



File: 2372H-1v2

Date: April 28, 2026

Attn: Mr. Sayf Hassan, CEO
EM Guelph Developments Limited Partnership
6 Adelaide Street East, Suite 300
Toronto, Ontario
M5C 1H6

Re: 'Scoped' Hydrogeological Assessment
Proposed Residential Development
210 – 222 College Avenue East, Guelph, Ontario

Project Overview

CHUNG & VANDER DOELEN ENGINEERING LTD. (CVD) was retained by EM Guelph Developments Limited Partnership ("Client") to complete a 'scoped' hydrogeological investigation involving a seasonal monitoring program of groundwater level fluctuations at the subject lands located at 210 – 222 College Avenue East in Guelph, Ontario (hereinafter referred to as the "Site"). The purpose of this investigation is to assess the seasonal high groundwater elevation at the Site to support the proposed construction of a 10-storey residential development.

It is understood that the structure will have a finished floor elevation (FFE) of 344.25 mASL, as shown in the Site Grading Plan (5468796 Architecture Inc., December 18, 2025) included in Appendix B. The building will also have one (1) level of underground parking with a basement finished floor elevation (BFFE) of 337.58 mASL.

This report presents a complete summary of the findings gathered from April 2025 to April 2026 as part of the CVD combined hydrogeological and geotechnical Investigation. This technical memorandum should be read in conjunction with the CVD Geotechnical Investigation Report, Rev. 2, dated January 9, 2026.

Field Investigation

As part of the CVD geotechnical investigation, three (3) boreholes were advanced to a depth of 12.20 m below ground surface (mbgs) across the Site between April 9 and 10, 2025, as shown in the CVD Borehole Location Plan included in Appendix E. All three (3) were converted into monitoring wells (labelled BHs 1, 2, and 3). Each monitoring well was developed using Waterra™ polyethylene tubing, foot-valve and hand pumps.

Based on the results of the investigation, following the surficial topsoil / fill deposits, which extend to

between 0.16 (BH 1) and 1.45 (BH 3) mbgs, the Site is predominantly underlain by a sand and silt to sandy silt till deposit. This deposit extends to the maximum depth of exploration between 12.45 and 12.80 mbgs (lowest elevation down to 329.03 mASL).

Groundwater Level Monitoring Program

During the groundwater monitoring program, water level readings were manually taken on six (6) occasions between April 2025 and April 2026 at all three (3) monitoring wells, representing a full year of groundwater level readings. Additionally, two (2) rounds of spring high groundwater level readings were recorded in April 2025 and April 2026.

Van Essen DI801 10 m TD-Diver automated pressure transducer data loggers were installed in all three (3) monitoring wells on April 14, 2025, to provide continuous water level measurements at 2-hr intervals during the monitoring period. A barometric pressure data logger was also installed to compensate for atmospheric noise and pressure at the ground surface.

Table 1 in Appendix C summarizes the depths and elevations of the on-Site water table as well as the fluctuations in the water table compared to a baseline reading taken on April 14, 2025, which are representative of peak groundwater conditions. Additionally, a hydrograph, Figure 1, which visually presents the water table elevations shown in Table 1 and from data loggers, is also included in Appendix C.

Furthermore, three (3) single well response test (SWRT) analyses were performed at each monitoring well. Hydraulic conductivities for the monitoring wells were calculated from 1.6×10^{-7} m/s to 2.5×10^{-7} m/s, which corresponds with the values expected from a compact to very dense sand and silt till which underlies the entirety of the Site. The hydraulic conductivity values were calculated using the Bouwer-Rice Method through AquiferTest software, and the results of these tests are included in both Appendix D and summarized in Table 1.

Site Characterization

According to the surficial soil mapping for the area (Karrow, 1968), the soil underlying the area is primarily comprised of sandy till corresponding to the Wentworth Till unit. The Wentworth Till is part of the Guelph Drumlin field in which drumlins are aligned in a northwesterly / southeasterly direction. Lacustrine sand deposits are interspaced within the Wentworth Till, along with northwesterly-southeasterly aligned eskers, and occasional areas of outwash gravel along with sand and silt pond deposits in low lying areas. The underlying bedrock is comprised of the Guelph formation dolostone, located approximately $15 \pm$ to $25 \pm$ mbgs, based on nearby water well records.

In terms of nearby groundwater receptors, the Site is situated approximately $550 \pm$ m south of the Eramosa River, $1.2 \pm$ km southeast of the bifurcation point of the Speed and Eramosa Rivers, and $550 \pm$ m southeast of Arboretum Creek.



The property is located in the Grand River Conservation Authority (GRCA) wellhead protection area (WHPA) zone WHPA-B of the municipal wellfield located on Water Street and Edinburgh Road, approximately 1.7± to 1.8± km west of the Site. WHPA-B denotes an area in which horizontal groundwater travel time to the well is 2 years or less, and the WHPA vulnerability score of the property is 6.

Hydrogeological Assessment

Based on the results of manual groundwater level measurements recorded during the monitoring period, water level elevations across the Site and across all visits varied from 338.40 to 343.77 mASL, corresponding to depths between 0.47 and 4.20 mbgs. Peak groundwater elevations, based on both manual and automated measurements, ranged from 341.46 to 343.79 mASL across all monitoring well locations and Site visits, with the highest overall groundwater elevations occurring during April 2026. The overall variation between the individual wells, i.e., in comparison to each other during each monitoring event, varied by up to 3.34 m across all measurements. This large seasonal variation is due to the aquitard nature of the predominant till subsoils.

As shown in Figures 2 and 3 (included in Appendix C), the overall shallow groundwater flow direction travels in a north / northeastern direction, with an approximate water table gradient of 0.04± m/m across the Site.

Preliminary Dewatering Assessment

Considering that the lowest basement level is proposed to be founded at elevation 337.58 mASL, based on the highest recorded groundwater elevation of 343.77 mASL, the basement walls and floor slab need to be waterproofed, and the floor slab designed as a structurally reinforced element capable of resisting hydrostatic uplift pressures. Anchorage may be required depending on the final structural design and analysis. Elevator pits, sump pits, or other structural components located below the groundwater table will require similar waterproofing and structural design considerations.

Furthermore, in order to facilitate construction of the underground level in the dry, groundwater control will be required. Since the water table is located in a sand and silt till with very low hydraulic conductivity (2.5×10^{-7} m/s) and considering that the depth of excavation requiring drawdown will be in the range of only 5.5± to 6.0± m, it is expected that the volume of groundwater ingress into the excavation will be less than 50,000 L/day with a zone of influence (ZOI) of less than 50 metres. As such, an Environment and Sector Registry (EASR) for temporary construction dewatering is not anticipated to be required.

Temporary construction dewatering can be accomplished using traditional filtered sump pumping techniques.



Recommendations

Based on the findings of this scoped hydrogeological assessment, it is concluded that as long as the underground structure is properly waterproofed and reinforced, no issues should arise in terms of constructability, and that the total quantity of pumped groundwater for temporary construction dewatering needs will be less than 50,000 L/day with a zone of influence (ZOI) of less than 50 m. This ZOI is far less than the distance required to reach any nearby water bodies (at least 550± m) or the nearby municipal wellfield (at least 1.7± km), and, as such, no notable issues should arise during temporary construction dewatering.

Based on the completed one-year groundwater level monitoring program, two spring high-water events were recorded, with peak groundwater elevations ranging from 341.46 to 343.79 mASL. Given that groundwater levels in spring 2026 were abnormally high, CVD considers these elevations to be generally representative of the Site's seasonal high groundwater conditions, notwithstanding typical yearly variability. As such, no further groundwater level monitoring is considered necessary, as additional groundwater level readings are unlikely to alter the conclusions of this assessment.



Closing

This assessment has been prepared for the exclusive use of the Client and their assigns for specific application to this project property.

The assessment was conducted in accordance with the verbal and written requests from the Client, and generally accepted assessment practices. Performance of this assessment is intended to reduce, but not eliminate, uncertainty regarding the hydrogeological conditions encountered at the project site, given reasonable limits of time and cost. No other warranty, expressed or implied, is made.

We trust this report is sufficient for your immediate requirements. If you have any questions, please do not hesitate to contact the undersigned.

Respectfully submitted,

CHUNG & VANDER DOELEN ENGINEERING LTD.



Yaroslav Chudin, P.Eng.
Project Engineer & Hydrogeologist



Peter Dao, M.Sc., P.Geo.
Manager – Environmental & Hydrogeology



APPENDIX A

Limitations of Report



STATEMENT OF LIMITATIONS

1. The work performed in this report was carried out in accordance with the Standard Terms of Conditions made part of our contract. The conclusions presented herein are based solely upon the scope of services and time and budgetary limitations described in our contract.
2. The report has been prepared in accordance with generally accepted hydrogeological study and/or engineering practices. No other warranties, either expressed or implied, are made as to the professional services provided under the terms of our contract and included in this report.
3. The services performed and outlined in this report were based, in part, upon visual observations of the site and attendant structures. Our opinion cannot be extended to portions of the site which were unavailable for direct observation, reasonably beyond the control of CHUNG & VANDER DOELEN ENGINEERING LTD.
4. The objective of this report was to assess hydrogeological conditions at the site, within the context of our contract and hydrogeological assessment guidelines within the applicable jurisdiction. Evaluating compliance of past or future owners with applicable local, provincial and federal government laws and regulations was not included in our contract for services.
5. CHUNG & VANDER DOELEN ENGINEERING LTD. has relied in good faith on information and services provided by others while conducting the record search. We accept no responsibility for any deficiency, misstatements or inaccuracies contained in this report as a result of omission, misinterpretation or fraudulent acts of the services used.
6. It should be noted that the observations and recommendations presented in this report are limited to the actual locations explored. The information presented in terms of the thickness and types of the subsoils encountered, groundwater levels, and chemical testing results, etc., are only applicable to the actual locations explored. Variations may be present between these locations. Should significant variation become apparent during later investigations, it may be necessary to reevaluate the findings of this report.
7. The conclusions of this report are based in part, on the information provided by others. The possibility remains that unexpected environmental conditions may be encountered at the site in locations not specifically investigated. Should such an event occur, CHUNG & VANDER DOELEN ENGINEERING LTD. must be notified in order that we may determine if modifications to our conclusions are necessary.



APPENDIX B

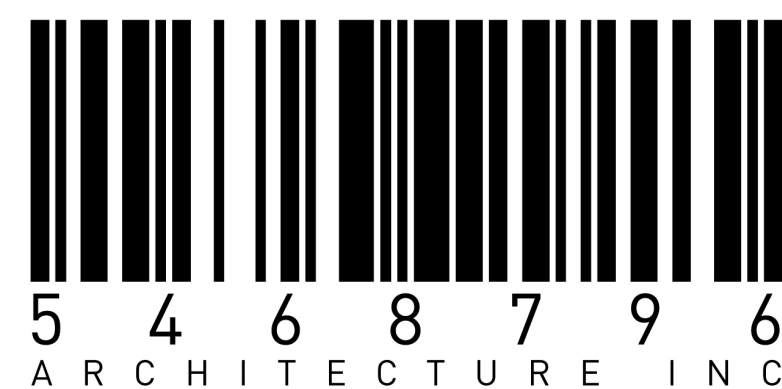
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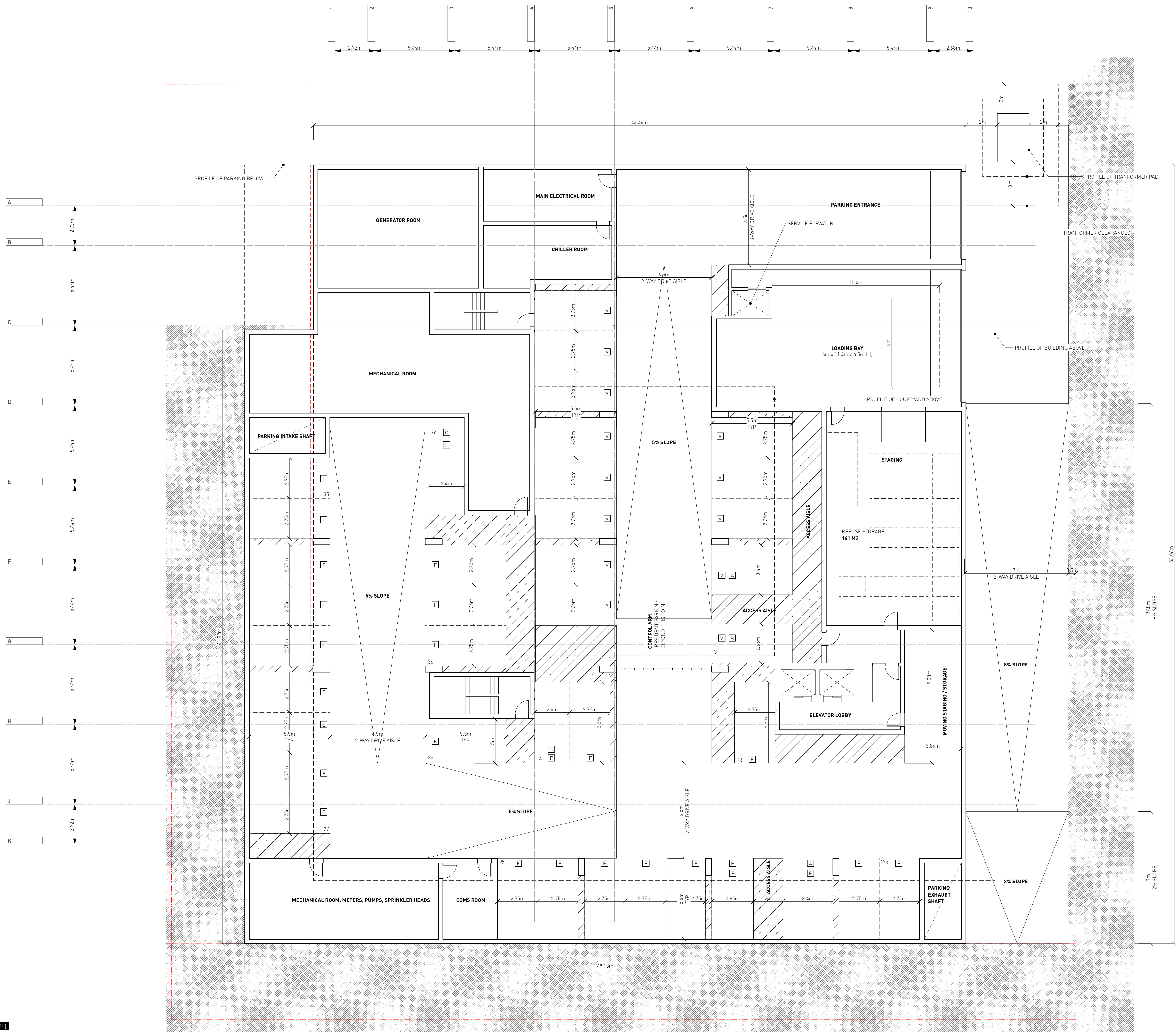
YARRA BY ELYSIUM

