Inspect all catch basins, and manholes. Remove and dispose of any accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.).
4. Inspect all swales and overflow locations. Remove and dispose of any accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.).
5. Inspect and maintain stormwater tank and oil and grit separator in line with manufacturer recommendations.

Please note that any structures identified during the annual inspection to be worn, missing or damaged are to be repaired or replaced within 48 hours.

8. Conclusions

In summary,

1. It is proposed that the site will be accessed by Royal Road and Woodlawn Road W, and that the grading of the development is controlled by these access routes.
2. Water supply for the proposed development will be provided via the extension of a 200mm diameter watermain from the existing 300mm diameter watermain on Royal Road.
3. Sanitary service for the development will be provided via one (1) connection to the 300mm diameter sanitary sewer on Royal Road.
4. Storm service for the development will be provided via one (1) storm sewer connection to the existing 600mm diameter storm sewer on Woodlawn Road W.
5. The post-development condition flow rates for the proposed development will be attenuated to allowable release rates during all design storm events.
6. Quality Control for Catchment 200 and 201 will be provided by the proposed pretreatment row in the underground EZStorm storage tank and further quality control will be provided by the proposed oil and grit separator (SDD3) for Catchments 200 and 201. Therefore, enhanced quality control is provided for the proposed development.
7. The reduction in infiltration under post-development conditions is approximately 438.2 m³ annually. Per the Ontario Sourcewater Information Atlas, the area of this site is in an Issue Contributing Area and not in a Significant Groundwater Recharge Area, and as such, the reduction in annual infiltration is not considered a significant change. Based on nearby sites, we anticipate the existing subsurface soils to be tight and have a shallow local groundwater table. Infiltration has therefore not been included in the proposed design and the site cannot meet the 5mm volume control or maintain pre-development recharge rates. This assumption will be updated with the completion of the Geotechnical Investigation.
8. The proposed development uses underground stormwater attenuation for minor design storm events and therefore minimizes temperature increases to generated runoff.
9. The Owner/Architect/Agent is to provide the Mechanical and Structural Engineers with copies of the Site Servicing and Stormwater Management Report and the Site Grading and Servicing Plan and the Notes and Details for their use in the design of the rooftop stormwater management measures.
10. A qualified Structural Engineer must review and verify that the roof of the proposed Operations and Maintenance building and proposed building addition have the structural capacity to carry the loading of the stored rainfall and all other loading in accordance with OBC 2017.
11. To ensure that the average depth of water storage on the rooftop does not exceed the average design depth criteria of 100mm, with a maximum storage depth of 150mm at the roof drains, we recommend that a qualified Mechanical Engineer review and verify the requirement for overflow structures to be included in the design.
12. Prior to construction, a silt fence will be installed along the property boundary in all locations where runoff will discharge from the site to adjacent lands. This will minimize the transport of sediment off-site during the construction period.

Appendix A Sewer Design Sheets

SANITARY SEWER DESIGN

City of Guelph

Q(i) = Cum. Area (ha) * Infiltration Rate / 1000
Infiltration Rate: 0.25 L/s/ha

D = Diameter (mm)

S = Slope (%)

n = 0.013 (PVC & Concrete), 01016 (Vitrified Clay)

Minimum Full Velocity = 0.80 m/s

[illegible]

DESIGNED BY: P.W.

5-Year Design Storm
City of Guelph

A = 750.423
B = 1.5
C = 0.769

The Cumulative A x C at BLDG to CBMH.4 has been reduced to represent the 100-year flow rate attenuated from rooftop controls (0.010 m³/s).
The Cumulative A x C at Tank to MH.11 has been reduced to represent the 100-year flow rate attenuated from storm tank and orifice through the storm sewer (0.030 m³/s)

Appendix B Stormwater Management Analysis

C.1. Existing Conditions Modelling

C.2. Post-Development Conditions Modelling

C.3. Oil and Grit Separator and Stormwater Tank Details

C.4. Water Balance Analysis

C.1. Existing Conditions Modelling


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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\
"          Working\ROHNBAD\2406127 - 422111 352 Woodlawn Rd Build\Design
Phase\Design Data\Modelling Files\Updated_Nov"
"          Output filename:                      _Ex_2yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                11/19/2024 at 3:39:58 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          563.699 Coefficient A"
"          1.500  Constant B"
"          0.766  Exponent C"
"          0.400  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          129.248  mm/hr"
"          Total depth                33.713  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 100"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          100 Catchment 100"
"          0.000 % Impervious"
"          2.030 Total Area"
"          85.000 Flow length"
"          4.000 Overland Slope"
"          2.030 Pervious Area"
"          85.000 Pervious length"
"          4.000 Pervious slope"
"          0.000 Impervious Area"
"          85.000 Impervious length"
"          4.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          12.500 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"

```

```

"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.001  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.030      0.000      0.000      0.000 c.m/sec"
"      Catchment 100      Pervious      Impervious      Total Area  "
"      Surface Area      2.030      0.000      2.030      hectare"
"      Time of concentration 29.161      3.038      29.161      minutes"
"      Time to Centroid      121.897      118.112      121.897      minutes"
"      Rainfall depth      33.713      33.713      33.713      mm"
"      Rainfall volume      684.37      0.00      684.37      c.m"
"      Rainfall losses      31.150      1.935      31.150      mm"
"      Runoff depth      2.562      31.777      2.562      mm"
"      Runoff volume      52.02      0.00      52.02      c.m"
"      Runoff coefficient      0.076      0.000      0.076      "
"      Maximum flow      0.030      0.000      0.030      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.030      0.030      0.000      0.000"
" 38      START/RE-START TOTALS 100"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      2.030      hectare"
"      Total Impervious area      0.000      hectare"
"      Total % impervious      0.000"
" 19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\
"          Working\ROHNBRAID\2406127 - 422111 352 Woodlawn Rd Build\Design
Phase\Design Data\Modelling Files\Updated_Nov"
"          Output filename:                      _Ex_5yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                11/19/2024 at 3:45:39 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          180.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          750.423  Coefficient A"
"          1.500  Constant B"
"          0.769  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          171.091  mm/hr"
"          Total depth          44.147  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 100"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          100  Catchment 100"
"          0.000  % Impervious"
"          2.030  Total Area"
"          85.000  Flow length"
"          4.000  Overland Slope"
"          2.030  Pervious Area"
"          85.000  Pervious length"
"          4.000  Pervious slope"
"          0.000  Impervious Area"
"          85.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"

```

```

"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.001  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.121      0.000      0.000      0.000 c.m/sec"
"      Catchment 100      Pervious      Impervious      Total Area  "
"      Surface Area      2.030      0.000      2.030      hectare"
"      Time of concentration 19.091      2.715      19.091      minutes"
"      Time to Centroid      116.903      116.631      116.903      minutes"
"      Rainfall depth      44.147      44.147      44.147      mm"
"      Rainfall volume      896.19      0.00      896.19      c.m"
"      Rainfall losses      35.988      1.940      35.988      mm"
"      Runoff depth      8.159      42.207      8.160      mm"
"      Runoff volume      165.64      0.00      165.64      c.m"
"      Runoff coefficient      0.185      0.000      0.185      "
"      Maximum flow      0.121      0.000      0.121      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.121      0.121      0.000      0.000"
" 38      START/RE-START TOTALS 100"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      2.030      hectare"
"      Total Impervious area      0.000      hectare"
"      Total % impervious      0.000"
" 19      EXIT"

```

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\
"          Working\ROHNBRAD\2406127 - 422111 352 Woodlawn Rd Build\Design
Phase\Design Data\Modelling Files\Updated_Nov"
"          Output filename:                      _Ex_10yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                11/19/2024 at 3:47:23 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          855.183  Coefficient A"
"          1.500  Constant B"
"          0.764  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          196.822  mm/hr"
"          Total depth          51.710  mm"
"          6  010hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 100"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          100  Catchment 100"
"          0.000  % Impervious"
"          2.030  Total Area"
"          85.000  Flow length"
"          4.000  Overland Slope"
"          2.030  Pervious Area"
"          85.000  Pervious length"
"          4.000  Pervious slope"
"          0.000  Impervious Area"
"          85.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.226	0.000	0.000	0.000 c.m/sec"	
"		Catchment 100	Pervious	Impervious	Total Area	"
"		Surface Area	2.030	0.000	2.030	hectare"
"		Time of concentration	16.384	2.567	16.384	minutes"
"		Time to Centroid	115.994	116.117	115.994	minutes"
"		Rainfall depth	51.710	51.710	51.710	mm"
"		Rainfall volume	1049.70	0.00	1049.70	c.m"
"		Rainfall losses	39.164	2.012	39.164	mm"
"		Runoff depth	12.545	49.697	12.545	mm"
"		Runoff volume	254.67	0.00	254.67	c.m"
"		Runoff coefficient	0.243	0.000	0.243	"
"		Maximum flow	0.226	0.000	0.226	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.226	0.226	0.000	0.000"	
" 38		START/RE-START TOTALS 100"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		2.030	hectare"	
"		Total Impervious area		0.000	hectare"	
"		Total % impervious		0.000"		
" 19		EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\
"          Working\ROHNBRAD\2406127 - 422111 352 Woodlawn Rd Build\Design
Phase\Design Data\Modelling Files\Updated_Nov"
"          Output filename:                      _Ex_25yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                11/19/2024 at 3:49:56 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          972.202  Coefficient A"
"          1.500  Constant B"
"          0.752  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                228.875    mm/hr"
"          Total depth                    62.786    mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 100"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          100  Catchment 100"
"          0.000  % Impervious"
"          2.030  Total Area"
"          85.000  Flow length"
"          4.000  Overland Slope"
"          2.030  Pervious Area"
"          85.000  Pervious length"
"          4.000  Pervious slope"
"          0.000  Impervious Area"
"          85.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.338	0.000	0.000	0.000 c.m/sec"	
"		Catchment 100	Pervious	Impervious	Total Area	"
"		Surface Area	2.030	0.000	2.030	hectare"
"		Time of concentration	14.247	2.417	14.247	minutes"
"		Time to Centroid	116.241	115.809	116.241	minutes"
"		Rainfall depth	62.786	62.786	62.786	mm"
"		Rainfall volume	1274.55	0.00	1274.56	c.m"
"		Rainfall losses	43.432	2.202	43.432	mm"
"		Runoff depth	19.354	60.584	19.354	mm"
"		Runoff volume	392.88	0.00	392.88	c.m"
"		Runoff coefficient	0.308	0.000	0.308	"
"		Maximum flow	0.338	0.000	0.338	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.338	0.338	0.000	0.000"	
" 38		START/RE-START TOTALS 100"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		2.030	hectare"	
"		Total Impervious area		0.000	hectare"	
"		Total % impervious		0.000"		
" 19		EXIT"				


```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\
"          Working\ROHNBRAD\2406127 - 422111 352 Woodlawn Rd Build\Design
Phase\Design Data\Modelling Files\Updated_Nov"
"          Output filename:                      _Ex_50yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                11/19/2024 at 3:50:58 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          1054.539  Coefficient A"
"          1.500  Constant B"
"          0.746  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                251.085  mm/hr"
"          Total depth                    70.383  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 100"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          100  Catchment 100"
"          0.000  % Impervious"
"          2.030  Total Area"
"          85.000  Flow length"
"          4.000  Overland Slope"
"          2.030  Pervious Area"
"          85.000  Pervious length"
"          4.000  Pervious slope"
"          0.000  Impervious Area"
"          85.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.414	0.000	0.000	0.000 c.m/sec"	
"		Catchment 100	Pervious	Impervious	Total Area	"
"		Surface Area	2.030	0.000	2.030	hectare"
"		Time of concentration	13.245	2.329	13.245	minutes"
"		Time to Centroid	116.774	115.601	116.774	minutes"
"		Rainfall depth	70.383	70.383	70.383	mm"
"		Rainfall volume	1428.77	0.00	1428.77	c.m"
"		Rainfall losses	46.156	2.395	46.156	mm"
"		Runoff depth	24.226	67.987	24.226	mm"
"		Runoff volume	491.80	0.00	491.80	c.m"
"		Runoff coefficient	0.344	0.000	0.344	"
"		Maximum flow	0.414	0.000	0.414	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.414	0.414	0.000	0.000"	
" 38		START/RE-START TOTALS 100"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		2.030	hectare"	
"		Total Impervious area		0.000	hectare"	
"		Total % impervious		0.000"		
" 19		EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\
"          Working\ROHNBRAD\2406127 - 422111 352 Woodlawn Rd Build\Design
Phase\Design Data\Modelling Files\Updated_Nov"
"          Output filename:                      _Ex_100yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                11/19/2024 at 3:52:12 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          1122.601  Coefficient A"
"          1.500  Constant B"
"          0.738  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                271.357  mm/hr"
"          Total depth                    78.288  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 100"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          100  Catchment 100"
"          0.000  % Impervious"
"          2.030  Total Area"
"          85.000  Flow length"
"          4.000  Overland Slope"
"          2.030  Pervious Area"
"          85.000  Pervious length"
"          4.000  Pervious slope"
"          0.000  Impervious Area"
"          85.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.480	0.000	0.000	0.000 c.m/sec"	
"		Catchment 100	Pervious	Impervious	Total Area	"
"		Surface Area	2.030	0.000	2.030	hectare"
"		Time of concentration	12.617	2.258	12.617	minutes"
"		Time to Centroid	117.994	115.464	117.994	minutes"
"		Rainfall depth	78.288	78.288	78.288	mm"
"		Rainfall volume	1589.24	0.00	1589.24	c.m"
"		Rainfall losses	48.574	2.483	48.574	mm"
"		Runoff depth	29.714	75.804	29.714	mm"
"		Runoff volume	603.19	0.00	603.19	c.m"
"		Runoff coefficient	0.380	0.000	0.380	"
"		Maximum flow	0.480	0.000	0.480	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.480	0.480	0.000	0.000"	
" 38		START/RE-START TOTALS 100"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		2.030	hectare"	
"		Total Impervious area		0.000	hectare"	
"		Total % impervious		0.000"		
" 19		EXIT"				

C.2. Post-Development Conditions Modelling

**81 Royal Road
City of Guelph
Our File: 2406127
November 29, 2024**

Catchment 200: Proposed Rooftop Storage

Design Discharge Rate =	1.50 l/min/mm/weir	2.50E-05 m ³ /s/mm/weir
Max. Average Storage Depth =	100 mm	
Design Discharge =	150.0 l/min/weir	0.0025 m ³ /s/weir
No. of Drains =	8	
No. Weirs/Drain =	1	
Allowable Release Rate =	1200.0 l/min	0.020 m ³ /s
Rooftop Area =	2,787 m ²	(flat rooftop area that is available for storage)

Therefore: 464.5 sq m/Roof Drain or 5000 sq ft/Roof Drain as per OBC

STAGE-STORAGE-DISCHARGE TABLE

Stage	Incremental Storage	Discharge
(m)	(m ³)	(m ³ /s)
0.000	0.00	0.000
0.025	69.68	0.005
0.050	139.35	0.010
0.075	209.03	0.015
0.100	278.70	0.020

81 Royal Road
City of Guelph
Our File: 2406127
November 29, 2024

Catchment 201: Underground Stormwater Tank (EZStorm System)

ELEV	INC. DEPTH	TANK SURFACE AREA	PONDING SURFACE AREA	INCR. VOL	ACCUM STORAGE VOL	
(m)	(m)	(m ²)	(m ²)	(m ³)	(m ³)	
340.42	0.00	286.72	0.00	0.00	0.00	Bottom of Storm Tank
340.72	0.30	286.72	0.00	82.58	82.58	
341.02	0.60	286.72	0.00	82.58	165.15	
341.32	0.90	286.72	0.00	82.58	247.73	
341.62	1.20	286.72	0.00	82.58	330.30	
341.74	1.32	286.72	0.00	33.03	363.33	Top of Storm Tank
342.04	1.62	0.00	2.26	0.68	364.01	
342.34	1.92	0.00	2.26	0.68	364.69	
342.64	2.22	0.00	2.26	0.68	365.37	
343.21	2.79	0.00	2.26	1.29	366.65	CB T/G
343.30	2.88	0.00	25.00	2.25	368.90	Weir
343.45	3.03	0.00	25.00	3.75	372.65	Overflow

Tank Parameters

L= 22.4 m
W= 12.8 m
H= 1320 mm

ORIFICE

invert= 340.420
Q = 0.031 m³/s
Cd = 0.63
H = 2.98 m
2g = 19.62
A = 0.006 m²
D = 0.090 m
D/2 = 0.045 m