210-214-222 COLLEGE AVENUE EAST, GUELPH ON CAN

Issued for client review
2025.12.18







LEGEND	
A	ACCESSIBLE PARKING SPOT TYPE A (2 TOTAL)
B	ACCESSIBLE PARKING SPOT TYPE B (2 TOTAL)
C	COMPACT PARKING SPOT (6 TOTAL)
V	VISITOR PARKING SPOT (13 TOTAL)
E	ELECTRIC VEHICLE PARKING SPOT (53 TOTAL)
66 PARKING SPOTS TOTAL	
194 LONG-TERM BICYCLE PARKING SPOTS TOTAL	

Drawn By: EA
Checked By: SR
Printing Date: 2025.12.18

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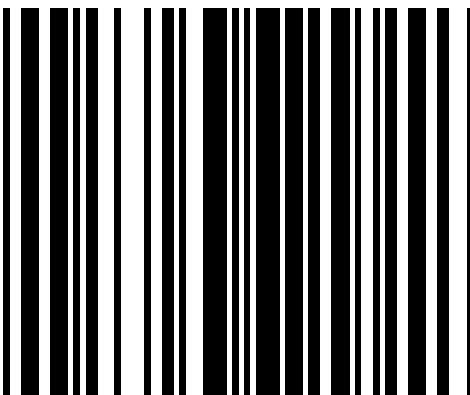
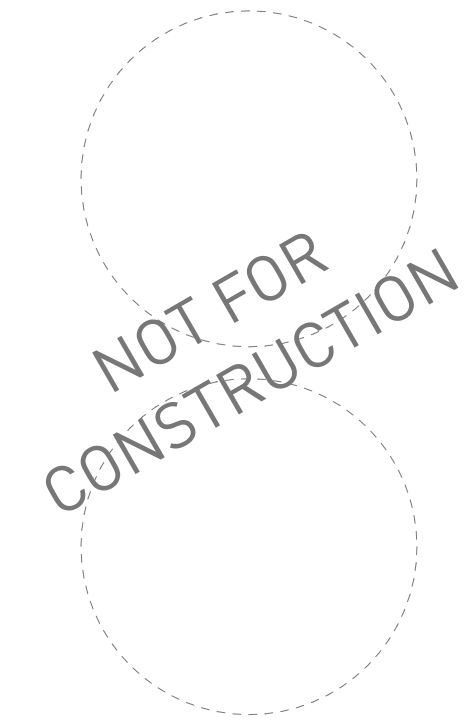
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F: 204.480.8876

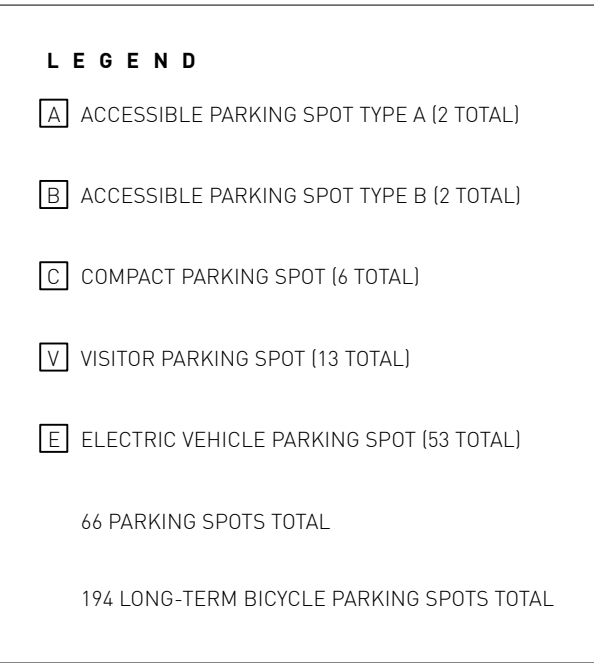
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210-222 COLLEGE AVE EAST
GUELPH ON CAN

PARKING LEVEL FLOOR
PLAN (ENTRY)

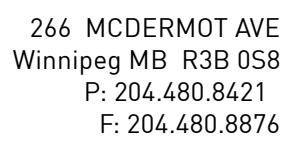
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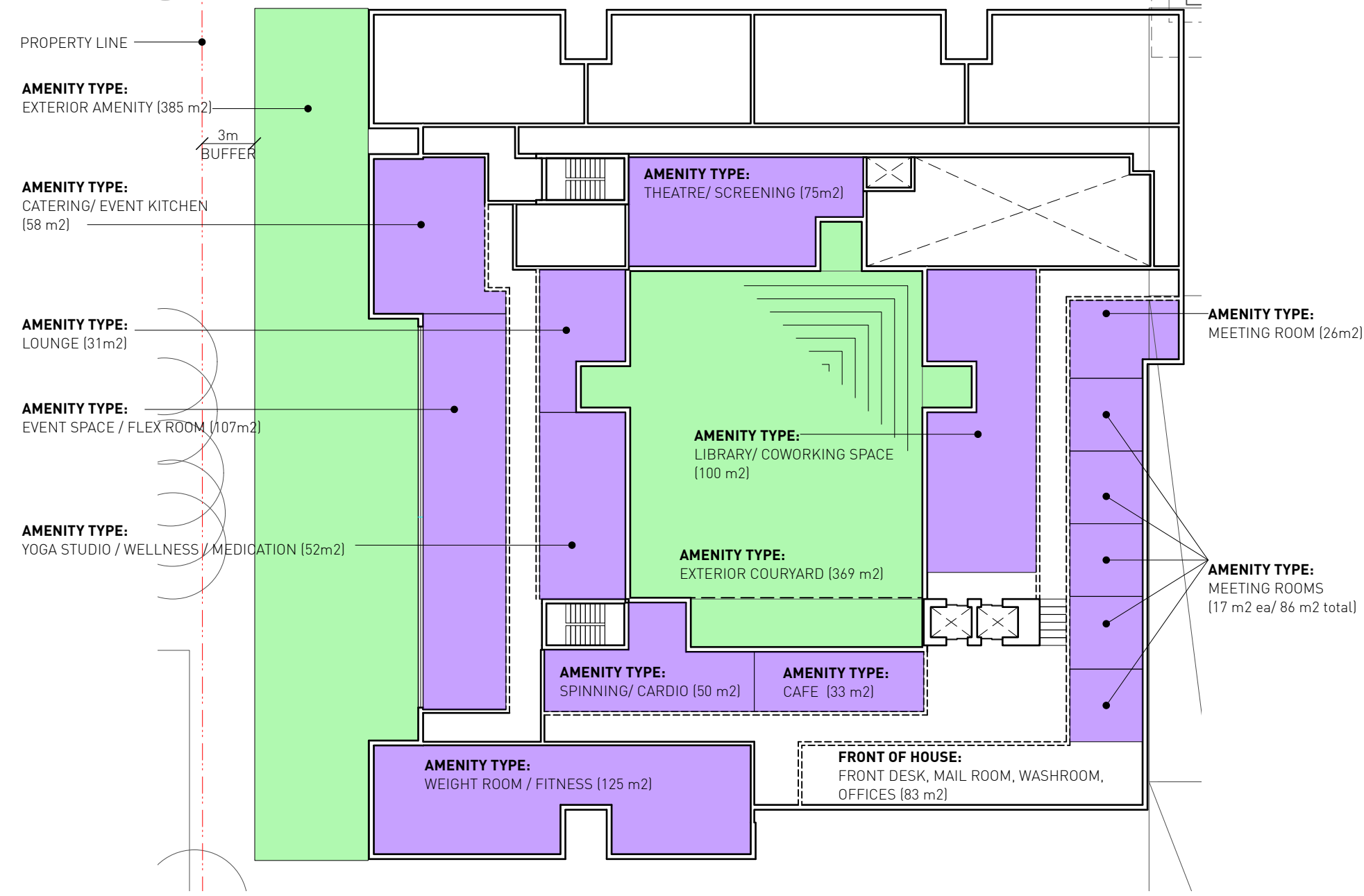