OVERFLOW WEIR

Q = 0.475 cu m/s
d1 = 0.240 m
h = 0.090 m
H = 0.150 m
2g = 19.620
L = 5.000 m

Stage/Storage/Discharge Table

ELEV (m)	STAGE (m)	STORAGE (m ³)	ORIFICE DISCHARGE (m ³ /s)	OVERFLOW WEIR (m ³ /s)	TOTAL DISCHARGE (m ³ /s)	
340.42	0.00	0.00	0.000	0.000	0.000	Bottom of Storm Tank
340.72	0.30	82.58	0.009	0.000	0.009	
341.02	0.60	165.15	0.013	0.000	0.013	
341.32	0.90	247.73	0.016	0.000	0.016	
341.62	1.20	330.30	0.019	0.000	0.019	
341.74	1.32	363.33	0.020	0.000	0.020	Top of Storm Tank
342.04	1.62	364.01	0.022	0.000	0.022	
342.34	1.92	364.69	0.024	0.000	0.024	
342.64	2.22	365.37	0.026	0.000	0.026	
343.21	2.79	366.65	0.029	0.000	0.029	CB T/G
343.30	2.88	368.90	0.030	0.000	0.030	Weir
343.45	3.03	372.65	0.031	0.475	0.506	Overflow

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          B:\Working\ROHNBRAD\
"          2406127 - 422111 352 Woodlawn Rd Build\Design Phase\Design
Data\Modelling Files\Updated_Dec2024"
"          Output filename:                      Post_2yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                12/13/2024 at 8:07:25 AM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          563.699  Coefficient A"
"          1.500  Constant B"
"          0.766  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          129.248  mm/hr"
"          Total depth          33.713  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          100.000  % Impervious"
"          0.280  Total Area"
"          10.000  Flow length"
"          1.000  Overland Slope"
"          0.000  Pervious Area"
"          10.000  Pervious length"
"          1.000  Pervious slope"
"          0.280  Impervious Area"
"          10.000  Impervious length"
"          1.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

```



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"      1.500  Impervious Depression storage"
"      0.084      0.000      0.000      0.000 c.m/sec"
"      Catchment 200      Pervious      Impervious      Total Area  "
"      Surface Area      0.000      0.280      0.280      hectare"
"      Time of concentration 12.240      1.275      1.275      minutes"
"      Time to Centroid 109.467      114.982      114.982      minutes"
"      Rainfall depth 33.713      33.713      33.713      mm"
"      Rainfall volume 0.00      94.40      94.40      c.m"
"      Rainfall losses 31.156      2.230      2.230      mm"
"      Runoff depth 2.557      31.482      31.482      mm"
"      Runoff volume 0.00      88.15      88.15      c.m"
"      Runoff coefficient 0.000      0.934      0.934      "
"      Maximum flow 0.000      0.084      0.084      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.084      0.084      0.000      0.000"
" 54      POND DESIGN"
"      0.084      Current peak flow      c.m/sec"
"      0.045      Target outflow      c.m/sec"
"      88.2      Hydrograph volume      c.m"
"      5.      Number of stages"
"      0.000      Minimum water level      metre"
"      0.100      Maximum water level      metre"
"      0.000      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"      Level Discharge      Volume"
"      0.000      0.000      0.000"
"      0.02500      0.00500      69.680"
"      0.05000      0.01000      139.350"
"      0.07500      0.01500      209.030"
"      0.1000      0.02000      278.700"
"      Peak outflow      0.004      c.m/sec"
"      Maximum level      0.022      metre"
"      Maximum storage      61.149      c.m"
"      Centroidal lag      5.728      hours"
"      0.084      0.084      0.004      0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.084      0.004      0.004      0.000"
" 33      CATCHMENT 201"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      201      Catchment 201"
"      100.000      % Impervious"
"      0.750      Total Area"
"      20.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      20.000      Pervious length"

```

"	2.000	Pervious slope"			
"	0.750	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.218	0.004	0.004	0.000 c.m/sec"
"		Catchment 201	Pervious	Impervious	Total Area "
"		Surface Area	0.000	0.750	0.750 hectare"
"		Time of concentration	15.069	1.570	1.570 minutes"
"		Time to Centroid	111.645	115.522	115.522 minutes"
"		Rainfall depth	33.713	33.713	33.713 mm"
"		Rainfall volume	0.00	252.84	252.84 c.m"
"		Rainfall losses	31.149	2.016	2.016 mm"
"		Runoff depth	2.564	31.696	31.696 mm"
"		Runoff volume	0.00	237.72	237.72 c.m"
"		Runoff coefficient	0.000	0.940	0.940 "
"		Maximum flow	0.000	0.218	0.218 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.218	0.220	0.004	0.000"
" 54		POND DESIGN"			
"	0.220	Current peak flow	c.m/sec"		
"	0.045	Target outflow	c.m/sec"		
"	325.6	Hydrograph volume	c.m"		
"	12.	Number of stages"			
"	340.420	Minimum water level	metre"		
"	343.450	Maximum water level	metre"		
"	340.420	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	340.420	0.000	0.000"		
"	340.720	0.00900	82.580"		
"	341.020	0.01300	165.150"		
"	341.320	0.01600	247.730"		
"	341.620	0.01900	330.300"		
"	341.740	0.02000	363.330"		
"	342.040	0.02200	364.010"		
"	342.340	0.02400	364.690"		
"	342.640	0.02600	365.370"		
"	343.210	0.02900	366.650"		
"	343.300	0.03000	368.900"		

"	343.450	0.5060	372.650"		
"	Peak outflow		0.013	c.m/sec"	
"	Maximum level		341.045	metre"	
"	Maximum storage		172.033	c.m"	
"	Centroidal lag		5.959	hours"	
"	0.218	0.220	0.013	0.000	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.218	0.013	0.013	0.000"	
" 33	CATCHMENT 202"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	202	Catchment 202"			
"	0.000	% Impervious"			
"	0.080	Total Area"			
"	5.500	Flow length"			
"	8.000	Overland Slope"			
"	0.080	Pervious Area"			
"	5.500	Pervious length"			
"	8.000	Pervious slope"			
"	0.000	Impervious Area"			
"	5.500	Impervious length"			
"	8.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n' "			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.004	0.013	0.013	0.000	c.m/sec"
"	Catchment 202	Pervious	Impervious	Total Area	"
"	Surface Area	0.080	0.000	0.080	hectare"
"	Time of concentration	4.582	0.477	4.582	minutes"
"	Time to Centroid	103.908	0.000	103.908	minutes"
"	Rainfall depth	33.713	33.713	33.713	mm"
"	Rainfall volume	26.97	0.00	26.97	c.m"
"	Rainfall losses	31.167	33.713	31.167	mm"
"	Runoff depth	2.545	0.000	2.545	mm"
"	Runoff volume	2.04	0.00	2.04	c.m"
"	Runoff coefficient	0.076	0.000	0.076	"
"	Maximum flow	0.004	0.000	0.004	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.004	0.015	0.013	0.000"	
" 33	CATCHMENT 203"				

"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	203	Catchment 203"				
"	0.000	% Impervious"				
"	0.020	Total Area"				
"	2.500	Flow length"				
"	8.000	Overland Slope"				
"	0.020	Pervious Area"				
"	2.500	Pervious length"				
"	8.000	Pervious slope"				
"	0.000	Impervious Area"				
"	2.500	Impervious length"				
"	8.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n' "				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.001	0.015	0.013	0.000 c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	0.020	0.000	0.020	hectare"
"		Time of concentration	2.855	0.297	2.855	minutes"
"		Time to Centroid	102.414	113.909	102.414	minutes"
"		Rainfall depth	33.713	33.713	33.713	mm"
"		Rainfall volume	6.74	0.00	6.74	c.m"
"		Rainfall losses	31.196	5.140	31.196	mm"
"		Runoff depth	2.517	28.572	2.517	mm"
"		Runoff volume	0.50	0.00	0.50	c.m"
"		Runoff coefficient	0.075	0.000	0.075	"
"		Maximum flow	0.001	0.000	0.001	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.001	0.016	0.013	0.000"
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.900	Total Area"				
"	85.000	Flow length"				
"	3.500	Overland Slope"				
"	0.900	Pervious Area"				
"	85.000	Pervious length"				

"	3.500	Pervious slope"			
"	0.000	Impervious Area"			
"	85.000	Impervious length"			
"	3.500	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.013	0.016	0.013	0.000 c.m/sec"
"		Catchment 300	Pervious	Impervious	Total Area "
"		Surface Area	0.900	0.000	0.900 hectare"
"		Time of concentration	30.353	3.162	30.353 minutes"
"		Time to Centroid	122.819	118.348	122.819 minutes"
"		Rainfall depth	33.713	33.713	33.713 mm"
"		Rainfall volume	303.41	0.00	303.41 c.m"
"		Rainfall losses	31.146	1.988	31.146 mm"
"		Runoff depth	2.566	31.725	2.566 mm"
"		Runoff volume	23.10	0.00	23.10 c.m"
"		Runoff coefficient	0.076	0.000	0.076 "
"		Maximum flow	0.013	0.000	0.013 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.013	0.026	0.013	0.000"
" 38		START/RE-START TOTALS 300"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area		2.030	hectare"
"		Total Impervious area		1.030	hectare"
"		Total % impervious		50.739"	
" 19		EXIT"			

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          B:\Working\ROHNBRAD\
"          2406127 - 422111 352 Woodlawn Rd Build\Design Phase\Design
Data\Modelling Files\Updated_Dec2024"
"          Output filename:                      Post_5yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                12/13/2024 at 8:08:50 AM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          750.423 Coefficient A"
"          1.500  Constant B"
"          0.769  Exponent C"
"          0.400  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          171.091  mm/hr"
"          Total depth                44.147  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          100.000 % Impervious"
"          0.280  Total Area"
"          10.000  Flow length"
"          1.000  Overland Slope"
"          0.000  Pervious Area"
"          10.000  Pervious length"
"          1.000  Pervious slope"
"          0.280  Impervious Area"
"          10.000  Impervious length"
"          1.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

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"      1.500  Impervious Depression storage"
"      0.112      0.000      0.000      0.000 c.m/sec"
"      Catchment 200      Pervious      Impervious      Total Area  "
"      Surface Area      0.000      0.280      0.280      hectare"
"      Time of concentration 8.013      1.140      1.140      minutes"
"      Time to Centroid 107.999      113.945      113.945      minutes"
"      Rainfall depth 44.147      44.147      44.147      mm"
"      Rainfall volume 0.00      123.61      123.61      c.m"
"      Rainfall losses 35.958      2.671      2.671      mm"
"      Runoff depth 8.189      41.476      41.476      mm"
"      Runoff volume 0.00      116.13      116.13      c.m"
"      Runoff coefficient 0.000      0.939      0.939      "
"      Maximum flow 0.000      0.112      0.112      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.112      0.112      0.000      0.000"
" 54      POND DESIGN"
"      0.112      Current peak flow      c.m/sec"
"      0.045      Target outflow      c.m/sec"
"      116.1      Hydrograph volume      c.m"
"      5.      Number of stages"
"      0.000      Minimum water level      metre"
"      0.100      Maximum water level      metre"
"      0.000      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"      Level Discharge      Volume"
"      0.000      0.000      0.000"
"      0.02500      0.00500      69.680"
"      0.05000      0.01000      139.350"
"      0.07500      0.01500      209.030"
"      0.1000      0.02000      278.700"
"      Peak outflow      0.006      c.m/sec"
"      Maximum level      0.029      metre"
"      Maximum storage      80.779      c.m"
"      Centroidal lag      5.711      hours"
"      0.112      0.112      0.006      0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.112      0.006      0.006      0.000"
" 33      CATCHMENT 201"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      201      Catchment 201"
"      100.000      % Impervious"
"      0.750      Total Area"
"      20.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      20.000      Pervious length"

```

"	2.000	Pervious slope"			
"	0.750	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.293	0.006	0.006	0.000 c.m/sec"
"		Catchment 201	Pervious	Impervious	Total Area "
"		Surface Area	0.000	0.750	0.750 hectare"
"		Time of concentration	9.865	1.403	1.403 minutes"
"		Time to Centroid	109.094	114.257	114.257 minutes"
"		Rainfall depth	44.147	44.147	44.147 mm"
"		Rainfall volume	0.00	331.10	331.10 c.m"
"		Rainfall losses	36.087	2.318	2.318 mm"
"		Runoff depth	8.060	41.829	41.829 mm"
"		Runoff volume	0.00	313.72	313.72 c.m"
"		Runoff coefficient	0.000	0.947	0.947 "
"		Maximum flow	0.000	0.293	0.293 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.293	0.296	0.006	0.000"
" 54		POND DESIGN"			
"	0.296	Current peak flow	c.m/sec"		
"	0.045	Target outflow	c.m/sec"		
"	429.6	Hydrograph volume	c.m"		
"	12.	Number of stages"			
"	340.420	Minimum water level	metre"		
"	343.450	Maximum water level	metre"		
"	340.420	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	340.420	0.000	0.000"		
"	340.720	0.00900	82.580"		
"	341.020	0.01300	165.150"		
"	341.320	0.01600	247.730"		
"	341.620	0.01900	330.300"		
"	341.740	0.02000	363.330"		
"	342.040	0.02200	364.010"		
"	342.340	0.02400	364.690"		
"	342.640	0.02600	365.370"		
"	343.210	0.02900	366.650"		
"	343.300	0.03000	368.900"		

"	343.450	0.5060	372.650"		
"	Peak outflow		0.016	c.m/sec"	
"	Maximum level		341.273	metre"	
"	Maximum storage		234.713	c.m"	
"	Centroidal lag		6.301	hours"	
"	0.293	0.296	0.016	0.000	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.293	0.016	0.016	0.000"	
" 33	CATCHMENT 202"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	202	Catchment 202"			
"	0.000	% Impervious"			
"	0.080	Total Area"			
"	5.500	Flow length"			
"	8.000	Overland Slope"			
"	0.080	Pervious Area"			
"	5.500	Pervious length"			
"	8.000	Pervious slope"			
"	0.000	Impervious Area"			
"	5.500	Impervious length"			
"	8.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.010	0.016	0.016	0.000	c.m/sec"
"	Catchment 202	Pervious	Impervious	Total Area	"
"	Surface Area	0.080	0.000	0.080	hectare"
"	Time of concentration	3.000	0.427	3.000	minutes"
"	Time to Centroid	103.324	0.000	103.324	minutes"
"	Rainfall depth	44.147	44.147	44.147	mm"
"	Rainfall volume	35.32	0.00	35.32	c.m"
"	Rainfall losses	36.150	44.147	36.150	mm"
"	Runoff depth	7.998	0.000	7.998	mm"
"	Runoff volume	6.40	0.00	6.40	c.m"
"	Runoff coefficient	0.181	0.000	0.181	"
"	Maximum flow	0.010	0.000	0.010	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.010	0.023	0.016	0.000"	
" 33	CATCHMENT 203"				

"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	203	Catchment 203"				
"	0.000	% Impervious"				
"	0.020	Total Area"				
"	2.500	Flow length"				
"	8.000	Overland Slope"				
"	0.020	Pervious Area"				
"	2.500	Pervious length"				
"	8.000	Pervious slope"				
"	0.000	Impervious Area"				
"	2.500	Impervious length"				
"	8.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n' "				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.003	0.023	0.016	0.000 c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	0.020	0.000	0.020	hectare"
"		Time of concentration	1.869	0.266	1.869	minutes"
"		Time to Centroid	102.535	112.720	102.535	minutes"
"		Rainfall depth	44.147	44.147	44.147	mm"
"		Rainfall volume	8.83	0.00	8.83	c.m"
"		Rainfall losses	35.968	6.539	35.968	mm"
"		Runoff depth	8.179	37.608	8.179	mm"
"		Runoff volume	1.64	0.00	1.64	c.m"
"		Runoff coefficient	0.185	0.000	0.185	"
"		Maximum flow	0.003	0.000	0.003	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.003	0.025	0.016	0.000"
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.900	Total Area"				
"	85.000	Flow length"				
"	3.500	Overland Slope"				
"	0.900	Pervious Area"				
"	85.000	Pervious length"				

"	3.500	Pervious slope"				
"	0.000	Impervious Area"				
"	85.000	Impervious length"				
"	3.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.051	0.025	0.016	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.900	0.000	0.900	hectare"
"		Time of concentration	19.871	2.826	19.871	minutes"
"		Time to Centroid	117.612	116.855	117.612	minutes"
"		Rainfall depth	44.147	44.147	44.147	mm"
"		Rainfall volume	397.32	0.00	397.32	c.m"
"		Rainfall losses	35.965	1.987	35.965	mm"
"		Runoff depth	8.182	42.161	8.182	mm"
"		Runoff volume	73.64	0.00	73.64	c.m"
"		Runoff coefficient	0.185	0.000	0.185	"
"		Maximum flow	0.051	0.000	0.051	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.051	0.067	0.016	0.000"	
" 38		START/RE-START TOTALS 300"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area			2.030	hectare"
"		Total Impervious area			1.030	hectare"
"		Total % impervious			50.739"	
" 19		EXIT"				

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          B:\Working\ROHNBRAD\
"          2406127 - 422111 352 Woodlawn Rd Build\Design Phase\Design
Data\Modelling Files\Updated_Dec2024"
"          Output filename:                      Post_10yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                12/13/2024 at 8:09:47 AM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          855.183  Coefficient A"
"          1.500  Constant B"
"          0.764  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          196.822  mm/hr"
"          Total depth          51.710  mm"
"          6  010hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          100.000  % Impervious"
"          0.280  Total Area"
"          10.000  Flow length"
"          1.000  Overland Slope"
"          0.000  Pervious Area"
"          10.000  Pervious length"
"          1.000  Pervious slope"
"          0.280  Impervious Area"
"          10.000  Impervious length"
"          1.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

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"      1.500  Impervious Depression storage"
"      0.130      0.000      0.000      0.000 c.m/sec"
"      Catchment 200      Pervious      Impervious      Total Area  "
"      Surface Area      0.000      0.280      0.280      hectare"
"      Time of concentration 6.877      1.078      1.078      minutes"
"      Time to Centroid 107.499      113.595      113.595      minutes"
"      Rainfall depth 51.710      51.710      51.710      mm"
"      Rainfall volume 0.00      144.79      144.79      c.m"
"      Rainfall losses 39.396      3.038      3.038      mm"
"      Runoff depth 12.313      48.671      48.671      mm"
"      Runoff volume 0.00      136.28      136.28      c.m"
"      Runoff coefficient 0.000      0.941      0.941      "
"      Maximum flow 0.000      0.130      0.130      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.130      0.130      0.000      0.000"
" 54      POND DESIGN"
"      0.130      Current peak flow      c.m/sec"
"      0.045      Target outflow      c.m/sec"
"      136.3      Hydrograph volume      c.m"
"      5.      Number of stages"
"      0.000      Minimum water level      metre"
"      0.100      Maximum water level      metre"
"      0.000      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"      Level Discharge      Volume"
"      0.000      0.000      0.000"
"      0.02500      0.00500      69.680"
"      0.05000      0.01000      139.350"
"      0.07500      0.01500      209.030"
"      0.1000      0.02000      278.700"
"      Peak outflow      0.007      c.m/sec"
"      Maximum level      0.034      metre"
"      Maximum storage      94.494      c.m"
"      Centroidal lag      5.705      hours"
"      0.130      0.130      0.007      0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.130      0.007      0.007      0.000"
" 33      CATCHMENT 201"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      201      Catchment 201"
"      100.000      % Impervious"
"      0.750      Total Area"
"      20.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      20.000      Pervious length"

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"	2.000	Pervious slope"			
"	0.750	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.340	0.007	0.007	0.000 c.m/sec"	
"	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.000	0.750	0.750	hectare"
"	Time of concentration	8.466	1.327	1.327	minutes"
"	Time to Centroid	108.900	113.872	113.872	minutes"
"	Rainfall depth	51.710	51.710	51.710	mm"
"	Rainfall volume	0.00	387.82	387.82	c.m"
"	Rainfall losses	39.099	2.557	2.557	mm"
"	Runoff depth	12.611	49.152	49.152	mm"
"	Runoff volume	0.00	368.64	368.64	c.m"
"	Runoff coefficient	0.000	0.951	0.951	"
"	Maximum flow	0.000	0.340	0.340	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.340	0.343	0.007	0.000"	
" 54	POND DESIGN"				
"	0.343	Current peak flow	c.m/sec"		
"	0.045	Target outflow	c.m/sec"		
"	504.6	Hydrograph volume	c.m"		
"	12.	Number of stages"			
"	340.420	Minimum water level	metre"		
"	343.450	Maximum water level	metre"		
"	340.420	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"	Level Discharge	Volume"			
"	340.420	0.000	0.000"		
"	340.720	0.00900	82.580"		
"	341.020	0.01300	165.150"		
"	341.320	0.01600	247.730"		
"	341.620	0.01900	330.300"		
"	341.740	0.02000	363.330"		
"	342.040	0.02200	364.010"		
"	342.340	0.02400	364.690"		
"	342.640	0.02600	365.370"		
"	343.210	0.02900	366.650"		
"	343.300	0.03000	368.900"		

"	343.450	0.5060	372.650"		
"	Peak outflow		0.017	c.m/sec"	
"	Maximum level		341.436	metre"	
"	Maximum storage		279.649	c.m"	
"	Centroidal lag		6.525	hours"	
"	0.340	0.343	0.017	0.000	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.340	0.017	0.017	0.000"	
" 33	CATCHMENT 202"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	202	Catchment 202"			
"	0.000	% Impervious"			
"	0.080	Total Area"			
"	5.500	Flow length"			
"	8.000	Overland Slope"			
"	0.080	Pervious Area"			
"	5.500	Pervious length"			
"	8.000	Pervious slope"			
"	0.000	Impervious Area"			
"	5.500	Impervious length"			
"	8.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n' "			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.016	0.017	0.017	0.000	c.m/sec"
"	Catchment 202	Pervious	Impervious	Total Area	"
"	Surface Area	0.080	0.000	0.080	hectare"
"	Time of concentration	2.574	0.403	2.574	minutes"
"	Time to Centroid	103.534	112.962	103.534	minutes"
"	Rainfall depth	51.710	51.710	51.710	mm"
"	Rainfall volume	41.37	0.00	41.37	c.m"
"	Rainfall losses	39.190	6.087	39.190	mm"
"	Runoff depth	12.519	45.623	12.519	mm"
"	Runoff volume	10.02	0.00	10.02	c.m"
"	Runoff coefficient	0.242	0.000	0.242	"
"	Maximum flow	0.016	0.000	0.016	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.016	0.029	0.017	0.000"	
" 33	CATCHMENT 203"				

"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	203	Catchment 203"				
"	0.000	% Impervious"				
"	0.020	Total Area"				
"	2.500	Flow length"				
"	8.000	Overland Slope"				
"	0.020	Pervious Area"				
"	2.500	Pervious length"				
"	8.000	Pervious slope"				
"	0.000	Impervious Area"				
"	2.500	Impervious length"				
"	8.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n' "				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.005	0.029	0.017	0.000 c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	0.020	0.000	0.020	hectare"
"		Time of concentration	1.604	0.251	1.604	minutes"
"		Time to Centroid	102.636	112.304	102.636	minutes"
"		Rainfall depth	51.710	51.710	51.710	mm"
"		Rainfall volume	10.34	0.00	10.34	c.m"
"		Rainfall losses	39.304	7.577	39.304	mm"
"		Runoff depth	12.405	44.132	12.405	mm"
"		Runoff volume	2.48	0.00	2.48	c.m"
"		Runoff coefficient	0.240	0.000	0.240	"
"		Maximum flow	0.005	0.000	0.005	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.005	0.032	0.017	0.000"
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.900	Total Area"				
"	85.000	Flow length"				
"	3.500	Overland Slope"				
"	0.900	Pervious Area"				
"	85.000	Pervious length"				

"	3.500	Pervious slope"			
"	0.000	Impervious Area"			
"	85.000	Impervious length"			
"	3.500	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.090	0.032	0.017	0.000 c.m/sec"
"		Catchment 300	Pervious	Impervious	Total Area "
"		Surface Area	0.900	0.000	0.900 hectare"
"		Time of concentration	17.054	2.672	17.054 minutes"
"		Time to Centroid	116.553	116.307	116.553 minutes"
"		Rainfall depth	51.710	51.710	51.710 mm"
"		Rainfall volume	465.39	0.00	465.39 c.m"
"		Rainfall losses	39.111	2.012	39.111 mm"
"		Runoff depth	12.599	49.698	12.599 mm"
"		Runoff volume	113.39	0.00	113.39 c.m"
"		Runoff coefficient	0.244	0.000	0.244 "
"		Maximum flow	0.090	0.000	0.090 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.090	0.108	0.017	0.000"
" 38		START/RE-START TOTALS 300"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area		2.030	hectare"
"		Total Impervious area		1.030	hectare"
"		Total % impervious		50.739"	
" 19		EXIT"			

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          B:\Working\ROHNBRAD\
"          2406127 - 422111 352 Woodlawn Rd Build\Design Phase\Design
Data\Modelling Files\Updated_Dec2024"
"          Output filename:                      Post_25yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                12/13/2024 at 8:10:49 AM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          972.202  Coefficient A"
"          1.500  Constant B"
"          0.752  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          228.875  mm/hr"
"          Total depth          62.786  mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          100.000  % Impervious"
"          0.280  Total Area"
"          10.000  Flow length"
"          1.000  Overland Slope"
"          0.000  Pervious Area"
"          10.000  Pervious length"
"          1.000  Pervious slope"
"          0.280  Impervious Area"
"          10.000  Impervious length"
"          1.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

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"      1.500  Impervious Depression storage"
"      0.152      0.000      0.000      0.000 c.m/sec"
"      Catchment 200      Pervious      Impervious      Total Area  "
"      Surface Area      0.000      0.280      0.280      hectare"
"      Time of concentration 5.980      1.014      1.014      minutes"
"      Time to Centroid 108.072      113.354      113.354      minutes"
"      Rainfall depth 62.786      62.786      62.786      mm"
"      Rainfall volume 0.00      175.80      175.80      c.m"
"      Rainfall losses 43.578      3.604      3.605      mm"
"      Runoff depth 19.208      59.182      59.181      mm"
"      Runoff volume 0.00      165.71      165.71      c.m"
"      Runoff coefficient 0.000      0.943      0.943      "
"      Maximum flow 0.000      0.152      0.152      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.152      0.152      0.000      0.000"
" 54      POND DESIGN"
"      0.152      Current peak flow      c.m/sec"
"      0.045      Target outflow      c.m/sec"
"      165.7      Hydrograph volume      c.m"
"      5.      Number of stages"
"      0.000      Minimum water level      metre"
"      0.100      Maximum water level      metre"
"      0.000      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"      Level Discharge      Volume"
"      0.000      0.000      0.000"
"      0.02500      0.00500      69.680"
"      0.05000      0.01000      139.350"
"      0.07500      0.01500      209.030"
"      0.1000      0.02000      278.700"
"      Peak outflow      0.008      c.m/sec"
"      Maximum level      0.041      metre"
"      Maximum storage      113.954      c.m"
"      Centroidal lag      5.701      hours"
"      0.152      0.152      0.008      0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.152      0.008      0.008      0.000"
" 33      CATCHMENT 201"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      201      Catchment 201"
"      100.000      % Impervious"
"      0.750      Total Area"
"      20.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      20.000      Pervious length"

```

"	2.000	Pervious slope"			
"	0.750	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.399	0.008	0.008	0.000 c.m/sec"	
"	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.000	0.750	0.750	hectare"
"	Time of concentration	7.362	1.249	1.249	minutes"
"	Time to Centroid	109.562	113.739	113.739	minutes"
"	Rainfall depth	62.786	62.786	62.786	mm"
"	Rainfall volume	0.00	470.89	470.89	c.m"
"	Rainfall losses	43.605	2.883	2.883	mm"
"	Runoff depth	19.181	59.903	59.903	mm"
"	Runoff volume	0.00	449.27	449.27	c.m"
"	Runoff coefficient	0.000	0.954	0.954	"
"	Maximum flow	0.000	0.399	0.399	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.399	0.403	0.008	0.000"	
" 54	POND DESIGN"				
"	0.403	Current peak flow	c.m/sec"		
"	0.045	Target outflow	c.m/sec"		
"	614.6	Hydrograph volume	c.m"		
"	12.	Number of stages"			
"	340.420	Minimum water level	metre"		
"	343.450	Maximum water level	metre"		
"	340.420	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"	Level Discharge	Volume"			
"	340.420	0.000	0.000"		
"	340.720	0.00900	82.580"		
"	341.020	0.01300	165.150"		
"	341.320	0.01600	247.730"		
"	341.620	0.01900	330.300"		
"	341.740	0.02000	363.330"		
"	342.040	0.02200	364.010"		
"	342.340	0.02400	364.690"		
"	342.640	0.02600	365.370"		
"	343.210	0.02900	366.650"		
"	343.300	0.03000	368.900"		

"	343.450	0.5060	372.650"		
"	Peak outflow		0.019	c.m/sec"	
"	Maximum level		341.675	metre"	
"	Maximum storage		345.554	c.m"	
"	Centroidal lag		6.819	hours"	
"	0.399	0.403	0.019	0.000	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.399	0.019	0.019	0.000"	
" 33	CATCHMENT 202"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	202	Catchment 202"			
"	0.000	% Impervious"			
"	0.080	Total Area"			
"	5.500	Flow length"			
"	8.000	Overland Slope"			
"	0.080	Pervious Area"			
"	5.500	Pervious length"			
"	8.000	Pervious slope"			
"	0.000	Impervious Area"			
"	5.500	Impervious length"			
"	8.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.024	0.019	0.019	0.000	c.m/sec"
"	Catchment 202	Pervious	Impervious	Total Area	"
"	Surface Area	0.080	0.000	0.080	hectare"
"	Time of concentration	2.239	0.380	2.239	minutes"
"	Time to Centroid	104.247	112.783	104.247	minutes"
"	Rainfall depth	62.786	62.786	62.786	mm"
"	Rainfall volume	50.23	0.00	50.23	c.m"
"	Rainfall losses	43.484	7.294	43.484	mm"
"	Runoff depth	19.302	55.492	19.302	mm"
"	Runoff volume	15.44	0.00	15.44	c.m"
"	Runoff coefficient	0.307	0.000	0.307	"
"	Maximum flow	0.024	0.000	0.024	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.024	0.037	0.019	0.000"	
" 33	CATCHMENT 203"				

"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	203	Catchment 203"				
"	0.000	% Impervious"				
"	0.020	Total Area"				
"	2.500	Flow length"				
"	8.000	Overland Slope"				
"	0.020	Pervious Area"				
"	2.500	Pervious length"				
"	8.000	Pervious slope"				
"	0.000	Impervious Area"				
"	2.500	Impervious length"				
"	8.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n' "				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.007	0.037	0.019	0.000 c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	0.020	0.000	0.020	hectare"
"		Time of concentration	1.395	0.237	1.395	minutes"
"		Time to Centroid	103.398	112.037	103.398	minutes"
"		Rainfall depth	62.786	62.786	62.786	mm"
"		Rainfall volume	12.56	0.00	12.56	c.m"
"		Rainfall losses	43.872	9.189	43.872	mm"
"		Runoff depth	18.914	53.597	18.914	mm"
"		Runoff volume	3.78	0.00	3.78	c.m"
"		Runoff coefficient	0.301	0.000	0.301	"
"		Maximum flow	0.007	0.000	0.007	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.007	0.044	0.019	0.000"
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.900	Total Area"				
"	85.000	Flow length"				
"	3.500	Overland Slope"				
"	0.900	Pervious Area"				
"	85.000	Pervious length"				

"	3.500	Pervious slope"				
"	0.000	Impervious Area"				
"	85.000	Impervious length"				
"	3.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.148	0.044	0.019	0.000	c.m/sec"
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.900	0.000	0.900	hectare"
"		Time of concentration	14.829	2.516	14.829	minutes"
"		Time to Centroid	116.832	115.974	116.832	minutes"
"		Rainfall depth	62.786	62.786	62.786	mm"
"		Rainfall volume	565.07	0.00	565.07	c.m"
"		Rainfall losses	43.426	2.155	43.426	mm"
"		Runoff depth	19.360	60.631	19.360	mm"
"		Runoff volume	174.24	0.00	174.24	c.m"
"		Runoff coefficient	0.308	0.000	0.308	"
"		Maximum flow	0.148	0.000	0.148	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.148	0.170	0.019	0.000"	
" 38		START/RE-START TOTALS 300"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		2.030	hectare"	
"		Total Impervious area		1.030	hectare"	
"		Total % impervious		50.739"		
" 19		EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          B:\Working\ROHNBRAD\
"          2406127 - 422111 352 Woodlawn Rd Build\Design Phase\Design
Data\Modelling Files\Updated_Dec2024"
"          Output filename:                      Post_50yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                12/13/2024 at 8:13:10 AM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          1054.539  Coefficient A"
"          1.500  Constant B"
"          0.746  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          251.085  mm/hr"
"          Total depth          70.383  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          100.000  % Impervious"
"          0.280  Total Area"
"          10.000  Flow length"
"          1.000  Overland Slope"
"          0.000  Pervious Area"
"          10.000  Pervious length"
"          1.000  Pervious slope"
"          0.280  Impervious Area"
"          10.000  Impervious length"
"          1.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