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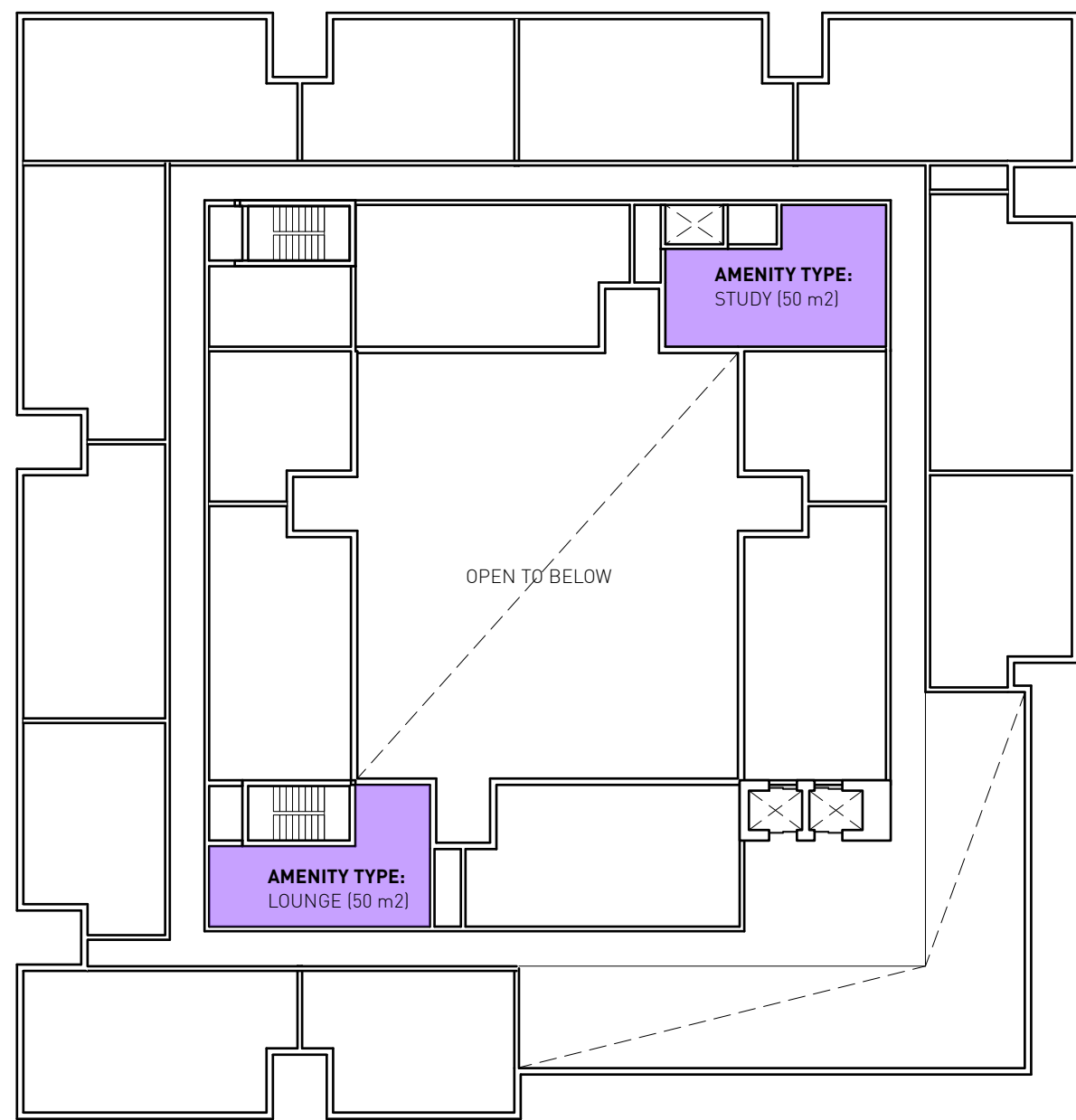
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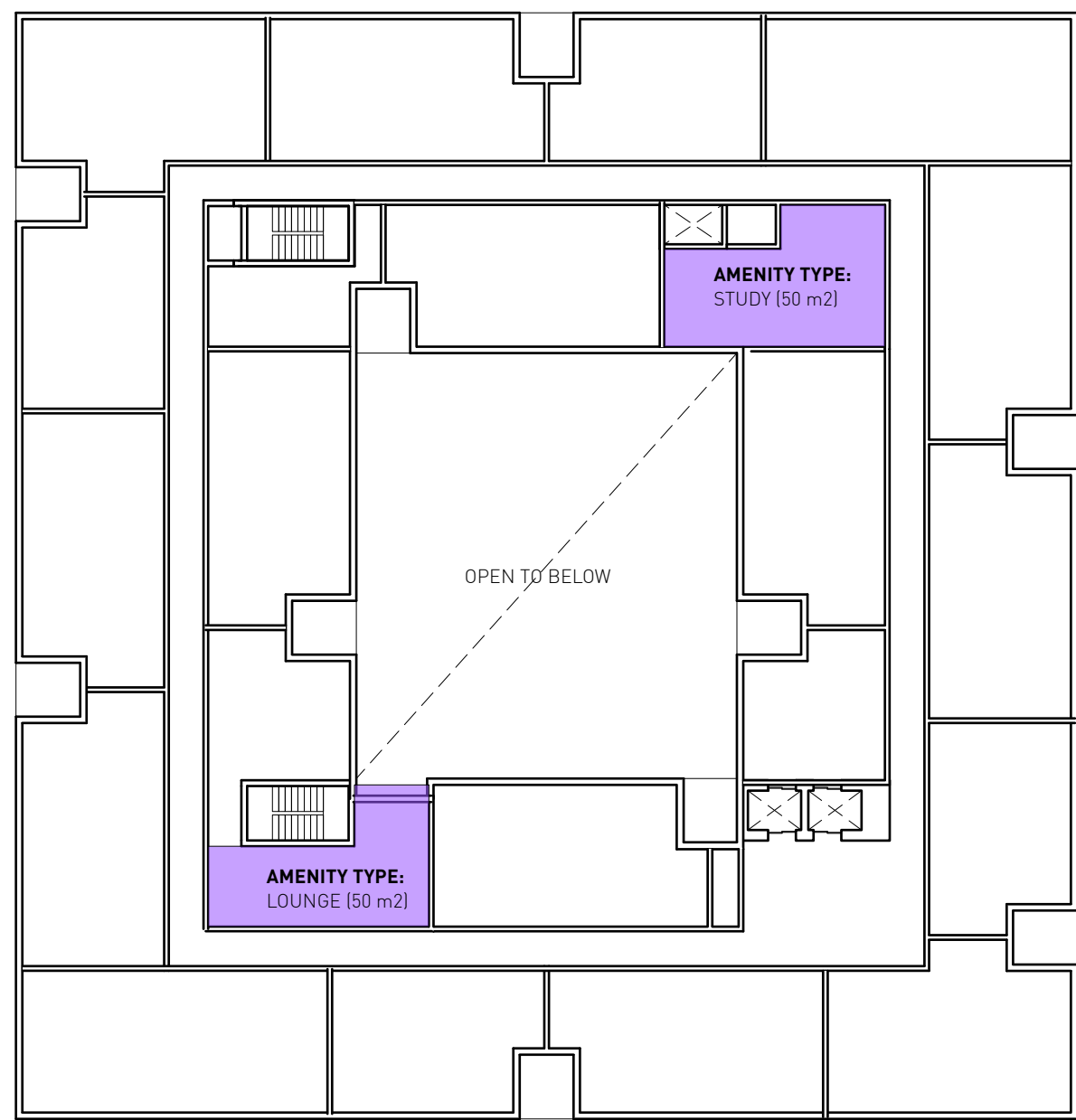
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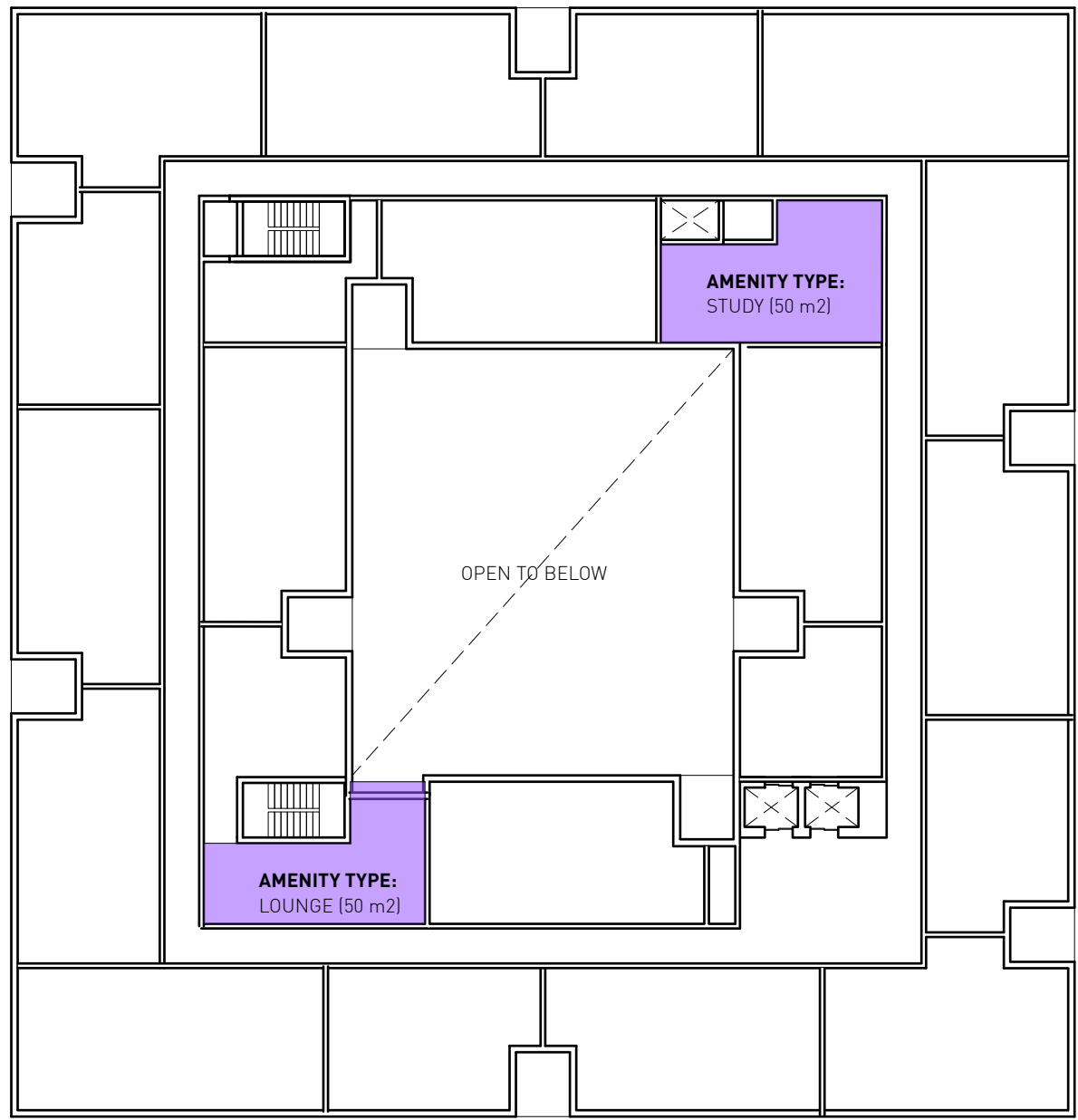
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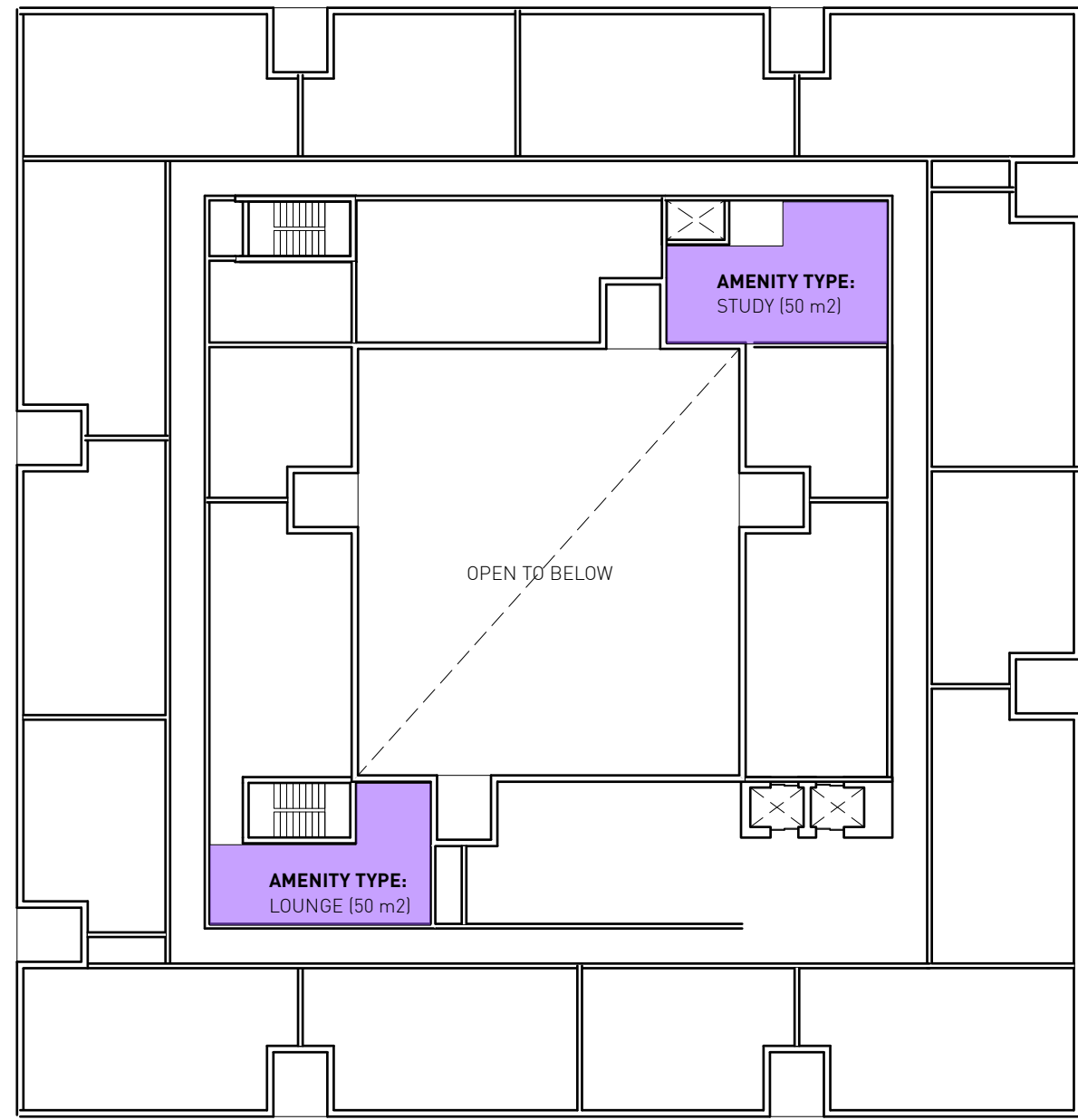
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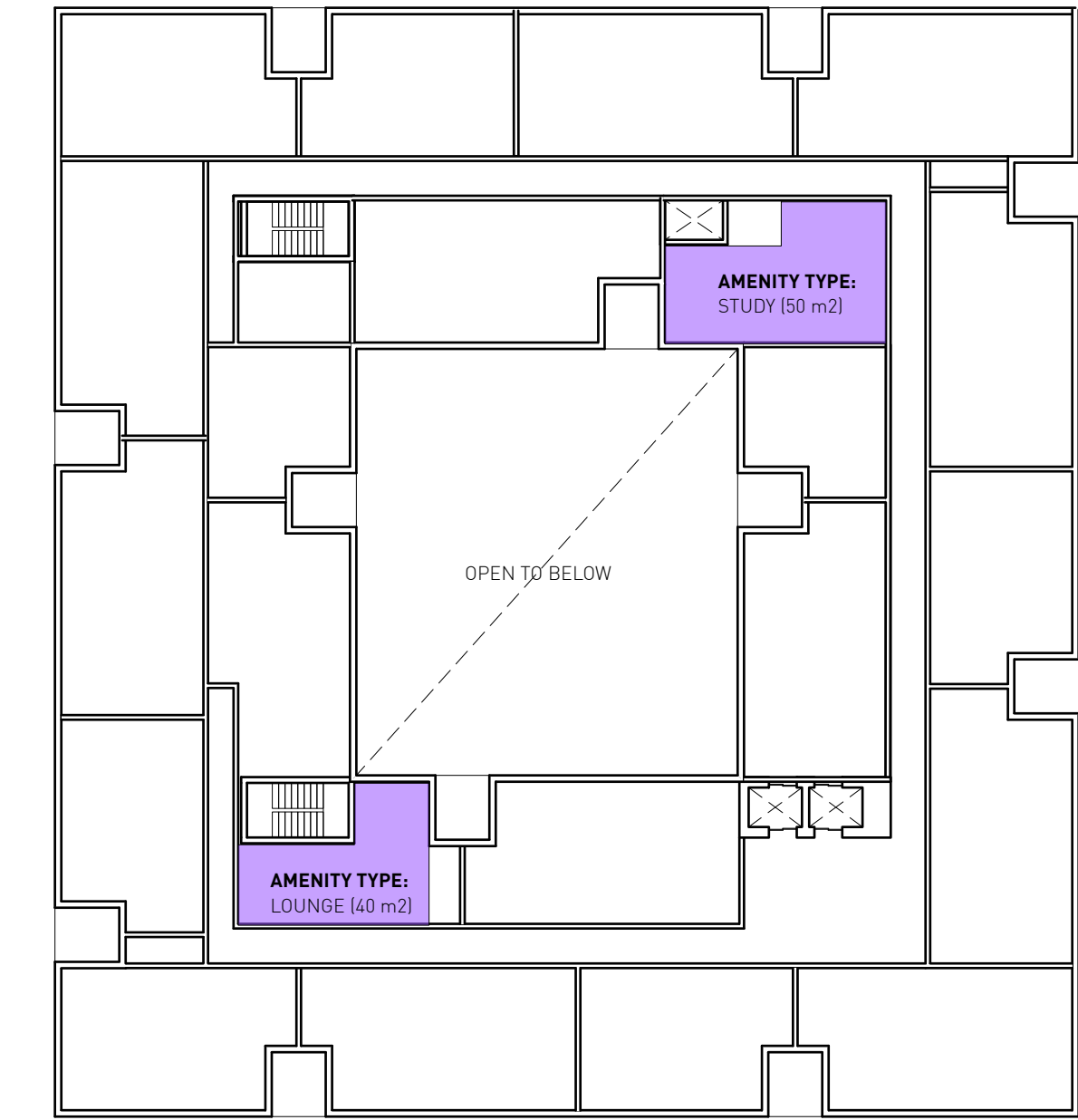
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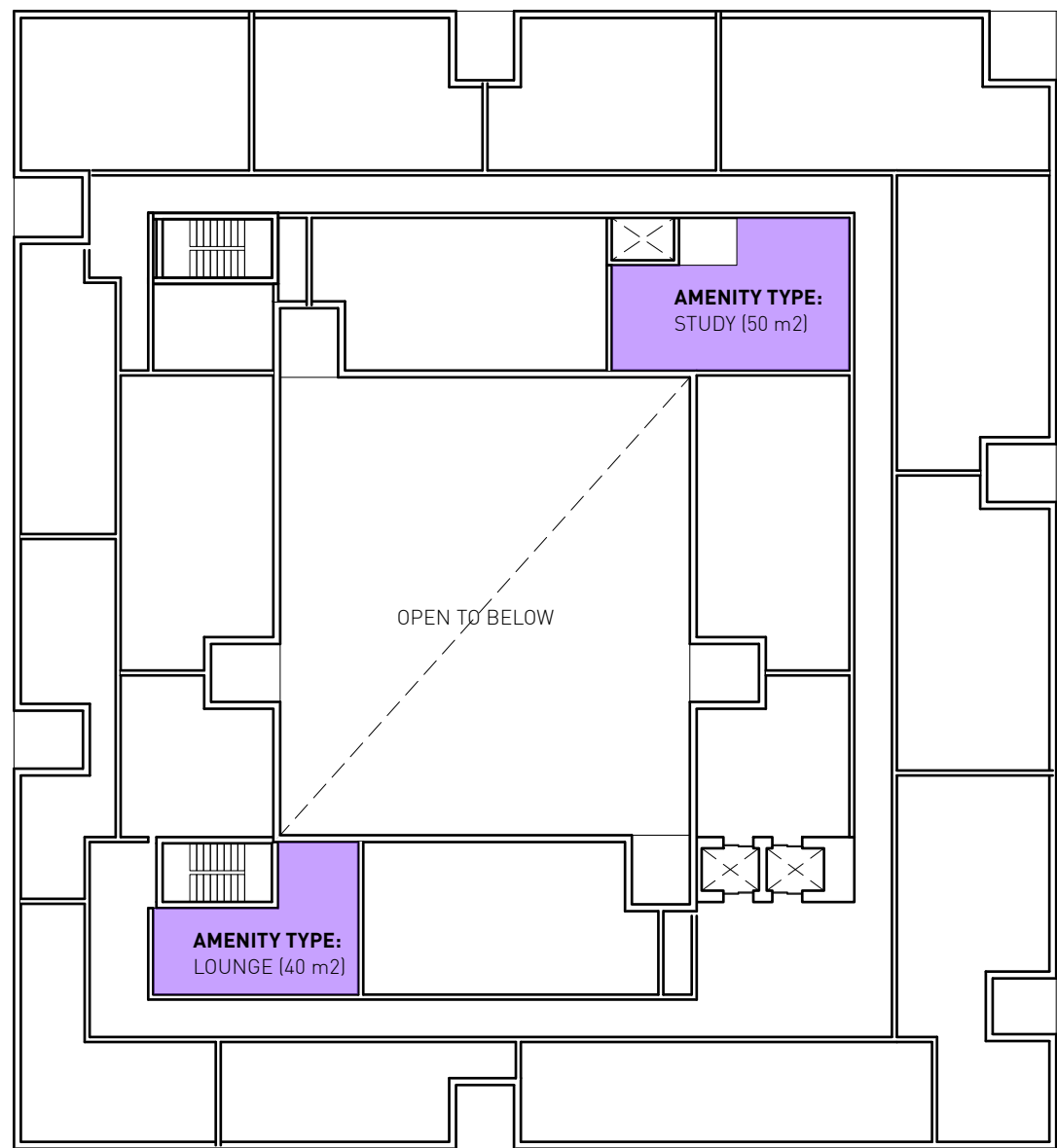