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"      1.500  Impervious Depression storage"
"      0.167      0.000      0.000      0.000 c.m/sec"
"      Catchment 200      Pervious      Impervious      Total Area  "
"      Surface Area      0.000      0.280      0.280      hectare"
"      Time of concentration 5.559      0.978      0.978      minutes"
"      Time to Centroid      108.752      113.191      113.191      minutes"
"      Rainfall depth      70.383      70.383      70.383      mm"
"      Rainfall volume      0.00      197.07      197.07      c.m"
"      Rainfall losses      46.029      4.038      4.038      mm"
"      Runoff depth      24.354      66.345      66.345      mm"
"      Runoff volume      0.00      185.76      185.76      c.m"
"      Runoff coefficient      0.000      0.943      0.943      "
"      Maximum flow      0.000      0.167      0.167      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.167      0.167      0.000      0.000"
" 54      POND DESIGN"
"      0.167      Current peak flow      c.m/sec"
"      0.045      Target outflow      c.m/sec"
"      185.8      Hydrograph volume      c.m"
"      5.      Number of stages"
"      0.000      Minimum water level      metre"
"      0.100      Maximum water level      metre"
"      0.000      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"      Level Discharge      Volume"
"      0.000      0.000      0.000"
"      0.02500      0.00500      69.680"
"      0.05000      0.01000      139.350"
"      0.07500      0.01500      209.030"
"      0.1000      0.02000      278.700"
"      Peak outflow      0.009      c.m/sec"
"      Maximum level      0.046      metre"
"      Maximum storage      127.240      c.m"
"      Centroidal lag      5.698      hours"
"      0.167      0.167      0.009      0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.167      0.009      0.009      0.000"
" 33      CATCHMENT 201"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      201      Catchment 201"
"      100.000      % Impervious"
"      0.750      Total Area"
"      20.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      20.000      Pervious length"

```

"	2.000	Pervious slope"			
"	0.750	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.440	0.009	0.009	0.000 c.m/sec"	
"	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.000	0.750	0.750	hectare"
"	Time of concentration	6.844	1.204	1.204	minutes"
"	Time to Centroid	110.091	113.625	113.625	minutes"
"	Rainfall depth	70.383	70.383	70.383	mm"
"	Rainfall volume	0.00	527.87	527.87	c.m"
"	Rainfall losses	46.539	3.161	3.161	mm"
"	Runoff depth	23.844	67.222	67.222	mm"
"	Runoff volume	0.00	504.17	504.17	c.m"
"	Runoff coefficient	0.000	0.955	0.955	"
"	Maximum flow	0.000	0.440	0.440	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.440	0.445	0.009	0.000"	
" 54	POND DESIGN"				
"	0.445	Current peak flow	c.m/sec"		
"	0.045	Target outflow	c.m/sec"		
"	689.5	Hydrograph volume	c.m"		
"	12.	Number of stages"			
"	340.420	Minimum water level	metre"		
"	343.450	Maximum water level	metre"		
"	340.420	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"	Level Discharge	Volume"			
"	340.420	0.000	0.000"		
"	340.720	0.00900	82.580"		
"	341.020	0.01300	165.150"		
"	341.320	0.01600	247.730"		
"	341.620	0.01900	330.300"		
"	341.740	0.02000	363.330"		
"	342.040	0.02200	364.010"		
"	342.340	0.02400	364.690"		
"	342.640	0.02600	365.370"		
"	343.210	0.02900	366.650"		
"	343.300	0.03000	368.900"		

"	343.450	0.5060	372.650"		
"	Peak outflow		0.032	c.m/sec"	
"	Maximum level		343.301	metre"	
"	Maximum storage		368.919	c.m"	
"	Centroidal lag		6.740	hours"	
"	0.440	0.445	0.032	0.000	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.440	0.032	0.032	0.000"	
" 33	CATCHMENT 202"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	202	Catchment 202"			
"	0.000	% Impervious"			
"	0.080	Total Area"			
"	5.500	Flow length"			
"	8.000	Overland Slope"			
"	0.080	Pervious Area"			
"	5.500	Pervious length"			
"	8.000	Pervious slope"			
"	0.000	Impervious Area"			
"	5.500	Impervious length"			
"	8.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.030	0.032	0.032	0.000	c.m/sec"
"	Catchment 202	Pervious	Impervious	Total Area	"
"	Surface Area	0.080	0.000	0.080	hectare"
"	Time of concentration	2.081	0.366	2.081	minutes"
"	Time to Centroid	104.934	112.663	104.934	minutes"
"	Rainfall depth	70.383	70.383	70.383	mm"
"	Rainfall volume	56.31	0.00	56.31	c.m"
"	Rainfall losses	46.075	8.189	46.074	mm"
"	Runoff depth	24.308	62.194	24.308	mm"
"	Runoff volume	19.45	0.00	19.45	c.m"
"	Runoff coefficient	0.345	0.000	0.345	"
"	Maximum flow	0.030	0.000	0.030	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.030	0.043	0.032	0.000"	
" 33	CATCHMENT 203"				

"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	203	Catchment 203"				
"	0.000	% Impervious"				
"	0.020	Total Area"				
"	2.500	Flow length"				
"	8.000	Overland Slope"				
"	0.020	Pervious Area"				
"	2.500	Pervious length"				
"	8.000	Pervious slope"				
"	0.000	Impervious Area"				
"	2.500	Impervious length"				
"	8.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n' "				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.009	0.043	0.032	0.000 c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	0.020	0.000	0.020	hectare"
"		Time of concentration	1.297	0.228	1.297	minutes"
"		Time to Centroid	104.067	111.866	104.067	minutes"
"		Rainfall depth	70.383	70.383	70.383	mm"
"		Rainfall volume	14.08	0.00	14.08	c.m"
"		Rainfall losses	46.714	10.321	46.714	mm"
"		Runoff depth	23.668	60.062	23.669	mm"
"		Runoff volume	4.73	0.00	4.73	c.m"
"		Runoff coefficient	0.336	0.000	0.336	"
"		Maximum flow	0.009	0.000	0.009	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.009	0.052	0.032	0.000"
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.900	Total Area"				
"	85.000	Flow length"				
"	3.500	Overland Slope"				
"	0.900	Pervious Area"				
"	85.000	Pervious length"				

"	3.500	Pervious slope"				
"	0.000	Impervious Area"				
"	85.000	Impervious length"				
"	3.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.182	0.052	0.032	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.900	0.000	0.900	hectare"
"		Time of concentration	13.786	2.424	13.786	minutes"
"		Time to Centroid	117.394	115.770	117.394	minutes"
"		Rainfall depth	70.383	70.383	70.383	mm"
"		Rainfall volume	633.44	0.00	633.44	c.m"
"		Rainfall losses	46.042	2.296	46.042	mm"
"		Runoff depth	24.341	68.087	24.341	mm"
"		Runoff volume	219.07	0.00	219.07	c.m"
"		Runoff coefficient	0.346	0.000	0.346	"
"		Maximum flow	0.182	0.000	0.182	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.182	0.207	0.032	0.000"	
" 38		START/RE-START TOTALS 300"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		2.030	hectare"	
"		Total Impervious area		1.030	hectare"	
"		Total % impervious		50.739"		
" 19		EXIT"				

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        B:\Working\ROHNBRAD\
"          2406127 - 422111 352 Woodlawn Rd Build\Design Phase\Design
Data\Modelling Files\Updated_Dec2024"
"          Output filename:                    Post_100yr.out"
"          Licensee name:                      "
"          Company                            "
"          Date & Time last used:              12/13/2024 at 8:14:00 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1122.601 Coefficient A"
"          1.500  Constant B"
"          0.738  Exponent C"
"          0.400  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          271.357  mm/hr"
"          Total depth                78.288  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          100.000 % Impervious"
"          0.280  Total Area"
"          10.000  Flow length"
"          1.000  Overland Slope"
"          0.000  Pervious Area"
"          10.000  Pervious length"
"          1.000  Pervious slope"
"          0.280  Impervious Area"
"          10.000  Impervious length"
"          1.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.001  Impervious Lag constant (hours)"

```

```

"      1.500  Impervious Depression storage"
"      0.181      0.000      0.000      0.000 c.m/sec"
"      Catchment 200      Pervious      Impervious      Total Area  "
"      Surface Area      0.000      0.280      0.280      hectare"
"      Time of concentration 5.296      0.948      0.948      minutes"
"      Time to Centroid 109.575      113.099      113.099      minutes"
"      Rainfall depth 78.288      78.288      78.288      mm"
"      Rainfall volume 0.00      219.20      219.21      c.m"
"      Rainfall losses 48.444      4.469      4.469      mm"
"      Runoff depth 29.844      73.818      73.818      mm"
"      Runoff volume 0.00      206.69      206.69      c.m"
"      Runoff coefficient 0.000      0.943      0.943      "
"      Maximum flow 0.000      0.181      0.181      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.181      0.181      0.000      0.000"
" 54      POND DESIGN"
"      0.181      Current peak flow      c.m/sec"
"      0.045      Target outflow      c.m/sec"
"      206.7      Hydrograph volume      c.m"
"      5.      Number of stages"
"      0.000      Minimum water level      metre"
"      0.100      Maximum water level      metre"
"      0.000      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"      Level Discharge      Volume"
"      0.000      0.000      0.000"
"      0.02500      0.00500      69.680"
"      0.05000      0.01000      139.350"
"      0.07500      0.01500      209.030"
"      0.1000      0.02000      278.700"
"      Peak outflow      0.010      c.m/sec"
"      Maximum level      0.051      metre"
"      Maximum storage      140.815      c.m"
"      Centroidal lag      5.696      hours"
"      0.181      0.181      0.010      0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.181      0.010      0.010      0.000"
" 33      CATCHMENT 201"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      201      Catchment 201"
"      100.000      % Impervious"
"      0.750      Total Area"
"      20.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      20.000      Pervious length"

```

"	2.000	Pervious slope"			
"	0.750	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.478	0.010	0.010	0.000	c.m/sec"
"	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.000	0.750	0.750	hectare"
"	Time of concentration	6.520	1.167	1.167	minutes"
"	Time to Centroid	110.981	113.592	113.592	minutes"
"	Rainfall depth	78.288	78.288	78.288	mm"
"	Rainfall volume	0.00	587.16	587.16	c.m"
"	Rainfall losses	48.955	3.477	3.477	mm"
"	Runoff depth	29.332	74.810	74.810	mm"
"	Runoff volume	0.00	561.08	561.08	c.m"
"	Runoff coefficient	0.000	0.956	0.956	"
"	Maximum flow	0.000	0.478	0.478	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.478	0.483	0.010	0.000"	
" 54	POND DESIGN"				
"	0.483	Current peak flow	c.m/sec"		
"	0.045	Target outflow	c.m/sec"		
"	767.2	Hydrograph volume	c.m"		
"	12.	Number of stages"			
"	340.420	Minimum water level	metre"		
"	343.450	Maximum water level	metre"		
"	340.420	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"	Level Discharge	Volume"			
"	340.420	0.000	0.000"		
"	340.720	0.00900	82.580"		
"	341.020	0.01300	165.150"		
"	341.320	0.01600	247.730"		
"	341.620	0.01900	330.300"		
"	341.740	0.02000	363.330"		
"	342.040	0.02200	364.010"		
"	342.340	0.02400	364.690"		
"	342.640	0.02600	365.370"		
"	343.210	0.02900	366.650"		
"	343.300	0.03000	368.900"		

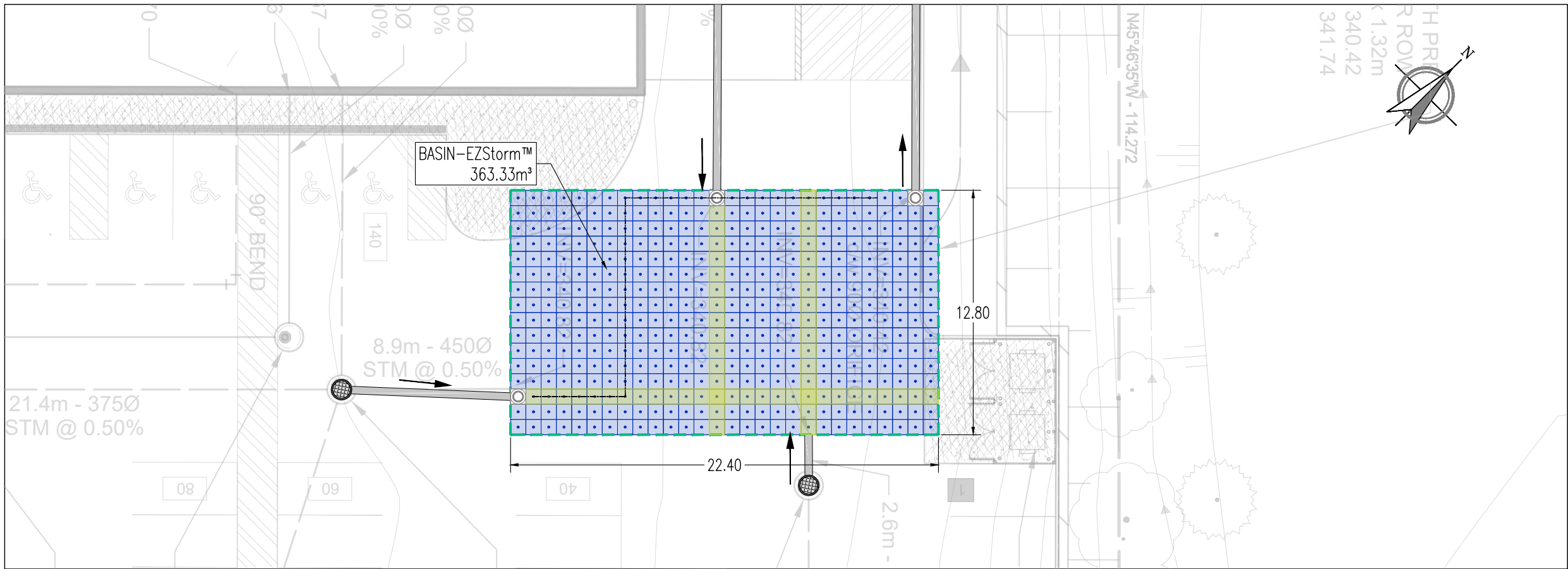
"	343.450	0.5060	372.650"		
"	Peak outflow		0.075	c.m/sec"	
"	Maximum level		343.314	metre"	
"	Maximum storage		369.257	c.m"	
"	Centroidal lag		6.424	hours"	
"	0.478	0.483	0.075	0.000	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.478	0.075	0.075	0.000"	
" 33	CATCHMENT 202"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	202	Catchment 202"			
"	0.000	% Impervious"			
"	0.080	Total Area"			
"	5.500	Flow length"			
"	8.000	Overland Slope"			
"	0.080	Pervious Area"			
"	5.500	Pervious length"			
"	8.000	Pervious slope"			
"	0.000	Impervious Area"			
"	5.500	Impervious length"			
"	8.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	12.500	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.001	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.035	0.075	0.075	0.000	c.m/sec"
"	Catchment 202	Pervious	Impervious	Total Area	"
"	Surface Area	0.080	0.000	0.080	hectare"
"	Time of concentration	1.983	0.355	1.983	minutes"
"	Time to Centroid	105.591	112.631	105.591	minutes"
"	Rainfall depth	78.288	78.288	78.288	mm"
"	Rainfall volume	62.63	0.00	62.63	c.m"
"	Rainfall losses	48.628	9.144	48.628	mm"
"	Runoff depth	29.660	69.143	29.660	mm"
"	Runoff volume	23.73	0.00	23.73	c.m"
"	Runoff coefficient	0.379	0.000	0.379	"
"	Maximum flow	0.035	0.000	0.035	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.035	0.079	0.075	0.000"	
" 33	CATCHMENT 203"				

"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	203	Catchment 203"				
"	0.000	% Impervious"				
"	0.020	Total Area"				
"	2.500	Flow length"				
"	8.000	Overland Slope"				
"	0.020	Pervious Area"				
"	2.500	Pervious length"				
"	8.000	Pervious slope"				
"	0.000	Impervious Area"				
"	2.500	Impervious length"				
"	8.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n' "				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.010	0.079	0.075	0.000 c.m/sec"	
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	0.020	0.000	0.020	hectare"
"		Time of concentration	1.235	0.221	1.235	minutes"
"		Time to Centroid	104.712	111.796	104.712	minutes"
"		Rainfall depth	78.288	78.288	78.288	mm"
"		Rainfall volume	15.66	0.00	15.66	c.m"
"		Rainfall losses	49.376	11.510	49.376	mm"
"		Runoff depth	28.912	66.777	28.912	mm"
"		Runoff volume	5.78	0.00	5.78	c.m"
"		Runoff coefficient	0.369	0.000	0.369	"
"		Maximum flow	0.010	0.000	0.010	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.010	0.080	0.075	0.000"	
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.900	Total Area"				
"	85.000	Flow length"				
"	3.500	Overland Slope"				
"	0.900	Pervious Area"				
"	85.000	Pervious length"				

"	3.500	Pervious slope"				
"	0.000	Impervious Area"				
"	85.000	Impervious length"				
"	3.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.001	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.211	0.080	0.075	0.000	c.m/sec"
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.900	0.000	0.900	hectare"
"		Time of concentration	13.133	2.350	13.133	minutes"
"		Time to Centroid	118.576	115.652	118.576	minutes"
"		Rainfall depth	78.288	78.288	78.288	mm"
"		Rainfall volume	704.59	0.00	704.59	c.m"
"		Rainfall losses	48.654	2.510	48.654	mm"
"		Runoff depth	29.634	75.778	29.634	mm"
"		Runoff volume	266.70	0.00	266.70	c.m"
"		Runoff coefficient	0.379	0.000	0.379	"
"		Maximum flow	0.211	0.000	0.211	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.211	0.239	0.075	0.000"	
" 38		START/RE-START TOTALS 300"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		2.030	hectare"	
"		Total Impervious area		1.030	hectare"	
"		Total % impervious		50.739"		
" 19		EXIT"				

C.3. Oil and Grit Separator and Stormwater Tank Details

81 ROYAL ROAD GUELPH MANUFACTURING, ON



INDEX

PAGE

COVER PAGE AND SYSTEM OVERLAY 1 of 6
SYSTEM LAYOUT - PLAN AND PROFILE 2 of 6
VOLUME CALCULATION SHEET 3 of 6
STANDARD BACKFILL REQUIREMENTS 4 of 6
LIST OF MATERIALS 5 of 6
ACCESSORIES 6 of 6

1	IMPLANTATION
01	SCALE 1:250

CONTACTS

SITE CONTACT	PARTH PUSHKARNA 647 278-7339 ppushkarna@brunet.cc
SALES REPRESENTATIVE	PARTH PUSHKARNA 647 278-7339 ppushkarna@brunet.cc
TECNICAL SUPPORT	NEXTSTORM 450 322-6260 info@nextstorm.ca

NOTE :

- These drawings may contain components, including but not limited to manholes, catch basins, storm pipes, fittings, manifolds, castings or other necessary appurtenances that may not be supplied by Nextstorm.
- It is the responsibility of the contractor to confirm all the material required is provided before installation.
- This drawing was prepared to support the project engineer of record for the proposed system. It is the ultimate responsibility of the project engineer of record to ensure that the EZSTORM™ System's design is in full compliance with all applicable laws and regulations. It is the contractor of record's responsibility to ensure that the Nextstorm products are designed in accordance with Nexstorm's minimum requirements. Nextstorm does not approve plans, sizings or systems designs.
- All measurements are in meters unless otherwise indicated.

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A	ISSUED FOR APPROVAL	16/08/2024	S.M.
N°.	REVISION	DATE	BY

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COVER

BASIN-EZSTORM™-363.33M³

PROJECT NAME:
81 ROYAL ROAD GUELPH
MANUFACTURING, ON

PROJECT N°:
240815-06

DATE:
16/08/2024

DRAWN BY:
S.M.

CHECKED BY:
S.K.

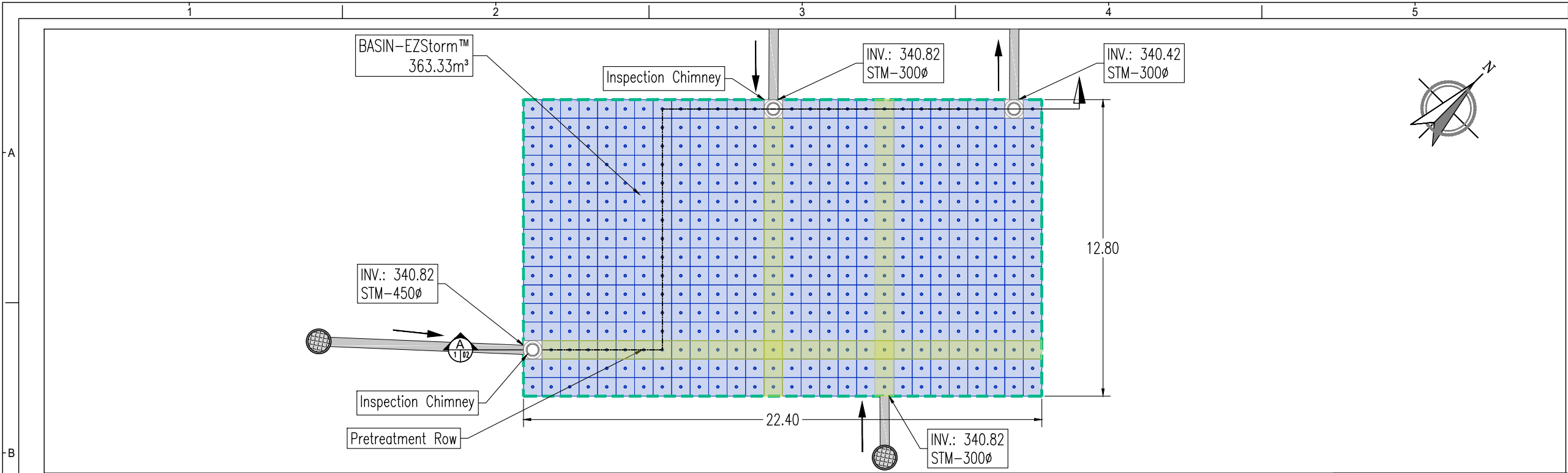
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SHEET N°:
6/6


NEXT STORM

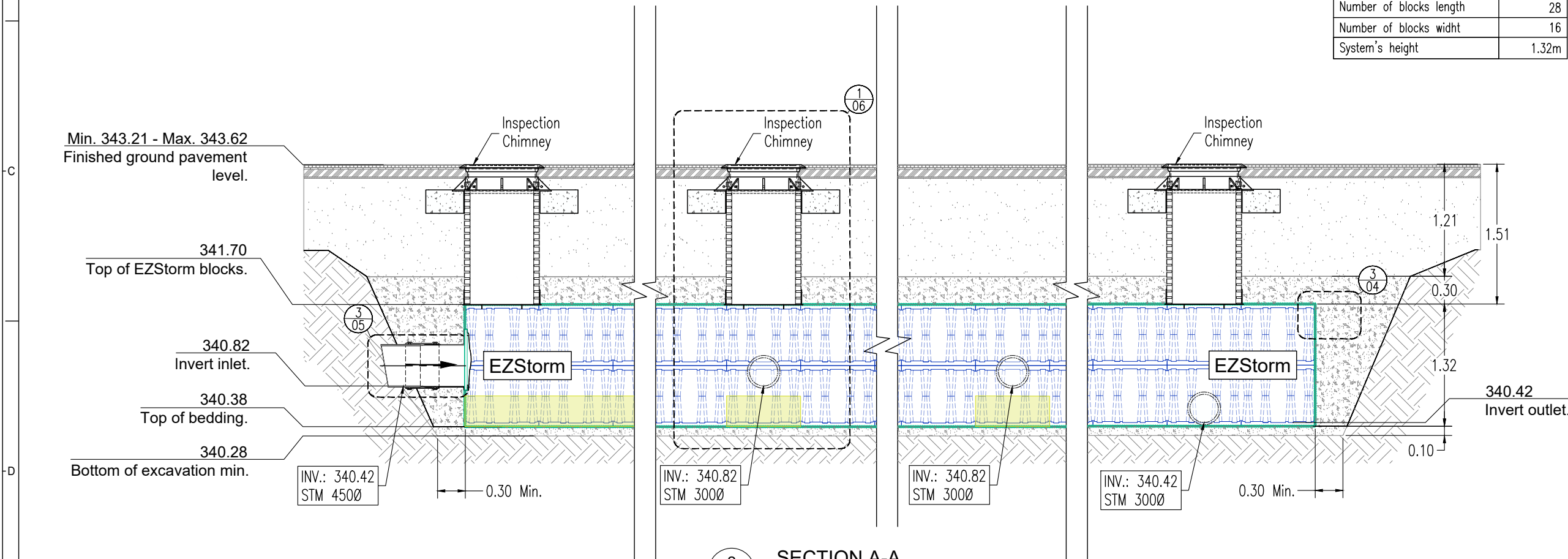
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Salaberry-de-Valleyfield J6S 1C2, Québec



1
02 2 LAYER PLAN VIEW
SCALE 1:200

EZSTORM™ SYSTEM	
Total volume storage capacity	363.33m³
EZSTORM™ Storage Volume	363.33m³
Clean Stone Storage Volume	000.0m³
System Area	286.72m²
Number of blocks length	28
Number of blocks width	16
System's height	1.32m



2
02 SECTION A-A
SCALE 1:50

LEGEND

Inspection Chimney

Catch basin

Connection-acces concrete manhole

Manhole

A Geotextile EZ-226.

B Geomembrane EZ-LLDPE-30

Drain PEHD 1500 (By others)

Clean Stone with maximum grain size of 20mm at 40% void ratio or MG-20 or MG-112 compacted sand at 90%PM

Thickness of this layer may vary according to project requirements

Paving bed

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PLAN ET SECTION DU BASIN

BASIN-EZSTORM™-363.33M³

PROJECT NAME:
81 ROYAL ROAD GUELPH
MANUFACTURING, ON

PROJECT N°:
240815-06

DATE:
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6/6

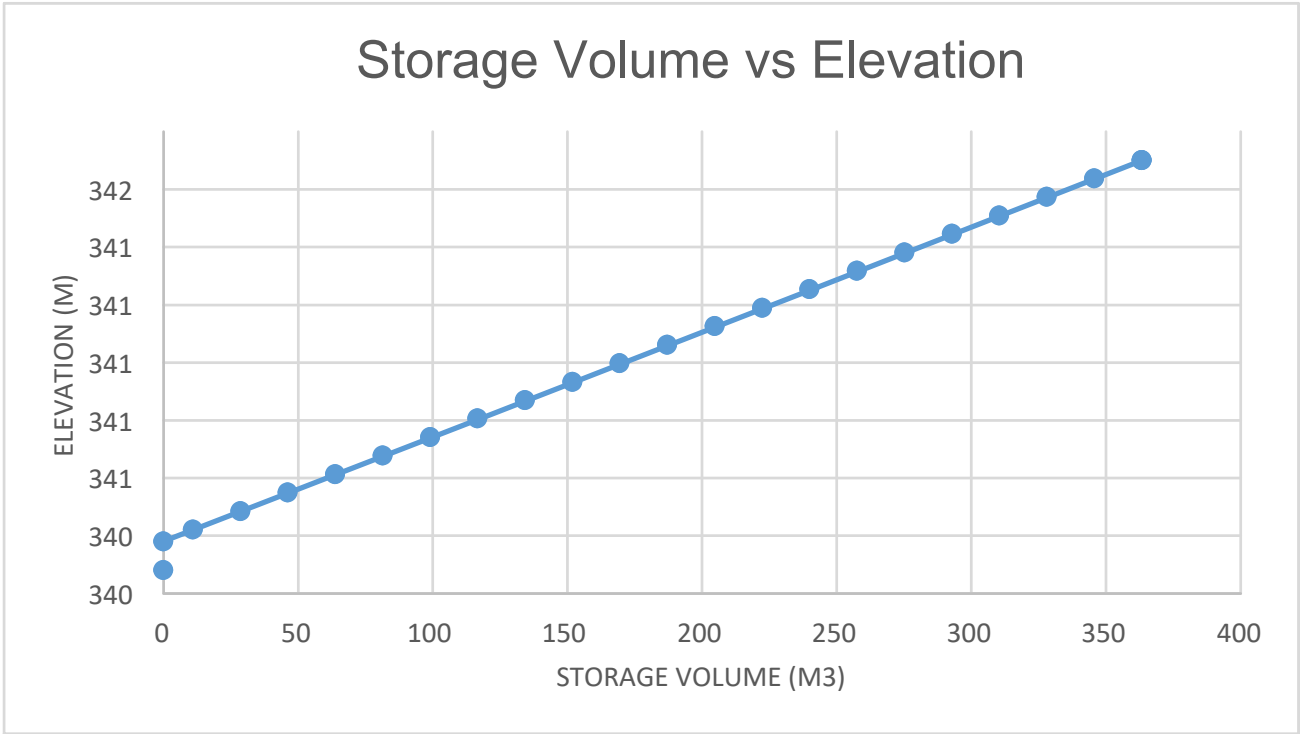
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Salaberry-de-Valleyfield J6S 1C2, Québec

SYSTEM CHARACTERISTICS			
Model	EZSTORM™ system B1		
	Number of blocks (unit)	Dimensions / blocks (m)	Dimensions EZStorm (m)
Height	2,0	0,66	1,32
Length	28	0,80	22,40
Width	16	0,80	12,80

EZSTORM area (m2)	286,7
EZSTORM + Clear stone area (m2)	0,0
Total storage volume (m3)	363,3
Invert (m)	340,42
Min finished ground level (m)	343,21

EZSTORM volume (m3)	363,3	Clear stone volume (m3)	0,0
Void in EZSTORM (%)	96%	Void in Clear stone (%)	40%

System height (m)	Storage volume (m3)	Elevation (m)	Notes
1,32	363,33	341,700	Top EZSTORM
1,26	345,72	341,636	
1,19	328,10	341,572	
1,13	310,48	341,508	
1,06	292,87	341,444	
1,00	275,25	341,380	
0,94	257,64	341,316	
0,87	240,02	341,252	
0,81	222,40	341,188	
0,74	204,79	341,124	
0,68	187,17	341,060	
0,62	169,55	340,996	
0,55	151,94	340,932	
0,49	134,32	340,868	
0,42	116,71	340,804	
0,36	99,09	340,740	
0,30	81,47	340,676	
0,23	63,86	340,612	
0,17	46,24	340,548	
0,10	28,63	340,484	
0,04	11,01	340,420	Invert
0,00	0,00	340,380	Bottom EZSTORM



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VOLUME CALCULATION

BASIN-EZSTORM™-363.33M³

PROJECT NAME:
81 ROYAL ROAD GUELPH
MANUFACTURING, ON

PROJECT N°: 240815-06	DATE: 16/08/2024
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DRAWN BY: S.M.	CHECKED BY: S.K.
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SCALE: SCALE	SHEET N°: 6/6
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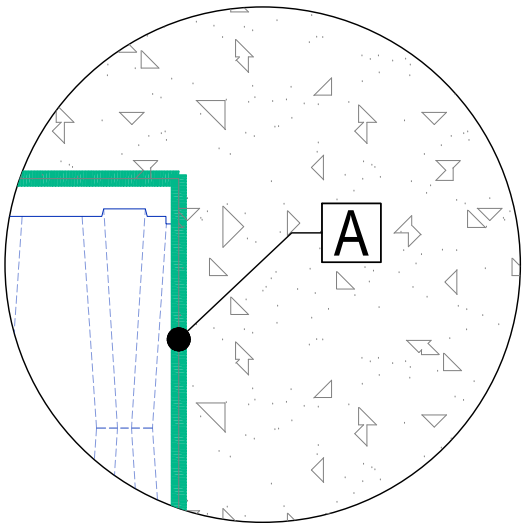
A