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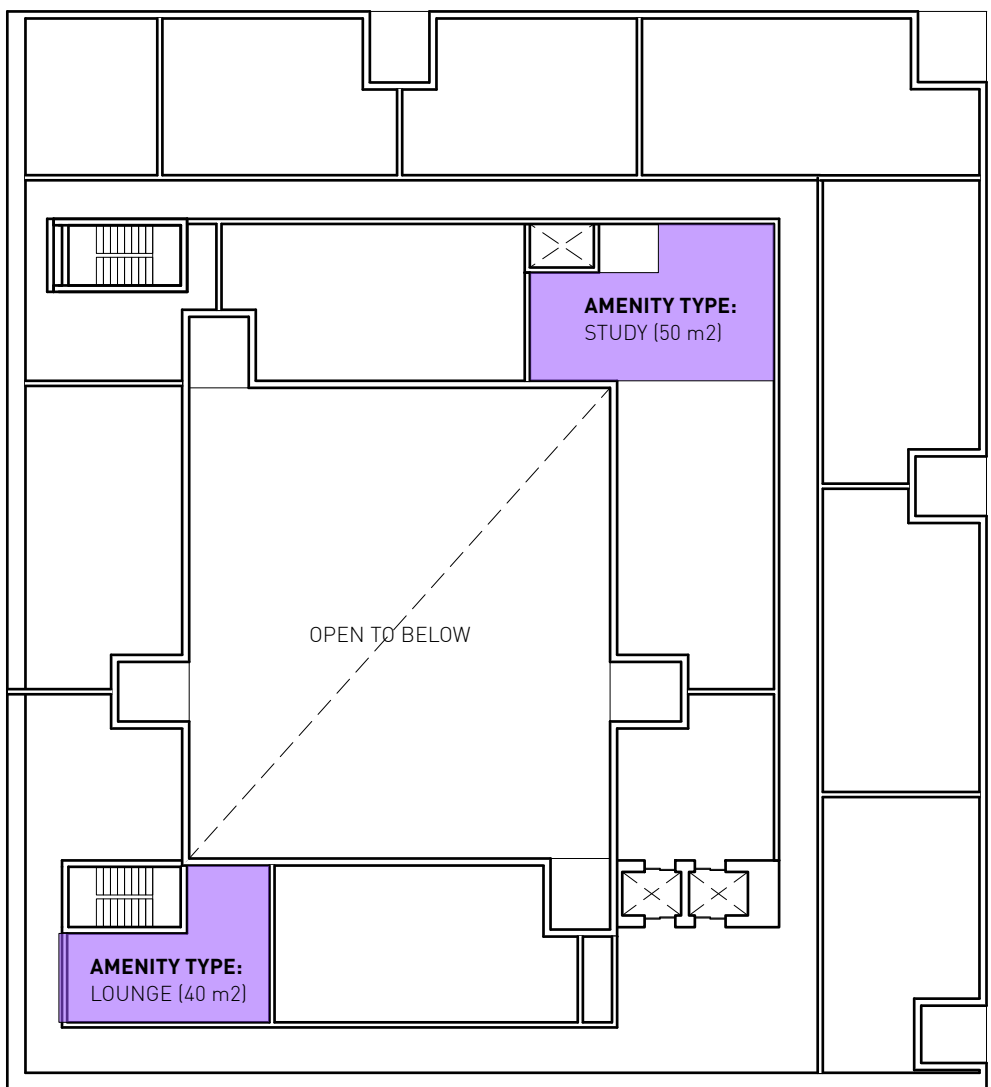
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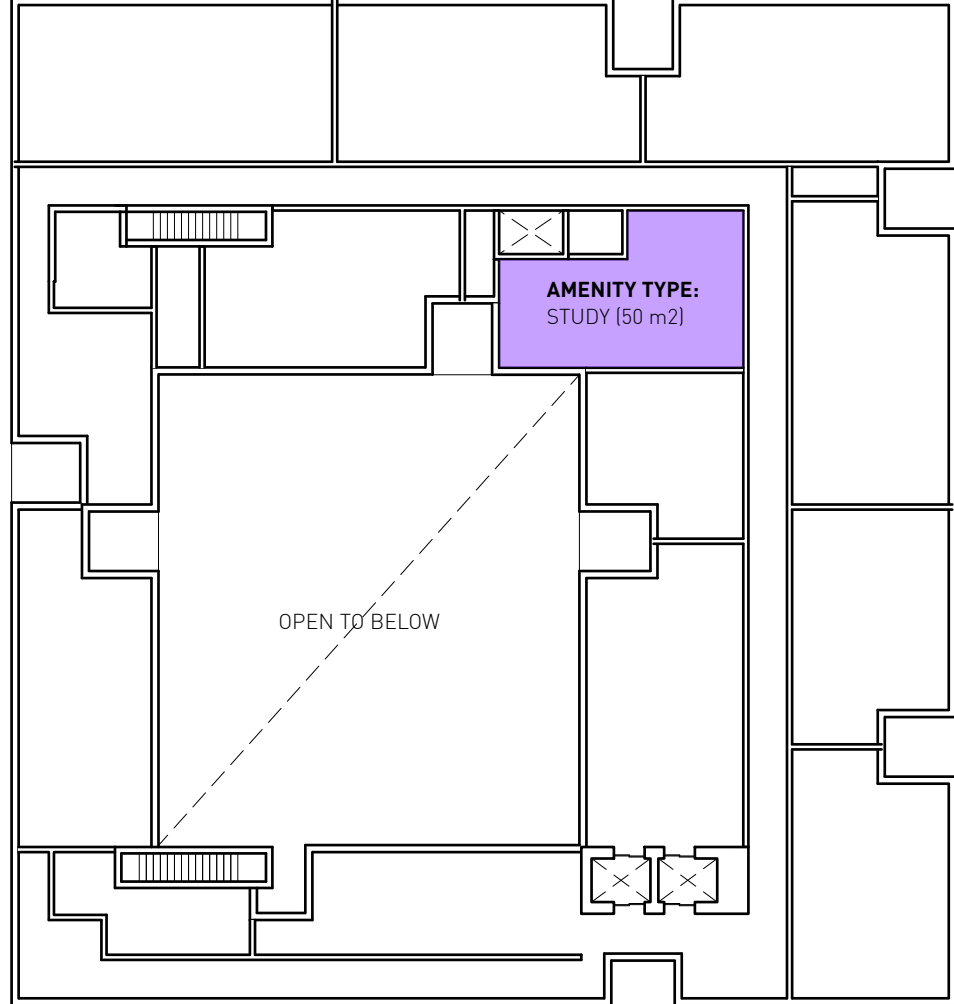
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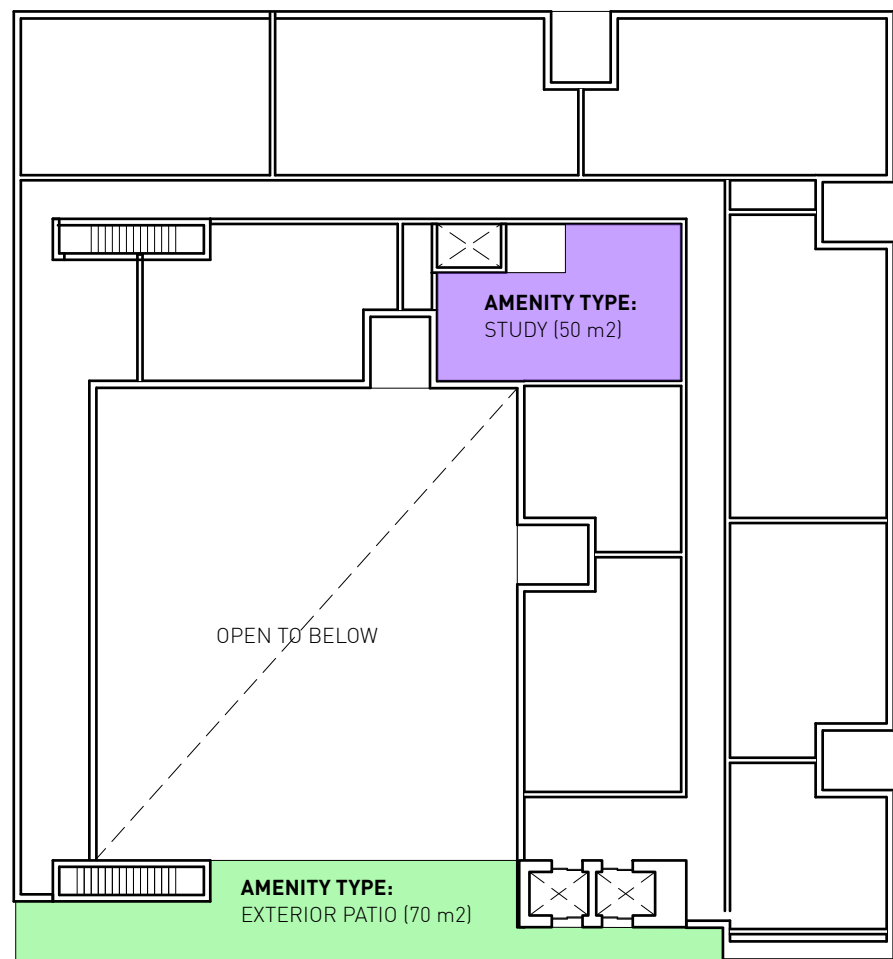
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L08 - AMENITY PLAN
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L09 - AMENITY PLAN
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10
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L10 - AMENITY PLAN
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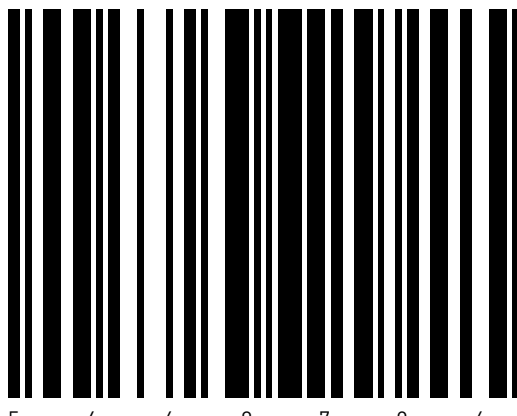