B

C

D

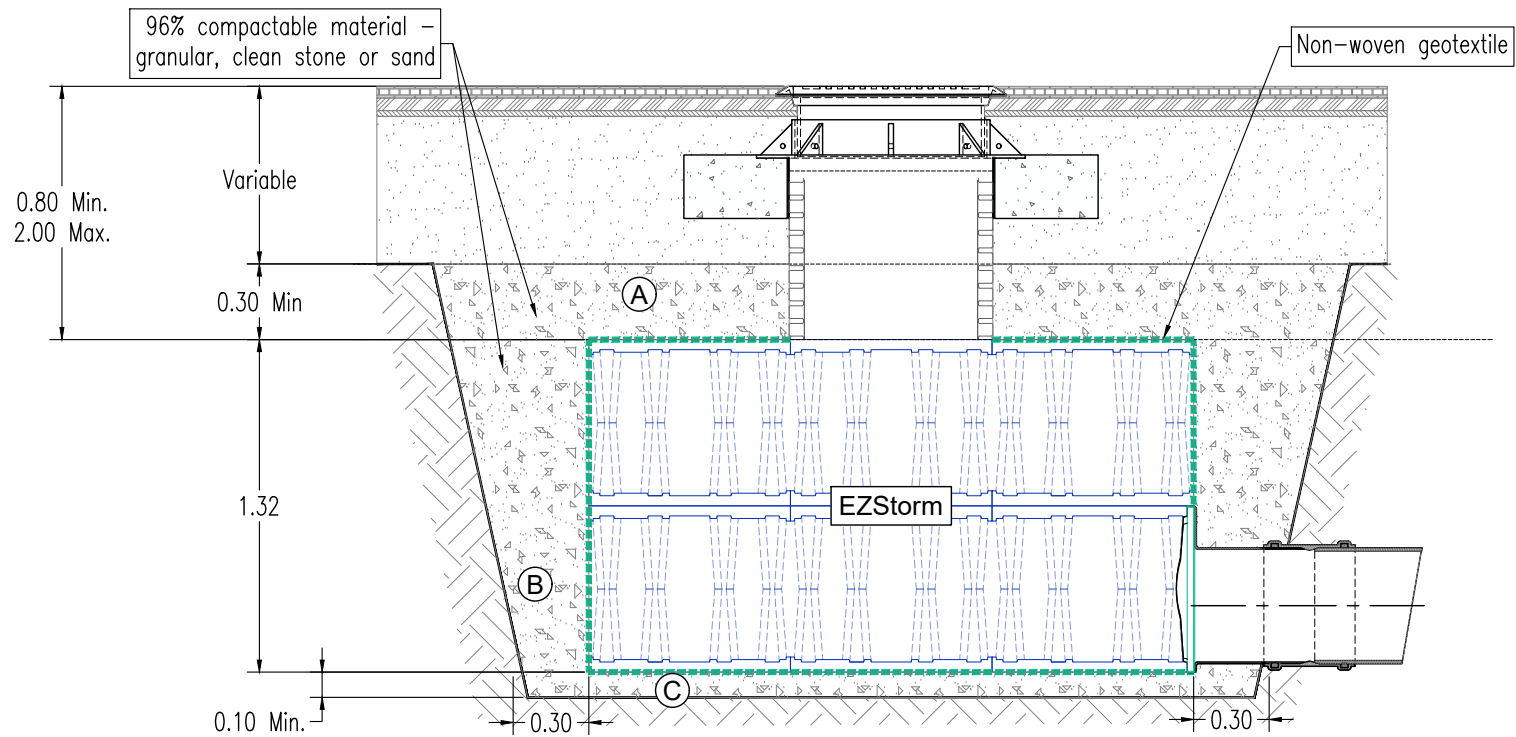
Acceptable backfill materials for this project			
Live Load: CL-625 (CSA-S6: 19)			
	Layer location	Backfill material	Density requirements
A	Top embankment: Embankment located directly above the EZSTORM™ chambers abd below the road structure.	Backfill with a 20 mm Max. granular material compacted at a rate > 95 % S.P.D. (3/4 (20mm) granular material, clean stone or sand)	Without driving over the structure, place a first layer of 450mm thick on top then compact to 90% M.P. using light equipment not exceeding 5.000 kg and always driving in the same direction. Then add layers up to 300 mm thick using the same equipment and always in the same direction. Normal traffic is only permitted once the final backfill height has been reached.
B	Lateral backfill: Located between the lateral faces of the EZSTORM™s and the limits of the excavated volume.	Frost-resistant granular earthwork material with a maximum grain diameter of 20 mm per 300 mm layer and compacted at a rate > 96% M.P.	Spread the backfill material with a hydraulic shovel or loader, then compact it with a compactor or vibratory plate to 90% M.P. in successive layers up to 300 mm thick, over the full width.
C	Laying bed: located under the EZSTORM™ blocks, between the foundation floor and the base of the blocks.	Subgrade granular material 100 mm Min. 3/4 (20mm) granular material, clean stone or sand to 96% M.P.	Compact to 90% M.P. using a vibrating plate or roller compactor. Place the system on a flat, solid, horizontal and stable surface.



303

DETAIL

SCALE NOT AT SCALE



103

TYPICAL SECTION

SCALE 1:30

LEGEND

Inspection Chimney

Catch basin

Connection-acces concrete manhole

Manhole

A Geotextile EZ-226.

B Geomembrane EZ-LLDPE-30

Drain PEHD 1500 (By others)

Clean Stone with maximum grain size of 20mm at 40% void ratio or MG-20 or MG-112 compacted sand at 90%PM

Thickness of this layer may vary according to project requirements

C Paving bed

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STANDARD BACKFILL REQUIREMENTS

BASIN-EZSTORM™-363.33M³

PROJECT NAME:
81 ROYAL ROAD GUELPH
MANUFACTURING, ON

PROJECT N°:
240815-06

DATE:
16/08/2024

DRAWN BY:
S.M.

CHECKED BY:
S.K.

SCALE:
SCALE

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List of materials		
CODE DE L'ARTICLE	DESCRIPTION	B1
EZ-SHD	EZStorm - half block 2 units/block (units)	1792
FL-EZSHD	EZSTORM Sidewall grid (units)	176
FL-EZSHD 1/2	EZSTORM Sidewall grid for half block (units)	0
PR-EZSHD	EZSTORM Cover plate	0
CONNECTEUR EZS-1	EZSTROM Single layer-connector (units)	0
CONNECTEUR EZS-2	EZSTROM Multi layer-connector (units)	1000
R-P	EZSTORM Pre-treatment row (0.8m / unit)	120
EZSTORM adapters		
FC-200mm-PVC	EZSTORM Adapter 200 mm PVC (unités)	0
FC-250mm-PVC	EZSTORM Adapter 250 mm PVC (unités)	0
FC-300mm-PVC	EZSTORM Adapter 300 mm PVC (unités)	3
FC-375mm-PVC	EZSTORM Adapter 375 mm PVC (unités)	0
FC-450mm-PVC	EZSTORM Adapter 450 mm PVC (unités)	1
FC-450mm-TBA	EZSTORM Adapter 450 mm PCP (unités)	0
FC-525mm-PVC	EZSTORM Adapter 525 mm PVC (unités)	0
FC-600mm-PEHD	EZSTORM Adapter 600 mm HDPE (unités)	0
Inspection Chimney		
EZSTORM-ACCES	EZSTORM half-elements with opening (units)	6
PP-EZSTORM	EZSTORM half-elements with positioning plate (units)	3
PP-EZSTORM 1/2	EZSTORM Cover plate with positioning plate (units)	0
REHAUSSE-PEHD-600	EZSTORM Extension Pipe - Chimney (units) - Ø 600mm - 1.5 m /unit	3
Dalle-répartition	EZSTORM Support concrete ring (units)	3
OPSD401.01ST	Cast iron frame and cover (unit)	3
OPSD401.01ST	Catch bassin Frame and grates (unités)	0
Rectangulare concrete inspection manhole 1200mm x 1200mm		
R1212	EZSTORM regtangular inspection concrete manhole	0
EZ-225	EZSTORM Protection geotextile (226g/m2) - Rolls of 6 m x 100 m	2
EZ-450	EZSTORM Protection geotextile (450g/m2) -Rolls of 6 m x 100 m	0
EZ-LLDPE30	LLDPE 30 mils liner (m2)	0
Clear Stone (by others)		
	Quantity of 20 mm (3/4') clear stone required (m3) (by others)	0

LEGEND

- ACCESSORIES not included in all projects
- Drawings for guidance only. For more details please refer to the DETAILES project plans

B	ISSUED FOR APPROVAL	02/12/2024	S.M.
A	ISSUED FOR APPROVAL	16/08/2024	S.M.
N°.	REVISION	DATE	BY


ISSUED FOR APPROVAL
NOT FOR PRODUCTION

LIST OF MATERIALS

BASIN-EZSTORM™-363.33M³

PROJECT NAME:
81 ROYAL ROAD GUELPH
MANUFACTURING, ON

PROJECT N°: 240815-06	DATE: 16/08/2024
DRAWN BY: S.M.	CHECKED BY: S.K.
SCALE: SCALE	SHEET N°: 6/6



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Salaberry-de-Valleyfield J6S 1C2, Québec

81 Royal Rd

DRAWING INDEX

COVER SHEET
SIZING REPORT
IMPLANTATION
SDD3 1800Ø

SHEET No

.....1 of 4
.....2 of 4
.....3 of 4
.....4 of 4

CONTACT INFORMATION

SITE CONTACT	Parth Pushkarna	647-278-7339	ppushkarna@brunet.cc
ENGINEER / TECHNICAL SPECIALIST	Andres Velez	819-696-7171	asanchez@nextstorm.ca
SALES REP:	Parth Pushkarna	647-278-7339	ppushkarna@brunet.cc

NOTES FOR BIDDING AND INSTALLATIONS

- CONTRACTORS ARE EXPECTED TO COMPREHEND AND USE THE MOST CURRENT INSTALLATION INSTRUCTIONS PRIOR TO BEGINNING A SYSTEM INSTALLATION. FOR THE MOST CURRENT INSTRUCTIONS, CONTACT NEXTSTORM AT LEAST TWO WEEKS PRIOR TO SYSTEM INSTALLATION TO ARRANGE FOR A PRE-CONSTRUCTION MEETING.
- USE SDD3 INSTALLATION INSTRUCTIONS AS A GUIDELINE ONLY FOR MINIMUM/MAXIMUM REQUIREMENTS. ACTUAL DESIGN MAY VARY. REFER TO APPROVED CONSTRUCTION DRAWINGS FOR JOB-SPECIFIC DETAILS. ENGINEERING DRAWINGS SUPERSEDE ALL PROVIDED DOCUMENTATION.
- ANY DISCREPANCIES WITH THE SYSTEM SUB-GRADE SOIL'S BEARING CAPACITY MUST BE REPORTED TO THE GEOTECHNICAL ENGINEER.
- EROSION AND SEDIMENT-CONTROL MEASURES MUST MEET LOCAL CODES AND THE DESIGN ENGINEER'S SPECIFICATIONS THROUGHOUT THE ENTIRE SITE CONSTRUCTION PROCESS.

NOTE: THESE SHOP DRAWINGS MAY CONTAIN COMPONENTS INCLUDING BUT NOT LIMITED TO MANHOLES, CATCH BASINS, STORM PIPES AND FITTINGS, MANIFOLDS, CASTINGS AND OTHER NECESSARY APPURTENANCES THAT MAY NOT BE SUPPLIED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR AND/OR SUPPLIER TO CONFIRM THE MATERIALS PROVIDED.

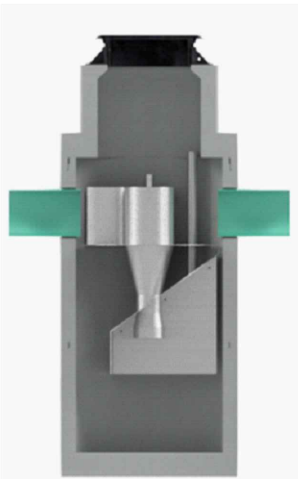
GENERAL NOTES

- MANHOLES AND APPURTENANCES TO FOLLOW DIVISION 700 OF OPSD OR APPLICABLE LOCAL STANDARDS.
- COORDINATE WITH MANUFACTURER'S REPRESENTATIVE/DISTRIBUTOR FOR PRE-CONSTRUCTION MEETING AND SITE INSPECTION DURING INSTALLATION.
- ENGINEERING DRAWINGS SUPERSEDE ALL PROVIDED DOCUMENTATION. REFER TO SITE ENGINEERS FOR ADDITIONAL INSTRUCTIONS.
- COORDINATE SDD3 INSTALLATION ACTIVITIES WITH OTHER SITE ACTIVITIES
- ALL DIMENSIONS ARE IN MILLIMETERS UNLESS NOTED OTHERWISE
- THE SUB-GRADE AND SIDE BACKFILL TO BE COMPACTED TO 95% SPD OR AS DIRECTED BY THE QUALIFIED ENGINEER.
- CONFIRM GEOTECHNICAL SOIL EVALUATION BY A QUALIFIED ENGINEER TO DETERMINE SUITABILITY OF STRUCTURAL INSTALLATION
- CONFIRM FOR BURIED UNDERGROUND UTILITIES INCLUDING GAS, ELECTRICAL, PIPELINES OR CONDUITS
- WHEN INSTALLED IN CONFORMANCE TO THE INSTALLATION GUIDELINES, SDD3 CAN HANDLE STANDARD CL-625 TRUCK LOADING. FOR NON-STANDARD LOADS CONTACT MANUFACTURER'S REPRESENTATIVE/DISTRIBUTOR
- PROTECT THE INSTALLATION AGAINST DAMAGE WITH CONSTRUCTION TAPE, FENCING OR OTHER MEANS TILL THE CONSTRUCTION IS COMPLETE.
- ENSURE THAT CONSTRUCTION FOLLOWS APPLICABLE FEDERAL, PROVINCIAL, LOCAL, MUNICIPAL AND LOCAL LAWS, ORDNANCES, REGULATIONS AND SAFETY REQUIREMENTS.

CHECK - REQUIRED MATERIALS AND EQUIPMENT

- ALL SDD3 COMPONENTS AND ACCESSORIES AS SPECIFIED IN THE ENGINEER'S PLANS INCLUDING FRAME AND COVER, LADDER AND RISER .
- RECIPROCATING SAW OR ROUTER
- TRANSIT OR LASER LEVEL MEASURING DEVICE
- COMPACTION EQUIPMENT
- ACCEPTABLE FILL MATERIAL AS SHOWN IN INSTALLATION INSTRUCTIONS.

THIS DRAWING WAS PREPARED TO SUPPORT THE PROJECT ENGINEER OF RECORD FOR THE PROPOSED SYSTEM. IT IS THE ULTIMATE RESPONSIBILITY OF THE PROJECT ENGINEER OF RECORD TO ENSURE THAT THE SDD3 SYSTEM'S DESIGN IS IN FULL COMPLIANCE WITH ALL APPLICABLE LAWS AND REGULATIONS. DOES NOT APPROVE PLANS, SIZING, OR SYSTEM DESIGNS.



A	ISSUED FOR APPROVAL	02/12/2024	M.C.
N°.	RÉVISION	DATE	PAR

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COVER PAGE

SDD3-1800

PROJECT:
81 Royal Rd

PROJECT N°: 240815-06 DATE: 02/12/2024

DRAWN BY: M.C. CHECKED BY: M.S.