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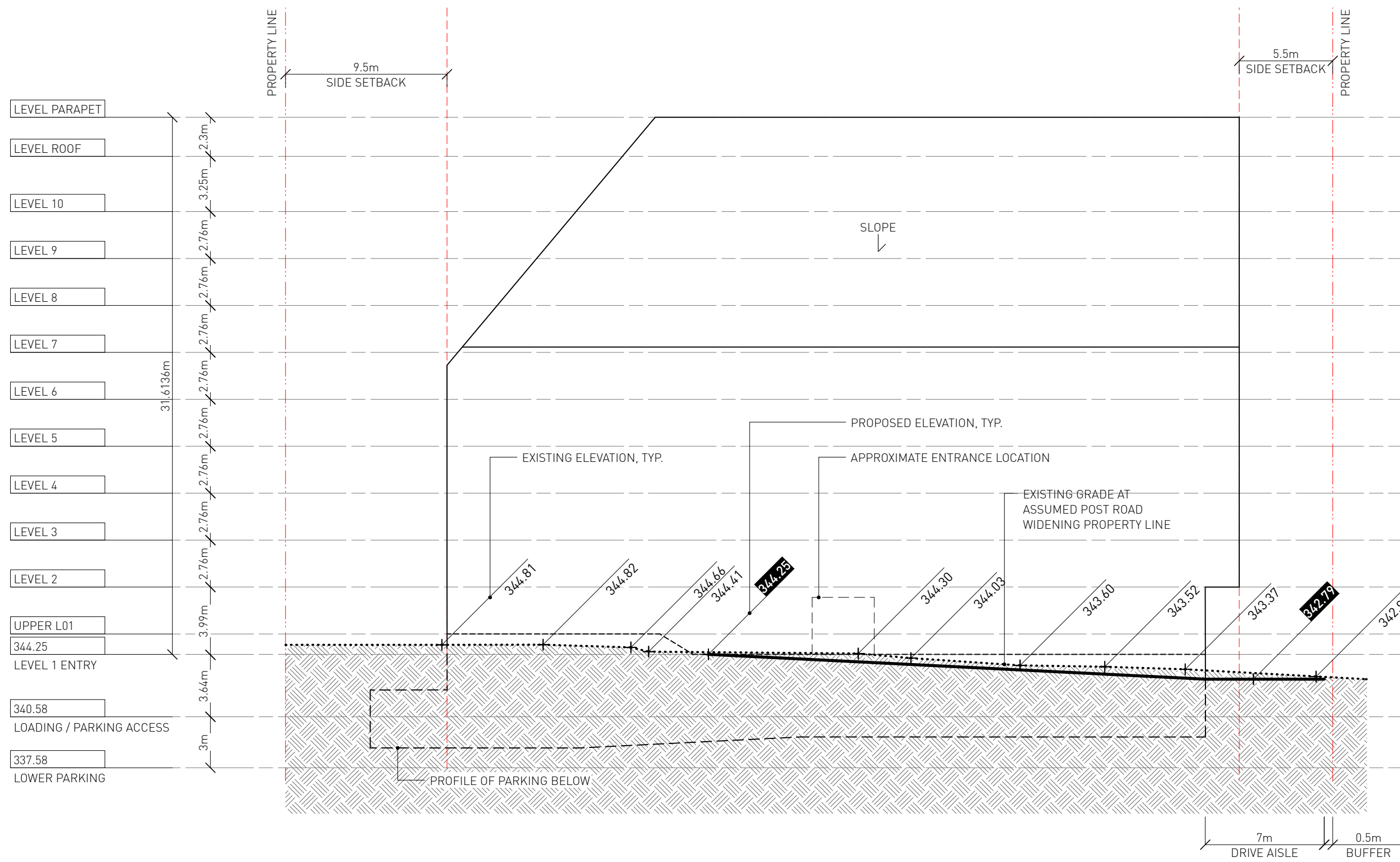
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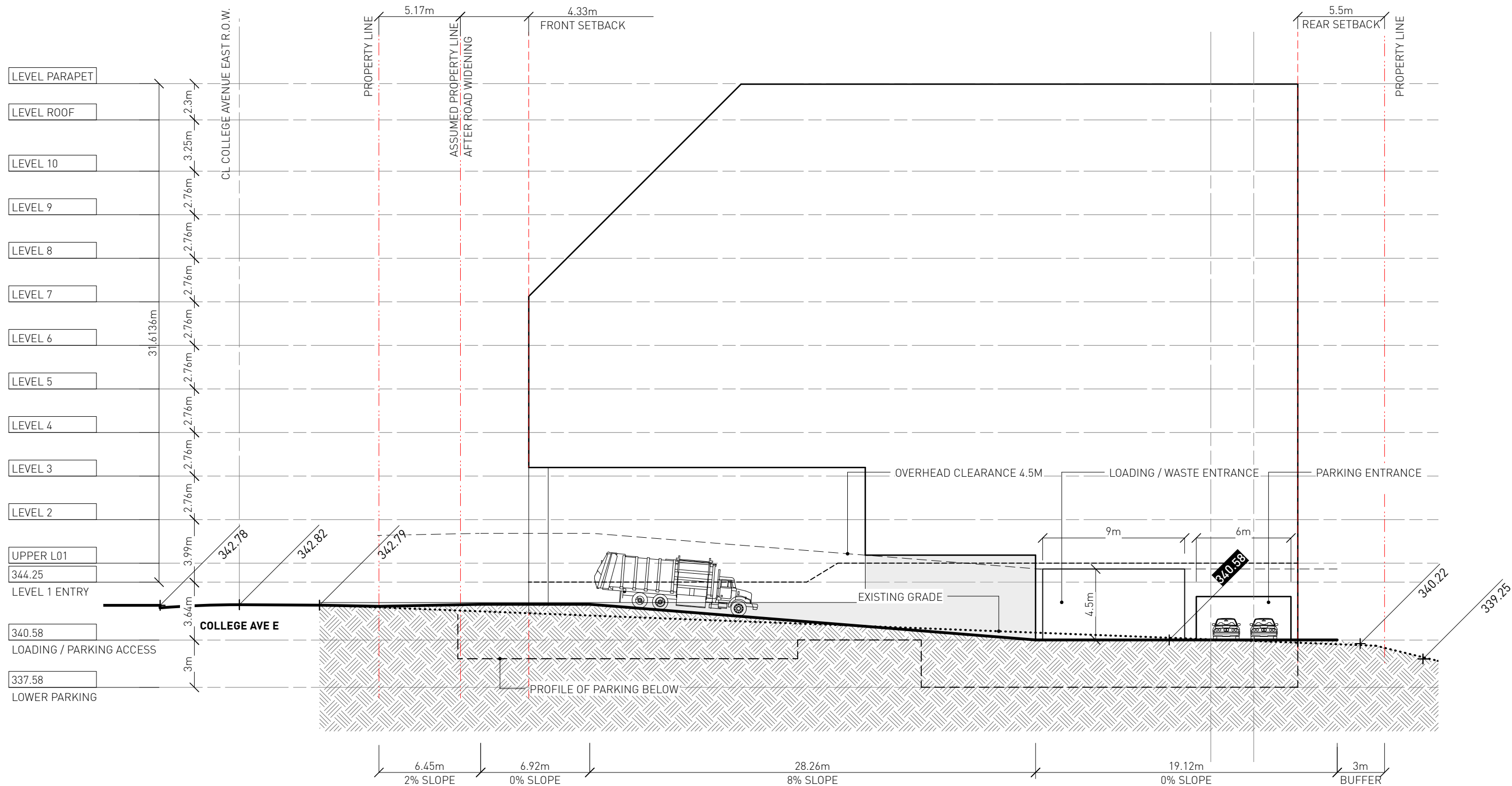
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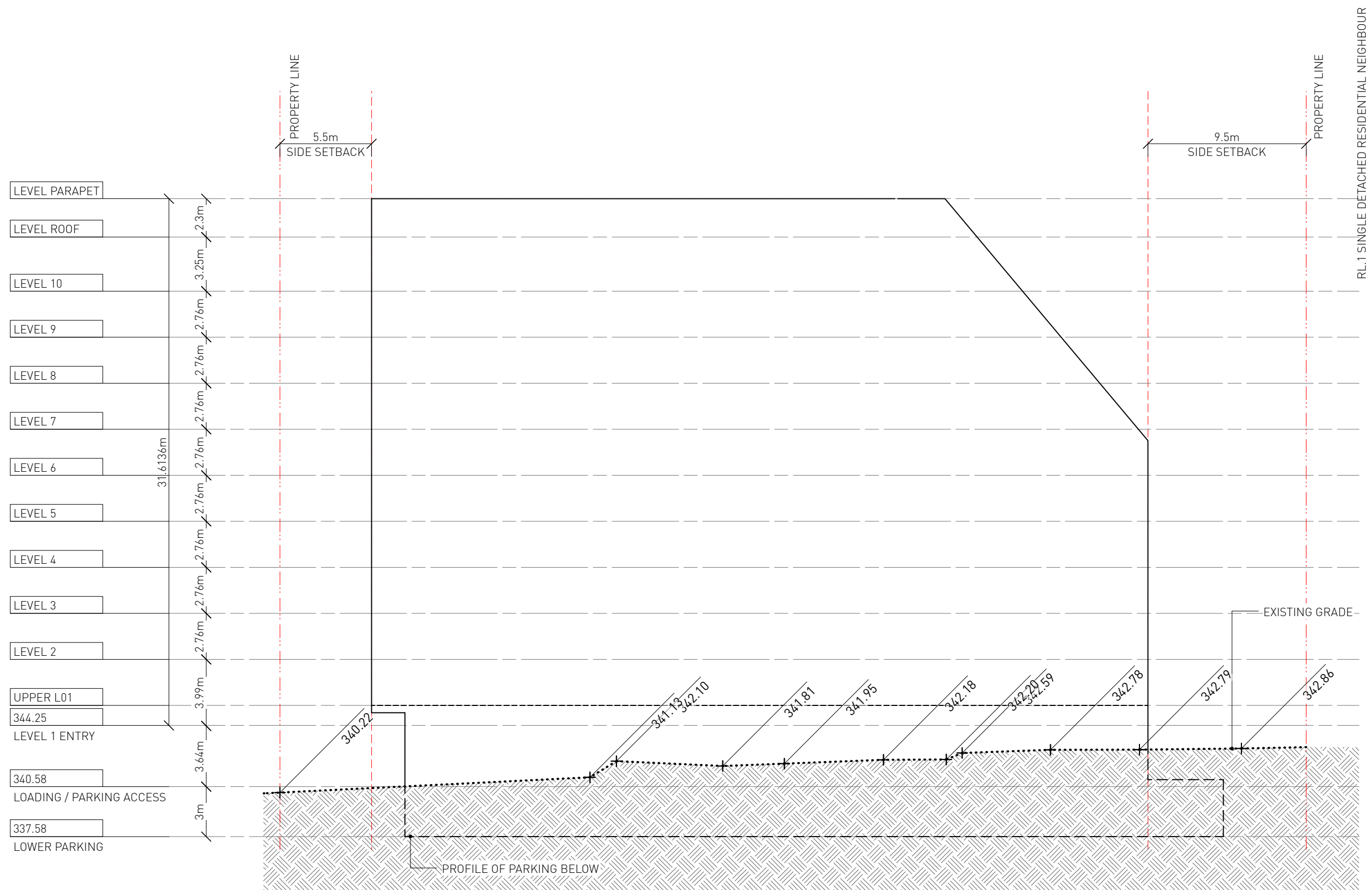
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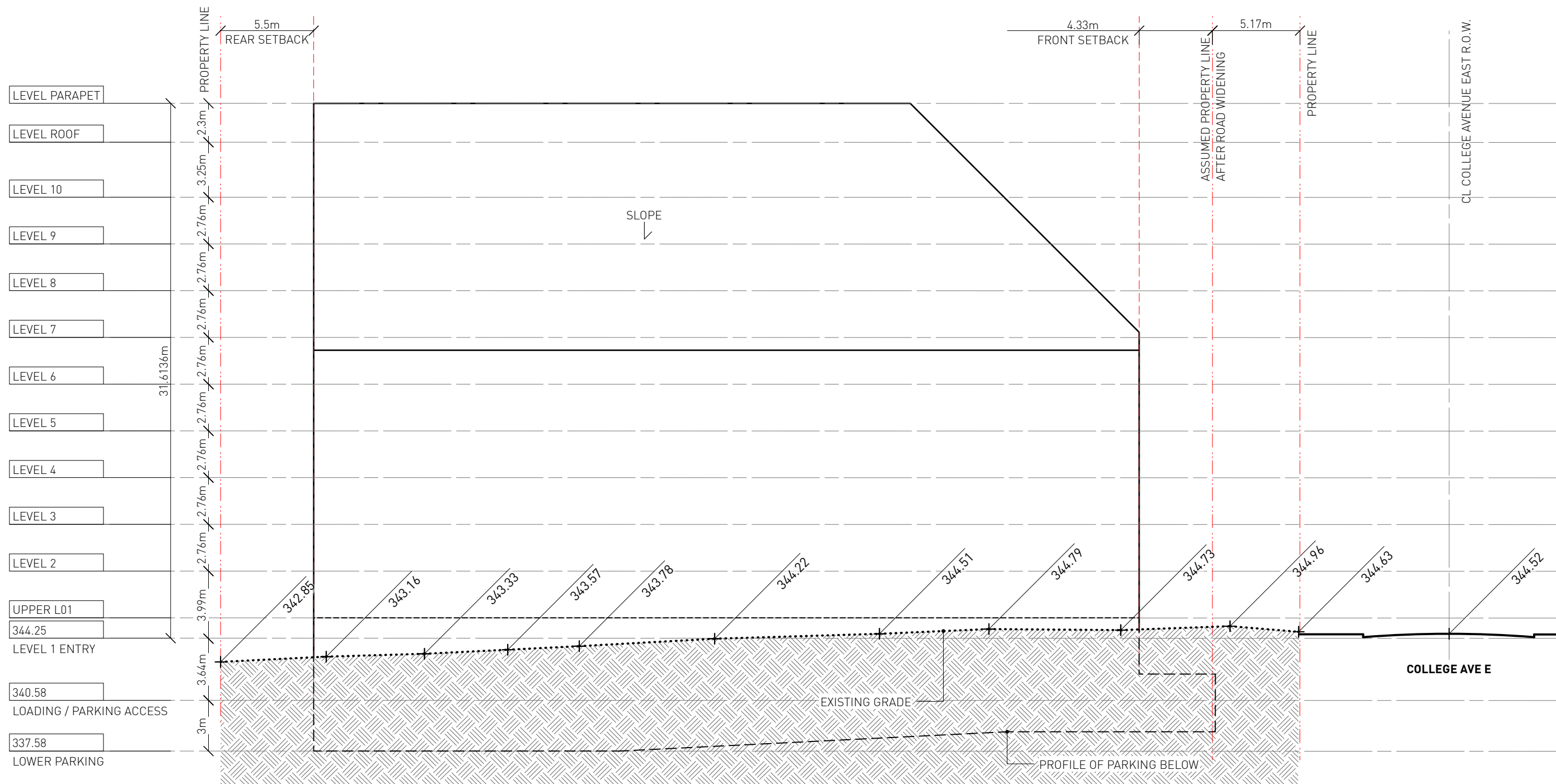
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3 ELEVATIONS
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4 ELEVATIONS
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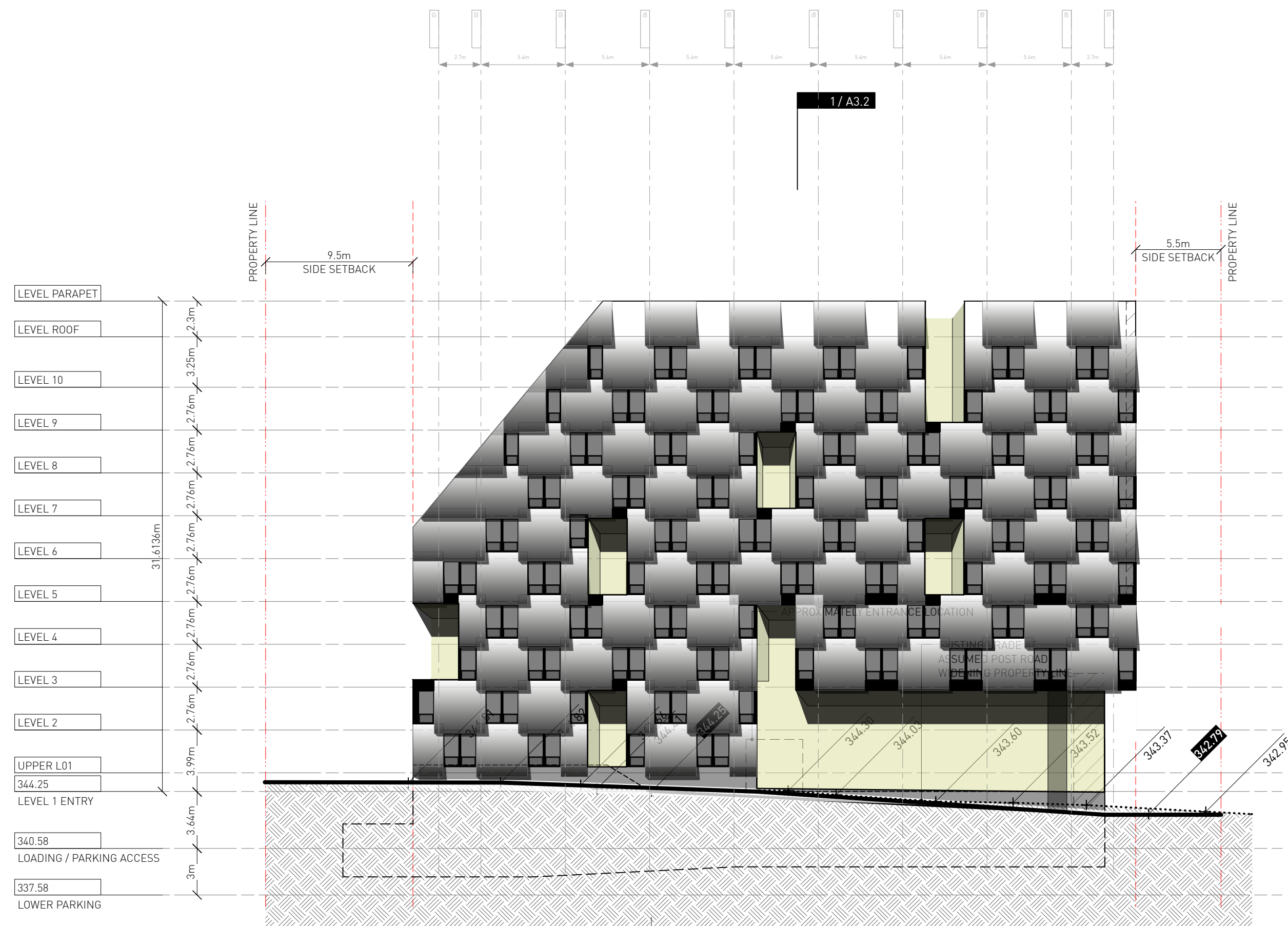


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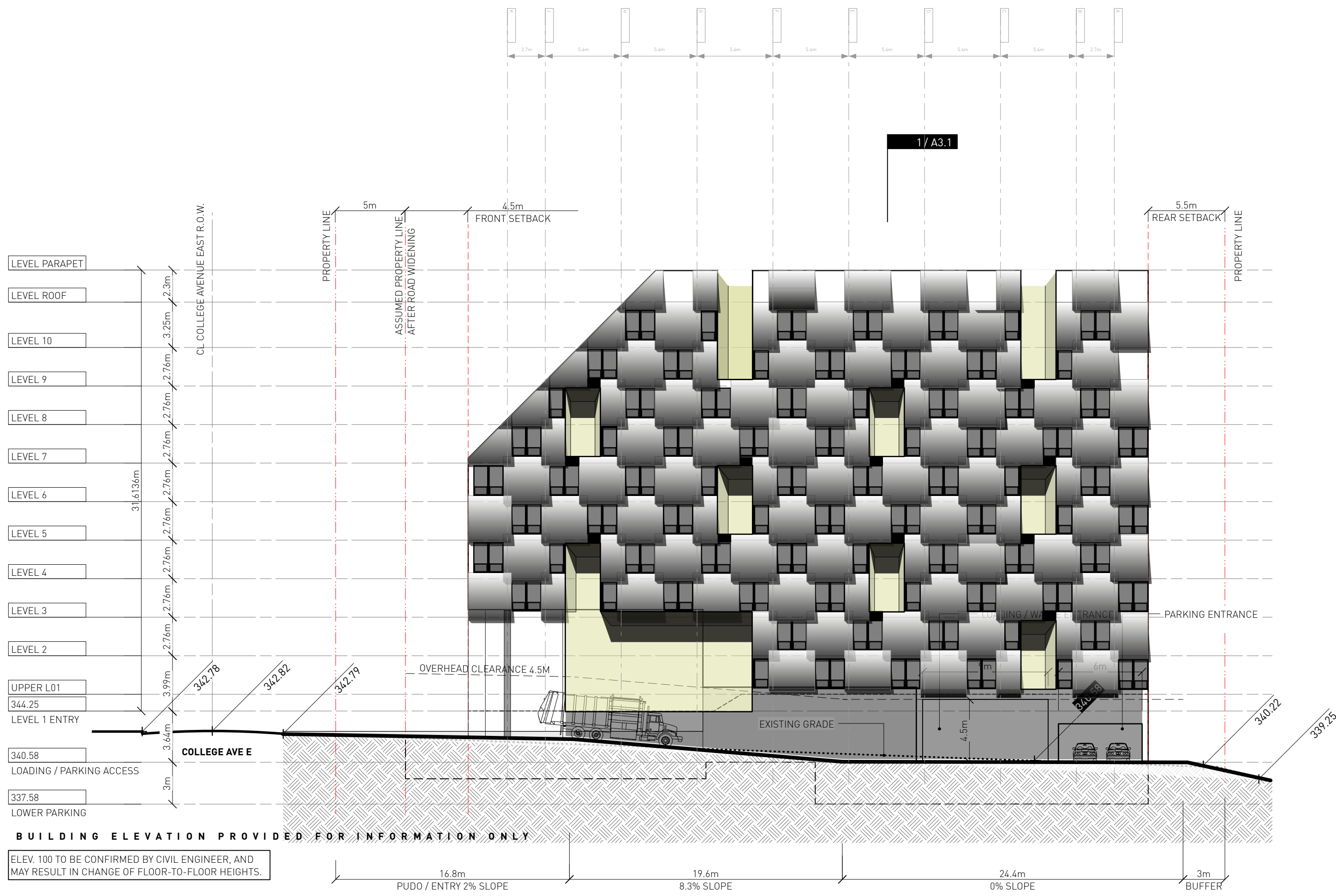
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EP-1