SCALE: SCALE SHEET N°: 1/4



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SDD3 SIZING REPORT

2024-11-29

PROJECT INFORMATION	
Project Name	81 Royal Rd
Location	Guelph
Province	Ontario
Unit ID	MH 9 OGS
Project Number	240815-06

SITE INFORMATION	
Site Area (hectares)	1.08
Runoff Coefficient	1
SIZING CRITERIA	
Rainfall Station	WATERLOO WELLINGTON AP
Target TSS removal (%)	80
Particle Size Distribution	ETV (20-1000µm)
Peak or Regulated Flow (L/s)	

STORMWATER TREATEMENT RECOMMENDATION

RESULTS SUMMARY		
Model	TSS (%)	Volume(%)
SDD3-900	71	99
SDD3-1200	75	100
SDD3-1500	79	100
SDD3-1600	79	100
SDD3-1800	81	100
SDD3-2400	85	100
SDD3-3000	87	100
SDD3-3200	87	100
SDD3-3600	88	100
SDD3-4000	88	100

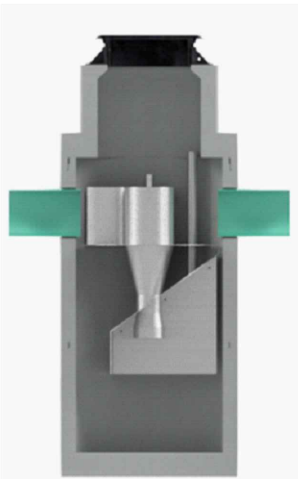
Recommended Model	SDD3 1800
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Annual TSS removal efficiency (%) ¹	Manhole Diameter (mm)	Maximum Pipe Diameter (mm)	Oil Storage Capacity (m3)	Sediment Storage Capacity (m3)	Height from invert to SDD floor (m)	Treatment area (m2)	Max treatment flow (lps)
81	1830	900	0.98	3.66	2.36	2.63	114

DETAILED SDD3 SIZING REPORT

DETAILED SDD3 SIZING REPORT							
Rainfall Intensity (mm / hr)	Total Rainfall (%)	Cumulative rainfall volume (%)	Flow Rate (Lps)	Surface Loading Rate (L/s/m2)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
1.0	20.00	20.0	1	0.4	88.9	17.8	18
1.5	7.90	27.9	2	0.6	88.9	7.0	25
2.0	7.90	35.8	2	0.8	87.8	6.9	32
2.5	5.60	41.4	3	1.0	86.4	4.8	37
3.0	5.60	47.0	3	1.2	85.0	4.8	41
3.5	4.35	51.4	4	1.4	83.9	3.6	45
4.0	4.35	55.7	4	1.6	83.0	3.6	49
4.5	3.80	59.5	5	1.8	82.1	3.1	52
5.0	3.80	63.3	5	2.1	81.2	3.1	55
5.5	3.20	66.5	6	2.3	80.4	2.6	57
6.0	3.20	69.7	6	2.5	79.5	2.5	60
6.5	2.05	71.8	7	2.7	78.6	1.6	62
7.0	2.05	73.8	8	2.9	77.7	1.6	63
7.5	1.40	75.2	8	3.1	76.8	1.1	64
8.0	1.40	76.6	9	3.3	76.0	1.1	65
8.5	1.35	78.0	9	3.5	75.1	1.0	66
9.0	1.35	79.3	10	3.7	74.3	1.0	67
9.5	1.20	80.5	10	3.9	73.5	0.9	68
10.0	1.20	81.7	11	4.1	72.6	0.9	69
10.5	1.35	83.1	11	4.3	71.8	1.0	70
11.0	1.35	84.4	12	4.5	71.0	1.0	71
11.5	1.10	85.5	12	4.7	70.2	0.8	72
12.0	1.10	86.6	13	4.9	69.3	0.8	72
12.5	0.90	87.5	14	5.1	68.5	0.6	73
13.0	0.90	88.4	14	5.3	67.7	0.6	74
13.5	0.50	88.9	15	5.5	66.9	0.3	74
14.0	0.50	89.4	15	5.7	66.0	0.3	74
14.5	0.85	90.3	16	6.0	65.2	0.6	75
15.0	0.85	91.1	16	6.2	64.4	0.5	75
15.5	0.60	91.7	17	6.4	63.6	0.4	76
16.0	0.60	92.3	17	6.6	62.7	0.4	76
16.5	0.55	92.9	18	6.8	62.5	0.3	77
17.0	0.55	93.4	18	7.0	62.7	0.3	77
17.5	0.30	93.7	19	7.2	62.9	0.2	77
18.0	0.30	94.0	19	7.4	63.1	0.2	77
18.5	0.15	94.2	20	7.6	63.3	0.1	77
19.0	0.15	94.3	21	7.8	63.5	0.1	78
19.5	0.35	94.7	21	8.0	63.7	0.2	78
20.0	0.35	95.0	22	8.2	63.9	0.2	78
20.5	0.35	95.4	22	8.4	64.1	0.2	78
21.0	0.35	95.7	23	8.6	64.4	0.2	78
21.5	0.15	95.9	23	8.8	64.6	0.1	78
22.0	0.15	96.0	24	9.0	64.8	0.1	79
22.5	0.45	96.5	24	9.2	65.0	0.3	79
23.0	0.45	96.9	25	9.4	65.2	0.3	79
23.5	0.25	97.2	25	9.6	65.4	0.2	79
24.0	0.25	97.4	26	9.9	65.6	0.2	80
24.5	0.10	97.5	26	10.1	65.8	0.1	80
25.0	0.10	97.6	27	10.3	65.8	0.1	80
27.5	0.45	98.1	30	11.3	65.8	0.3	80
30.0	0.45	98.5	32	12.3	65.8	0.3	80
32.5	0.45	99.0	35	13.3	65.8	0.3	81
35.0	0.45	99.4	38	14.4	65.9	0.3	81
37.5	0.15	99.6	41	15.4	65.9	0.1	81
40.0	0.15	99.7	43	16.4	65.9	0.1	81
42.5	0.15	99.9	46	17.5	65.5	0.1	81
45.0	0.15	100.0	49	18.5	64.9	0.1	81
47.5	0.00	100.0	51	19.5	64.3	0.0	81
50.0	0.00	100.0	54	20.5	63.7	0.0	81
Total cumulative rainfall (%)		100	Net Annual TSS removal (%)		81		

1 Performance efficiency based on ETV (20 1000µm) particle size distribution



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SIZING REPORT

SDD3-1800

PROJECT:
81 Royal Rd

PROJECT N°: 240815-06 DATE: 02/12/2024

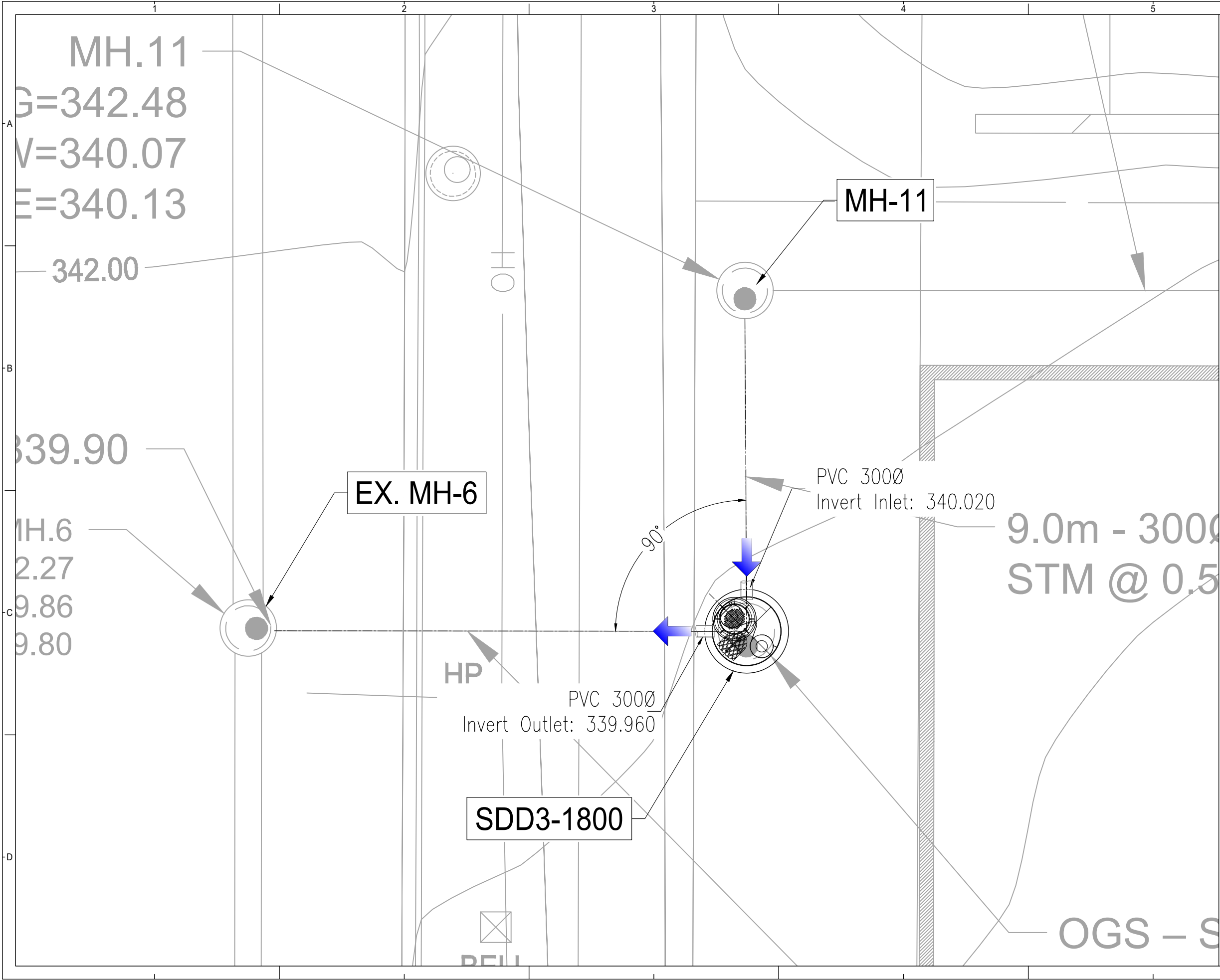
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IMPLANTATION

SDD3-1800

PROJECT:
81 Royal Rd

PROJECT N°: 240815-06
DATE: 02/12/2024

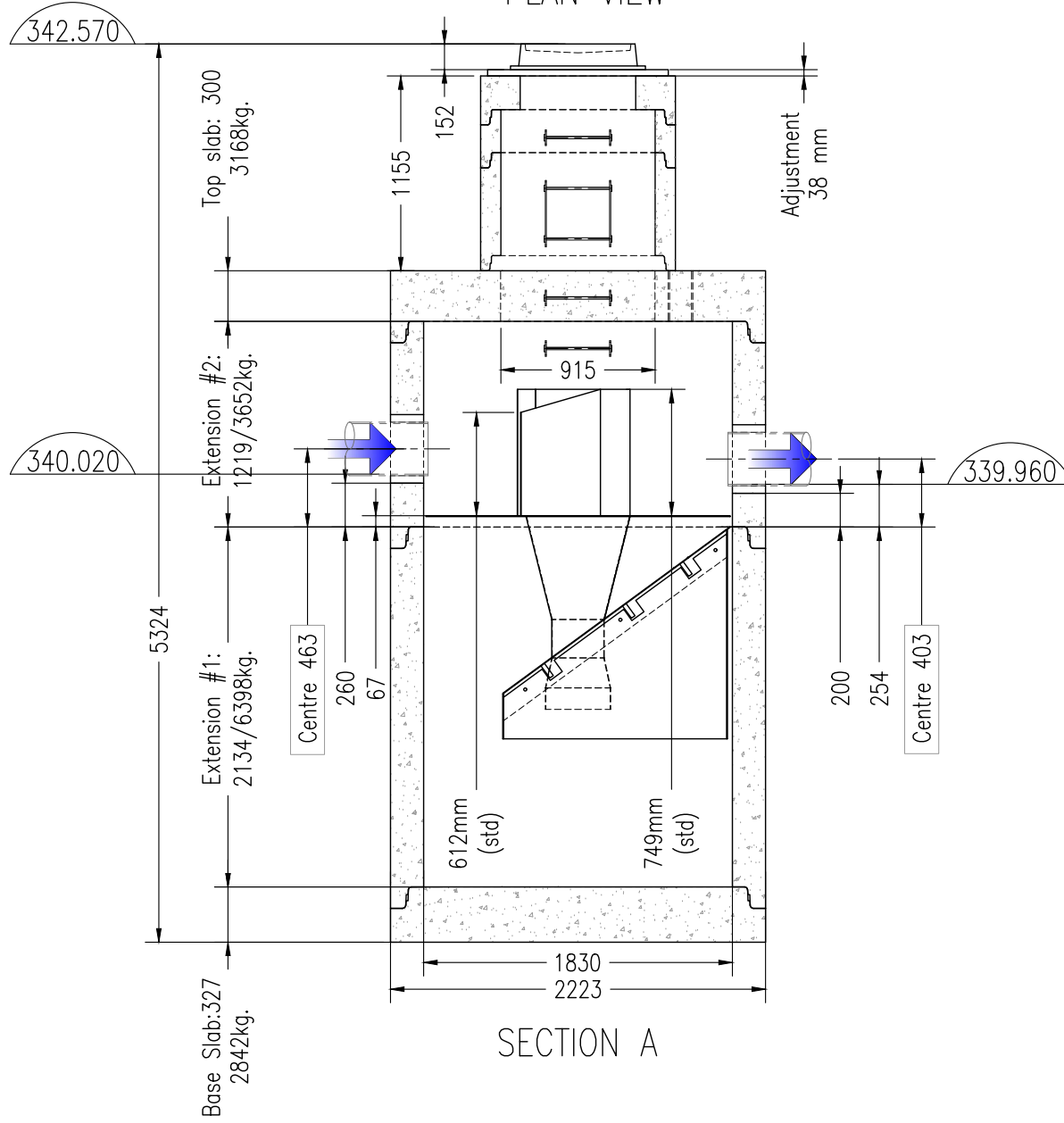
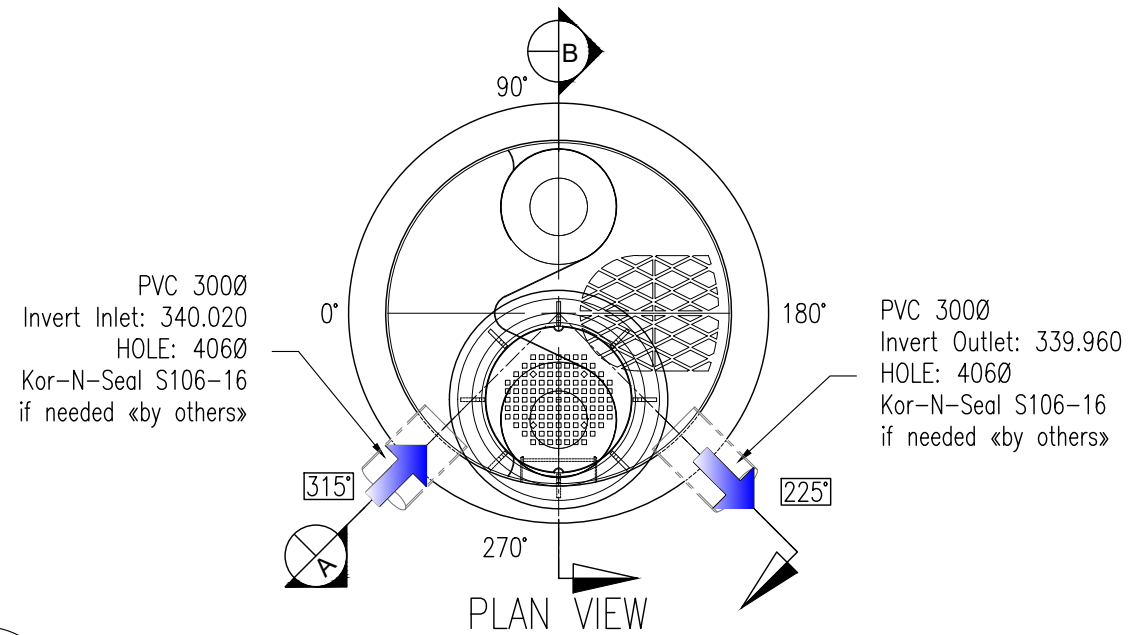
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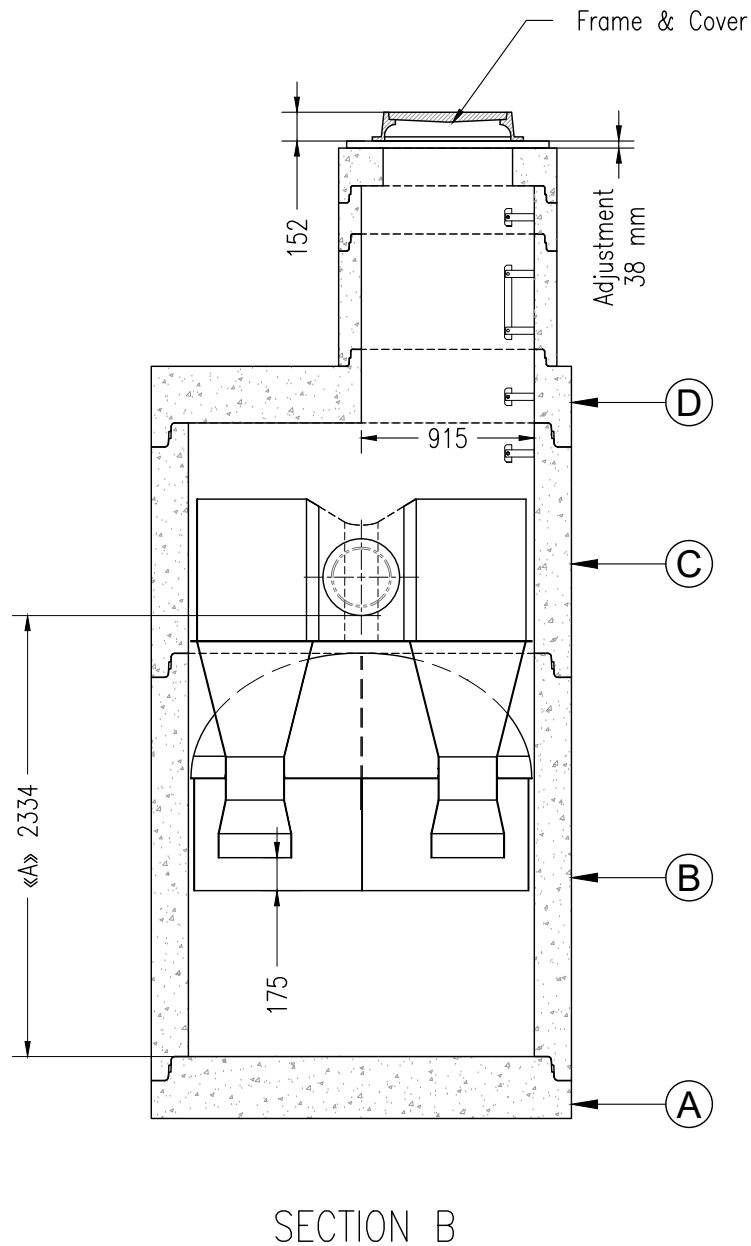