BUILDING ELEVATION PROVIDED FOR INFORMATION ONLY

1 ELEVATIONS
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BUILDING ELEVATION PROVIDED FOR INFORMATION ONLY

ELEV. 100 TO BE CONFIRMED BY CIVIL ENGINEER, AND
MAY RESULT IN CHANGE OF FLOOR-TO-FLOOR HEIGHTS.

2 ELEVATIONS
EP-2 SCALE: 1:250

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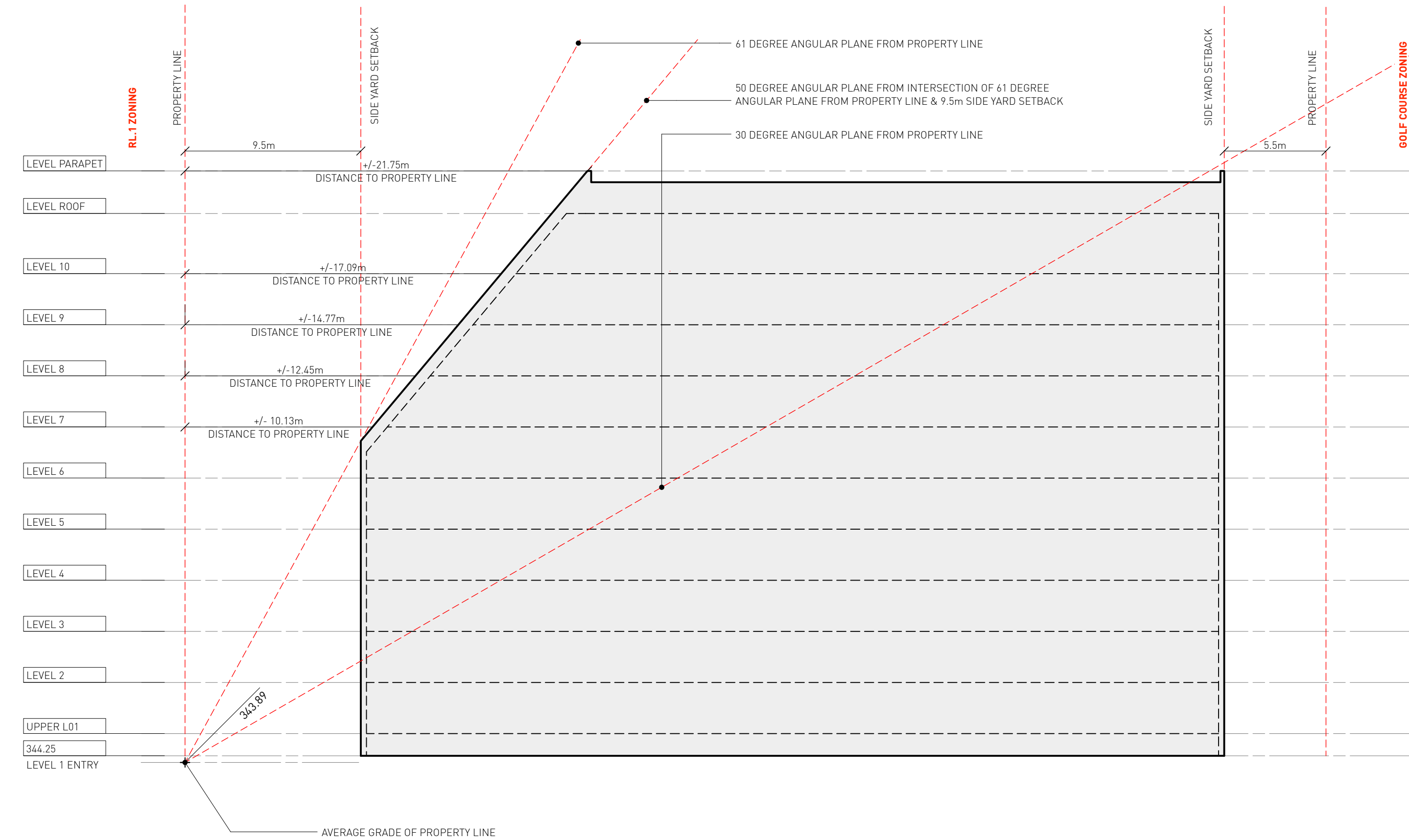
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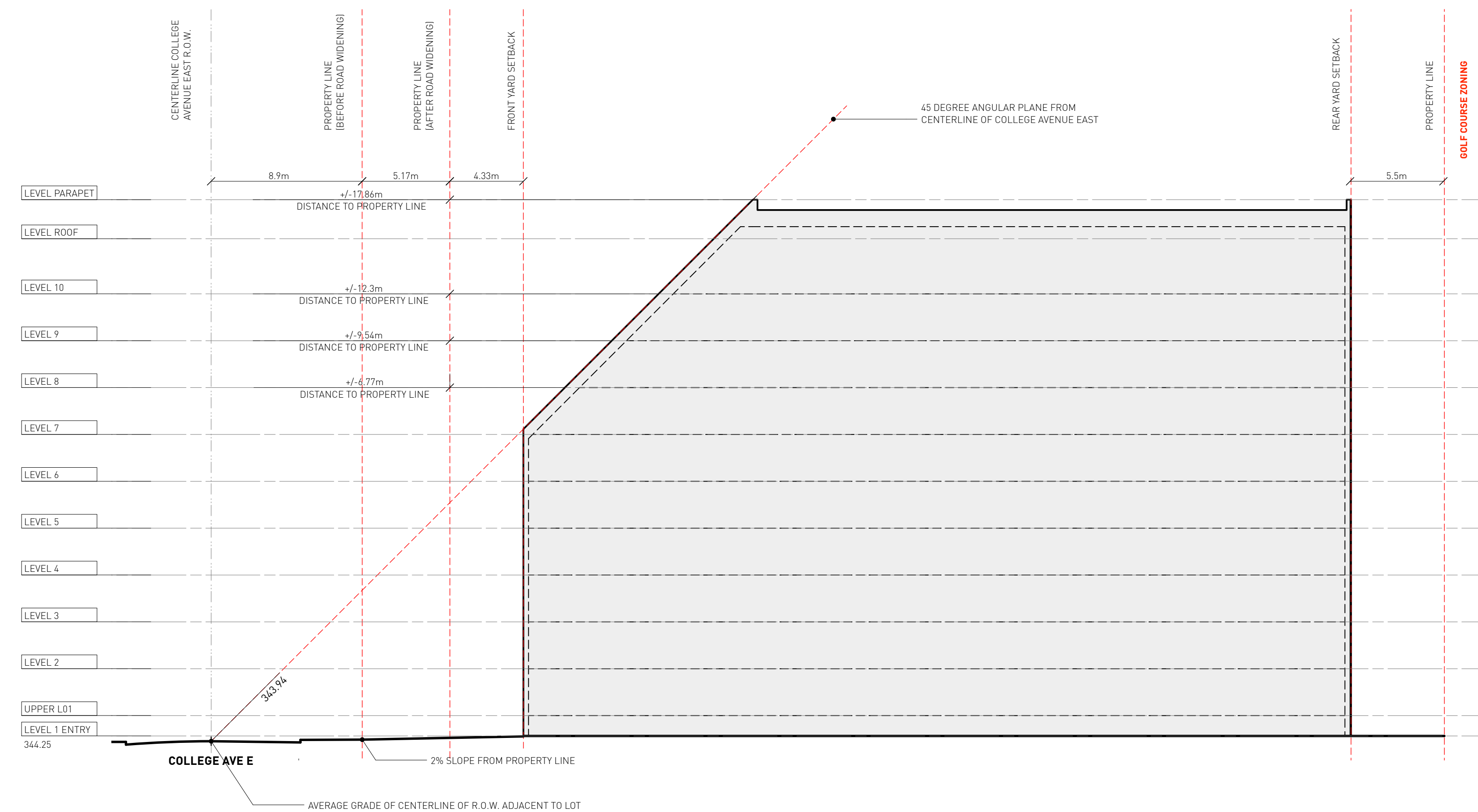
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ELEVATIONS Project
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EP-2



1 ANGULAR PLANE DIAGRAM WEST / EAST
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2 ANGULAR PLANE DIAGRAM NORTH / SOUTH
AP-1 SCALE: 1:200

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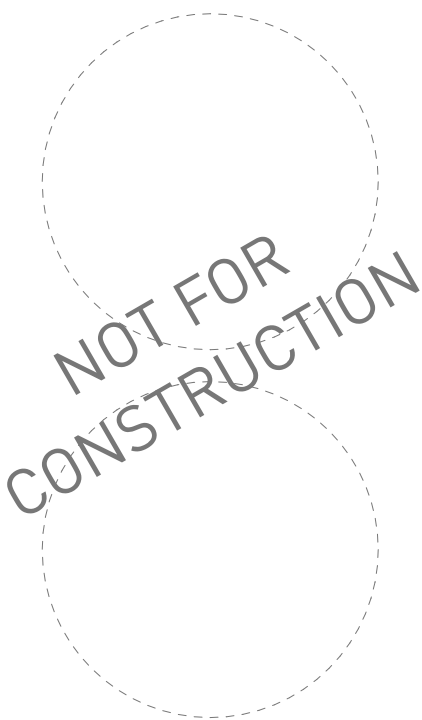
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ANGULAR PLANE
DIAGRAMS
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AP-1

APPENDIX C

Groundwater Data



Table 1 - Summary of Water Levels, Elevations & Fluctuations

210 – 222 College Avenue East, Guelph
CVD Engineering Ltd.
Project: 2372H

Well	Ground Elevation (mASL)	Stickup (m)	Hydraulic Conductivity (m/s)	Water Level (m Below Ground)						Water Elevation (mASL)						Water Level Fluctuations Relative to April 14, 2025 (m)					Max Groundwater Elevation (Manual + Loggers) (mASL)
				14-Apr-25	01-May-25	29-Aug-25	12-Nov-25	29-Jan-26	17-Apr-26	14-Apr-25	01-May-25	29-Aug-25	12-Nov-25	29-Jan-26	17-Apr-26	01-May-25	29-Aug-25	12-Nov-25	29-Jan-26	17-Apr-26	
BH 1	344.59	0.00	2.5×10^{-7}	1.36	1.95	3.78	4.16	2.24	0.83	343.23	342.64	340.81	340.43	342.35	343.77	-0.59	-2.42	-2.80	-0.88	0.54	343.79
BH 2	341.83	1.11	2.1×10^{-7}	0.89	1.54	2.99	3.44	1.54	0.47	340.95	340.29	338.85	338.40	340.29	341.36	-0.66	-2.10	-2.55	-0.66	0.42	341.46
BH 3	342.94	0.00	1.6×10^{-7}	1.95	2.55	4.05	4.20	2.95	1.25	340.99	340.39	338.89	338.74	339.99	341.69	-0.60	-2.09	-2.25	-1.00	0.70	341.72

- Notes:
- 1) All Elevations Referenced to Geodetic Survey by CVD.

2) **Bolded** elevations represent the maximum water table aquifer elevation manually measured at each monitoring well throughout all seasons.

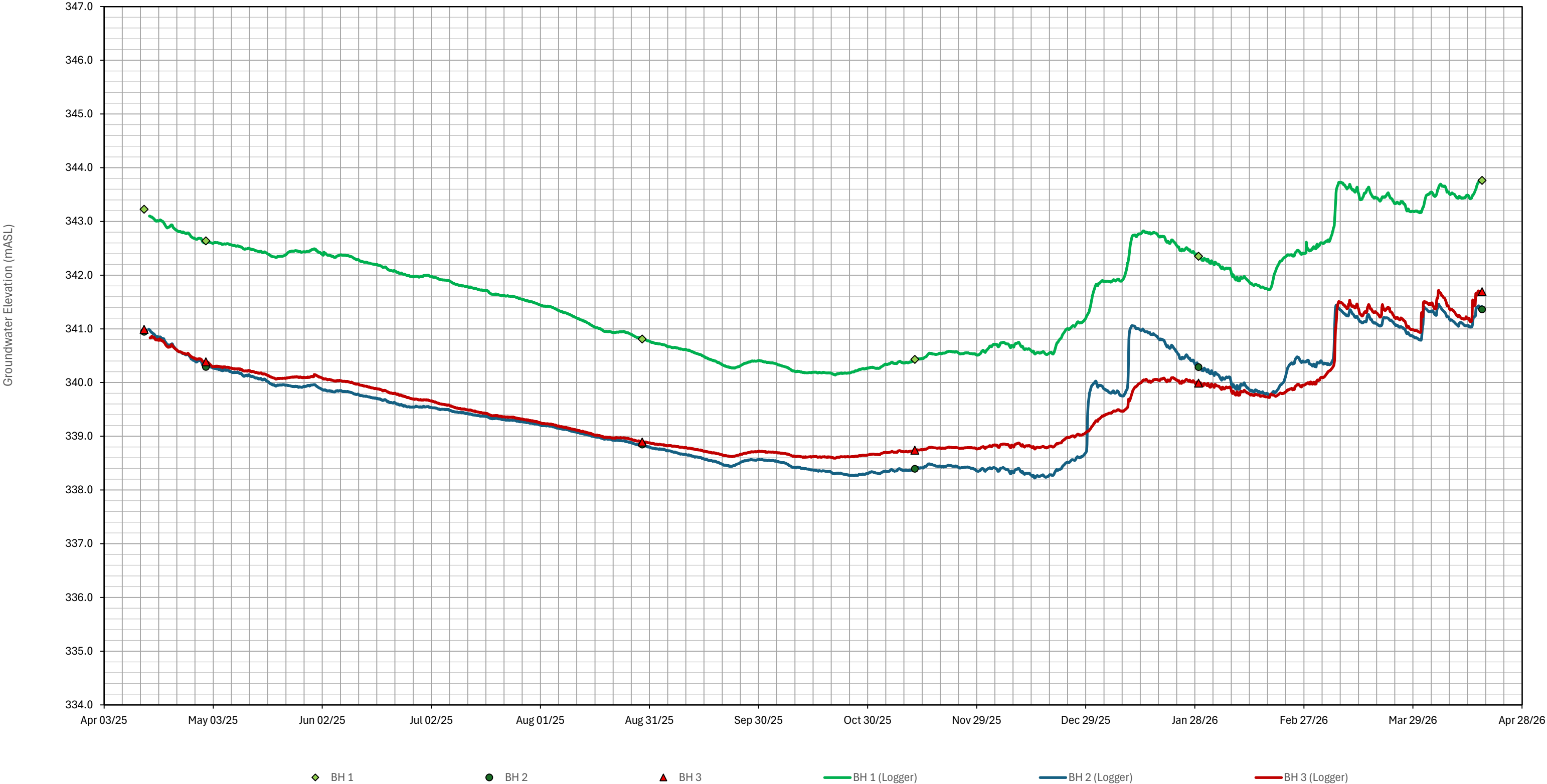
3) Negative water level indicates that water level is above ground.

4) : Monitoring well dry.

5) Negative fluctuation indicates drop in water level relative to baseline.

6) : Water level not stabilized.