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J6S 1C2, Québec



BILL OF MATERIALS					
✓	QTY	PIECE	DESCRIPTION	MATERIAL	WEIGHT (Kg)
	1	A	Base 1800Ø	Concrete	2842
	1	B	Extension #1	Concrete	6398
	1	C	Extension #2	Concrete	3652
	1	D	Top slab 1800Ø	Concrete	3168
	1	-	Top slab-ON 200mm	Concrete	547
	Var.		Chimney elements	Concrete	
	1		Frame and cover 30-1/2"Ø	Iron	
	Var.		Gasket		



- NOTES:
- PRECAST CONCRETE COMPONENTS SHALL BE ACCORDING TO CSA A257.4
 - ALL REINFORCING STEEL HAS 30mm MIN. COVER
 - LADDER RUNGS
 - ALL DIMENSIONS ARE NOMINAL
 - ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE SHOWN.
- FLOW AND POLLUTANTS
- 100% TSS VOLUME STORAGE 2.659m³
 - EFFECT ON PIEZOMETRIC LINE: NIL
 - IN-LINE
- MEASUREMENT MAINTENANCE
- TOTAL HEIGHT (A): 2.334 m

Model MB – Standard

A	ISSUED FOR APPROVAL	02/12/2024	M.C.
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OIL GRIT SEPARATOR

SDD3-1800

PROJECT:
81 Royal Rd

PROJECT N°: 240815-06 DATE: 02/12/2024

DRAWN BY: M.C. CHECKED BY: M.S.

SCALE: 1:40 SHEET N°: 4/4

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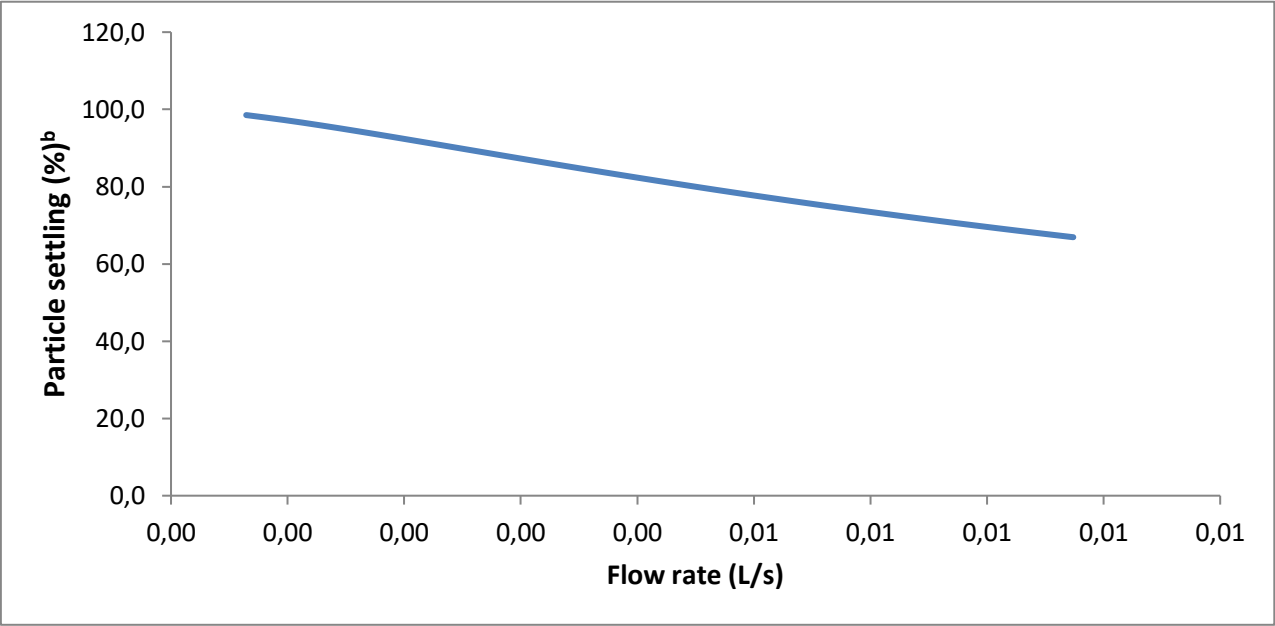
EZStorm Treatment Row Performance Report

PROJECT INFORMATION

Project Name: 81 ROYAL ROAD GUELPH MANUFACTURING
Location: GUELPH, ON
Comments:

Treatment Row

Length	48,0 m
Width	0,8 m
Total Area (m2)	38,4 m2
Max. Volume (m)	11,5 m3
TSS annual removal (%) ^a	78 %



Estimation of Treatment Train Performance

Treatment Train Component	Sediment Removal
SDD3 TSS Removal	80 %
Treatment Row trapping capacity	78 %
Weighted Treatment Train Performance	96 %

Notes:

^a Sediment trapping and detention could differ depending on detention time Treatment Row configuration.

Weighted annual sediment removal based on the distribution and intensity of local rainfall for 90% of rainfall volume.

^b Theoretical sediment trapping capacity estimated and based on the sedimentation area and shape of the treatment row, particle settling velocity and flow rate.

Vertical settling velocity in accordance with Stokes' law for particles over 20um .

No treatment is provided over the Treatment Row maximum volume (by-pass)

Performance shown in figure 1 do not take in account turbulence, scour or resuspension.

C.4. Water Balance Analysis

The following is a summary of calculations using Table 3.1 in the Stormwater Management Planning and Design Manual (MOE, 2003) for the identification of the hydrologic breakdown

Table 3.1 identifies the approximate relationship between soil types, ground cover, and hydrologic components.

The table utilizes a base precipitation of 940mm. The ratios have been retained for a base of 844mm of precipitation to match the Guelph average annual precipitation (Table 4.1, City of Guelph Stormwater Management Master Plan, dated December 2022).

Values Calculated using Table 3.1

Soil Type		Urban Lawn - Silt Loam			
Soil Group	Water Holding Capacity (mm)	Precipitation (mm)	Evapotranspiration (mm)	Runoff (mm)	Infiltration* (mm)
C	125	844	536	217	91

* Infiltration factor has been determined using the following values from Table 3.1

Topography	0.2	Flat Land
Soils	0.2	Medium combinations of clay and loam
Cover	0.1	Cultivated Land
Total Infiltration Factor:	0.5	

81 Royal Road
City of Guelph
Existing Conditions Site Water Balance

EXISTING CONDITION

Contributing Catchments:100

Soil Type: Silt Loam
Vegetation: Shallow Rooted
Root Zone Depth =0.62m
Soil Moisture Retention Capacity (mm)125

Runoff Factor =0.70

Contributing Area =2.03 ha
Percent Impervious =0.0%

Evapotranspiration
Factor for Impervious
Surfaces =0.34

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily Potential Evapotranspiration	Correction Factors	Adjusted Potential Evapotranspiration	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual Evapotranspiration	Pervious ET - Actual ET	Moisture Deficit	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Actual Runoff	Recharge Volume	Runoff Volume
	(°C)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m³)	(m³)
Jan	-7.4	0.0	0.0	24.3	0.0	57.7	57.7		249.5	0.0	0.0	0.0	0.0	0.0	0.0	4.0	0.0	4.0	2.8	24	57
Feb	-6.5	0.0	0.0	24.6	0.0	50.9	50.9		300.4	0.0	0.0	0.0	0.0	0.0	0.0	2.2	0.0	2.2	1.5	13	31
Mar	-1.7	0.0	0.0	30.6	0.0	61.8	61.8		362.2	0.0	0.0	0.0	0.0	0.0	0.0	1.1	0.0	1.1	0.8	7	16
Apr	5.6	1.2	0.9	33.6	30.2	73.4	43.2		125.0	0.0	30.2	30.2	0.0	0.0	43.2	21.6	23.7	45.3	31.7	276	644
May	12.0	3.8	1.9	37.8	71.8	75.0	3.2	-32.3	125.0	0.0	71.8	71.8	0.0	0.0	3.2	12.4	106.7	119.1	83.4	725	1,693
Jun	17.1	6.4	2.8	38.4	107.5	75.2	-32.3	-79.6	96.0	-29.0	104.2	104.2	0.0	3.3	0.0	6.2	53.4	59.6	41.7	363	846
Jul	19.6	7.9	3.3	38.7	127.7	80.4	-47.3	-108.9	65.0	-31.0	111.4	111.4	0.0	16.3	0.0	3.1	26.7	29.8	20.8	181	423
Aug	18.7	7.4	3.1	36.0	109.8	80.5	-29.3		51.0	-14.0	94.5	94.5	0.0	15.3	0.0	1.5	13.3	14.9	10.4	91	212
Sep	14.6	5.1	2.4	31.2	74.9	75.4	0.5		51.5	0.5	74.9	74.9	0.0	0.0	0.0	0.8	6.7	7.4	5.2	45	106
Oct	8.5	2.2	1.4	28.5	39.9	71.0	31.1		82.6	31.1	39.9	39.9	0.0	0.0	0.0	0.4	4.2	4.6	3.2	28	65
Nov	2.5	0.4	0.4	24.3	9.7	76.2	66.5		125.0	42.4	9.7	9.7	0.0	0.0	24.1	12.2	2.5	14.7	10.3	90	210
Dec	-3.7	0.0	0.0	23.1	0.0	66.8	66.8		191.8	0.0	0.0	0.0	0.0	0.0	0.0	6.1	0.0	6.1	4.3	37	87
Total		34.3				844.3	272.7				536.7	536.7	0.0	34.9	70.4	71.6	237.2	308.8	216.2	1880.9	4388.7

Notes: Precipitation and Temperature data from City of Guelph Stormwater Management Master Plan (dated December 2022)
Monthly water balance strategy as outlined in the document*Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance (Thornthwaite and Mather, 1957)*
Monthly Heat Index (I) from Table 2 of*Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*
Correction Factors from Table 6 of*Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*
Evaporation Factor for Impervious Surfaces = Average Annual Evapotranspiration for Impervious Surfaces (183mm/year) / Average Annual Evapotranspiration for Pervious Surfaces (536.7mm/year) = 0.34
Runoff Factor = [(Impervious Percentage of Site x Average Annual Runoff for Impervious Surfaces) + (Pervious Silt Till Percentage of Site x Average Annual Runoff for Pervious Silt Till Surfaces)] / Total Annual Recharge & Runoff

81 Royal Road
City of Guelph

Post-Development Site Monthly Water Balance

POST-DEVELOPMENT CONDITIONS

Contributing Catchments:200 - 202, 300

Contributing Area =2.03ha

Percent Impervious =52.0%

Soil Type: Silt Loam

Vegetation: Shallow Rooted

Root Zone Depth =0.62m

Soil Moisture Retention Capacity125

Runoff Factor =0.86

Evapotranspiration Factor for Impervious Surfaces =0.34

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily Potential Evapotranspiration	Correction Factors	Adjusted Potential Evapotranspiration	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual Evapotranspiration	Pervious ET - Actual ET	Moisture Deficit	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Actual Runoff	Recharge Volume	Runoff Volume
	(°C)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m ³)	(m ³)
Jan	-7.4	0.0	0.0	24.3	0.0	57.7	57.7		249.5	0.0	0.0	0.0	0.0	0.0	0.0	6.1	0.0	6.1	5.3	18	107
Feb	-6.5	0.0	0.0	24.6	0.0	50.9	50.9		300.4	0.0	0.0	0.0	0.0	0.0	0.0	5.1	0.0	5.1	4.4	15	89
Mar	-1.7	0.0	0.0	30.6	0.0	61.8	61.8		362.2	0.0	0.0	0.0	0.0	0.0	0.0	3.0	0.0	3.0	2.6	9	52
Apr	5.6	1.2	0.9	33.6	30.2	73.4	43.2		125.0	0.0	30.2	19.9	10.4	10.4	53.5	26.8	23.7	50.5	43.2	148	877
May	12.0	3.8	1.9	37.8	71.8	75.0	3.2		125.0	0.0	71.8	47.2	24.6	24.6	27.8	27.3	106.7	134.0	114.7	392	2,329
Jun	17.1	6.4	2.8	38.4	107.5	75.2	-32.3	-32.3	96.0	-29.0	104.2	68.5	35.7	39.0	35.7	31.5	53.4	84.9	72.6	248	1,475
Jul	19.6	7.9	3.3	38.7	127.7	80.4	-47.3	-79.6	65.0	-31.0	111.4	73.2	38.2	54.5	38.2	34.8	26.7	61.5	52.7	180	1,069
Aug	18.7	7.4	3.1	36.0	109.8	80.5	-29.3	-108.9	51.0	-14.0	94.5	62.1	32.4	47.7	32.4	33.6	13.3	47.0	40.2	137	816
Sep	14.6	5.1	2.4	31.2	74.9	75.4	0.5		51.5	0.5	74.9	49.2	25.7	25.7	25.7	29.6	6.7	36.3	31.1	106	631
Oct	8.5	2.2	1.4	28.5	39.9	71.0	31.1		82.6	31.1	39.9	26.2	13.7	13.7	13.7	21.7	4.2	25.9	22.1	76	449
Nov	2.5	0.4	0.4	24.3	9.7	76.2	66.5		125.0	42.4	9.7	6.4	3.3	3.3	27.4	24.5	2.5	27.0	23.1	79	470
Dec	-3.7	0.0	0.0	23.1	0.0	66.8	66.8		191.8	0.0	0.0	0.0	0.0	0.0	0.0	12.3	0.0	12.3	10.5	36	213
Total		34.3				844.3	272.7				536.7	352.8	183.9	218.8	254.3	256.3	237.2	493.5	422.5	1,442.7	8,576.1

Notes: Precipitation and Temperature data from City of Guelph Stormwater Management Master Plan (dated December 2022)

Monthly water balance strategy as outlined in the document*Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance (Thornthwaite and Mather, 1957)*

Monthly Heat Index (I) from Table 2 of*Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*

Correction Factors from Table 6 of*Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*

Evaporation Factor for Impervious Surfaces = Average Annual Evapotranspiration for Impervious Surfaces (183mm/year) / Average Annual Evapotranspiration for Pervious Surfaces (536.7mm/year) = 0.34

Runoff Factor = [(Impervious Percentage of Site x Average Annual Runoff for Impervious Surfaces) + (Pervious Silt Till Percentage of Site x Average Annual Runoff for Pervious Silt Till Surfaces)] / Total Annual Recharge & Runoff