Figure 1 - Hydrographs (April 2025 to April 2026)



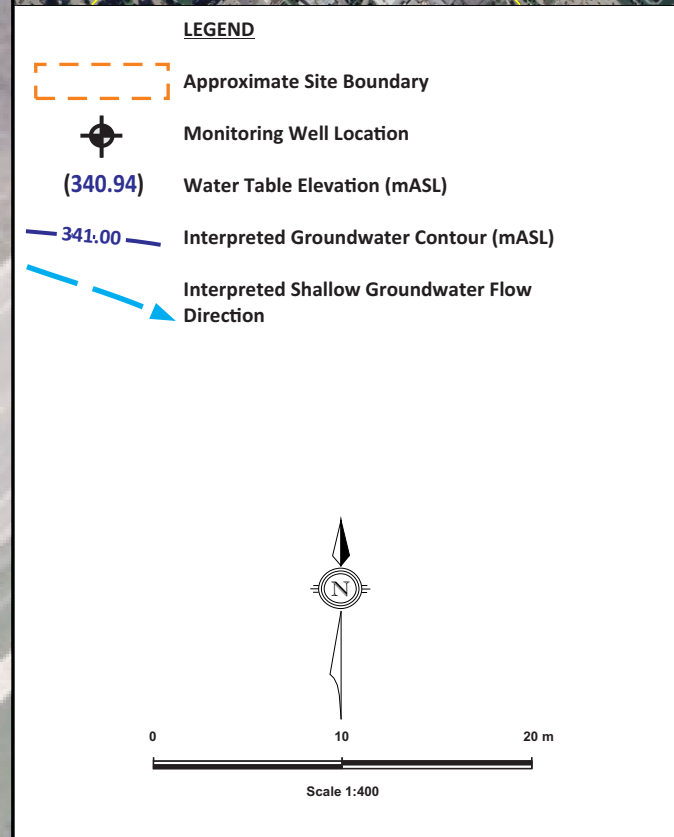
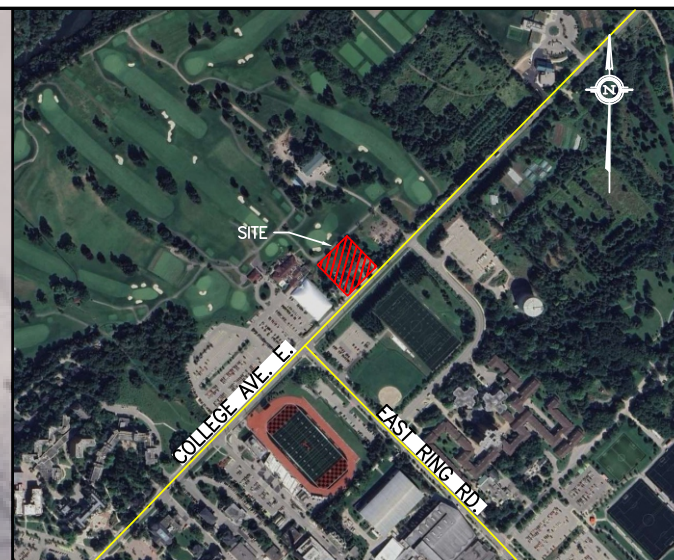


Image Ref.: City of Guelph; GIS Locator; ESRI; Retrieved: December 2025

Figure 2:
Water Table Interpretation (April 14, 2025)

Scoped Hydrogeological Assessment
Proposed Residential Development

210 – 222 College Avenue East Guelph, ON

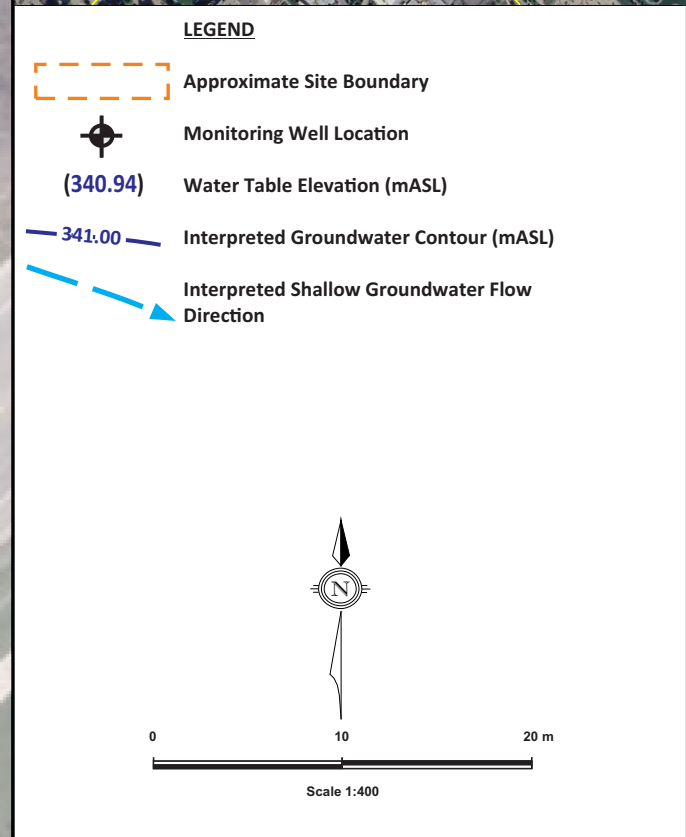
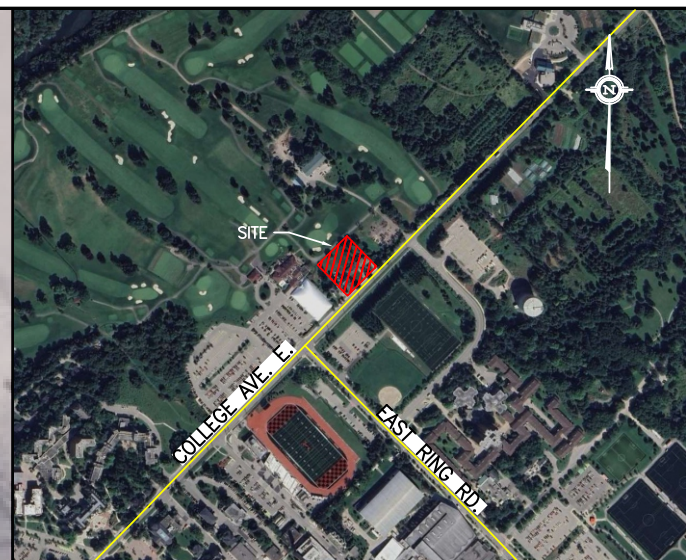


Image Ref.: City of Guelph; GIS Locator; ESRI; Retrieved: December 2025

Figure 3:
Water Table Interpretation (April 17, 2026)

Scoped Hydrogeological Assessment
Proposed Residential Development

210 – 222 College Avenue East Guelph, ON

APPENDIX D

Single Well Response Tests





CHUNG & VANDER DOELEN ENGINEERING LTD.

311 Victoria Street North
Kitchener / Ontario / N2H 5E1
519-742-8979

Response Test - Water Level Data and Analysis

Page 1 of 3

Project: Proposed Residential Development

Number: 2372

Client: Trolleybus Urban Development

Location: 210 – 222 College Avenue East, Guelph, ON Response Test: BH 1

Test Conducted by: R.S.

Test Date: 2025-05-01

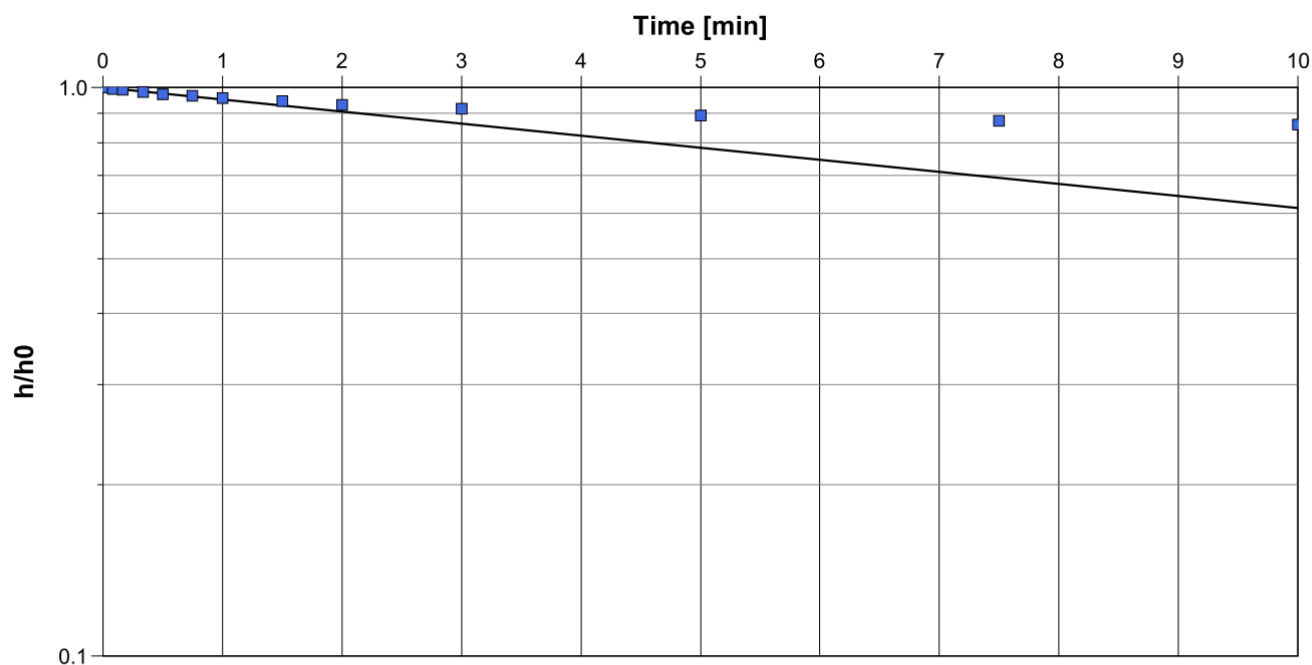
Aquifer Thickness: 12.0 m

Water level at t=0 [m]: 3.03

Static Water Level [m]: 1.95

Water level change at t=0 [m]: 1.08

	Time [min]	Water Level [m]	WL Change [m]
1	0	3.03	1.077
2	0.0833	3.025	1.072
3	0.1667	3.02	1.067
4	0.3333	3.011	1.058
5	0.5	3.00	1.047
6	0.75	2.995	1.042
7	1	2.985	1.032
8	1.5	2.972	1.019
9	2	2.958	1.005
10	3	2.941	0.988
11	5	2.915	0.962
12	7.5	2.895	0.942
13	10	2.881	0.928
14	12.5	2.872	0.919
15	15	2.86	0.907



Calculation using Bouwer & Rice

Observation Well	Hydraulic Conductivity [m/s]
BH 1	2.45×10^{-7}

**CHUNG & VANDER DOELEN**
ENGINEERING LTD.311 Victoria Street North
Kitchener / Ontario / N2H 5E1
519-742-8979**Response Test - Water Level Data and Analysis**

Page 2 of 3

Project: Proposed Residential Development

Number: 2372

Client: Trolleybus Urban Development

Location: 210 – 222 College Avenue East, Guelph, ON Response Test: BH 2

Test Conducted by: R.S.

Test Date: 2025-05-01

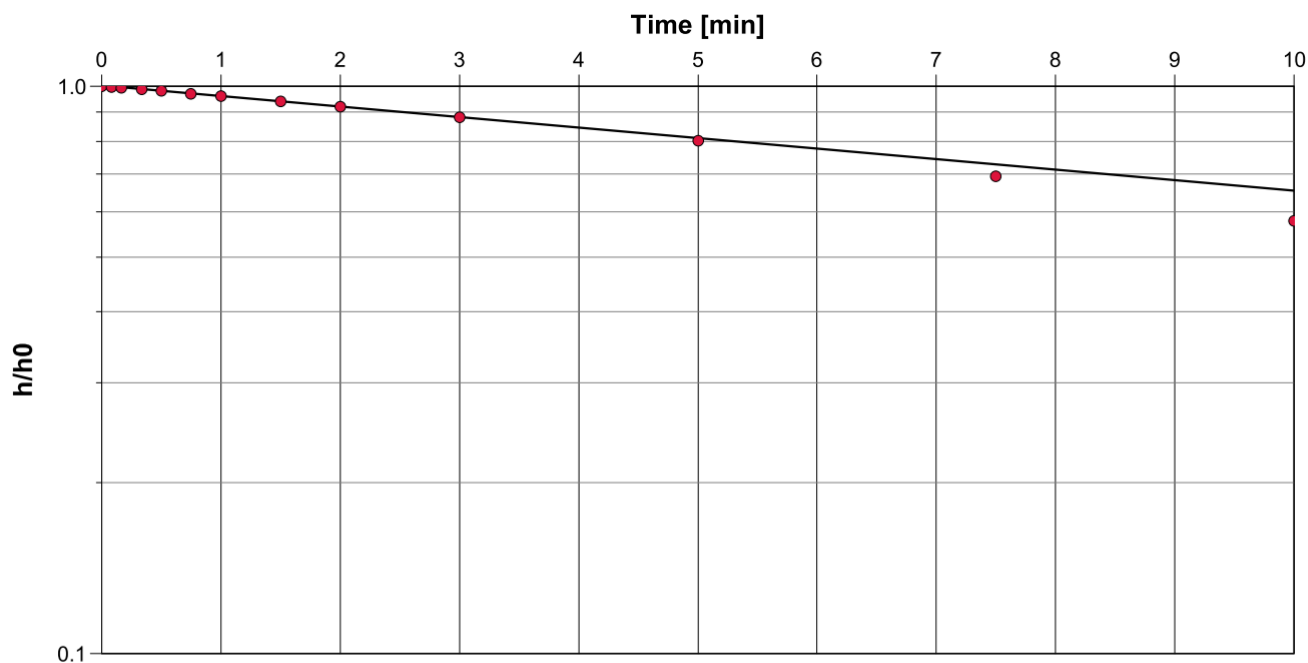
Aquifer Thickness: 12.0 m

Water level at t=0 [m]: 3.03

Static Water Level [m]: 1.55

Water level change at t=0 [m]: 1.49

	Time [min]	Water Level [m]	WL Change [m]
1	0	3.03	1.485
2	0.0833	3.027	1.482
3	0.1667	3.022	1.477
4	0.3333	3.013	1.468
5	0.5	3.003	1.458
6	0.75	2.986	1.441
7	1	2.97	1.425
8	1.5	2.941	1.396
9	2	2.912	1.367
10	3	2.853	1.308
11	5	2.734	1.189
12	7.5	2.575	1.03
13	10	2.405	0.86
14	12.5	2.226	0.681
15	15	2.048	0.503



Calculation using Bouwer & Rice

Observation Well

Hydraulic Conductivity
[m/s]

BH 2

 2.11×10^{-7}



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Response Test - Water Level Data and Analysis

Page 3 of 3

Project: Proposed Residential Development

Number: 2372

Client: Trolleybus Urban Development

Location: 210 – 222 College Avenue East, Guelph, ON Response Test: BH 3

Test Conducted by: R.S.

Test Date: 2025-05-01

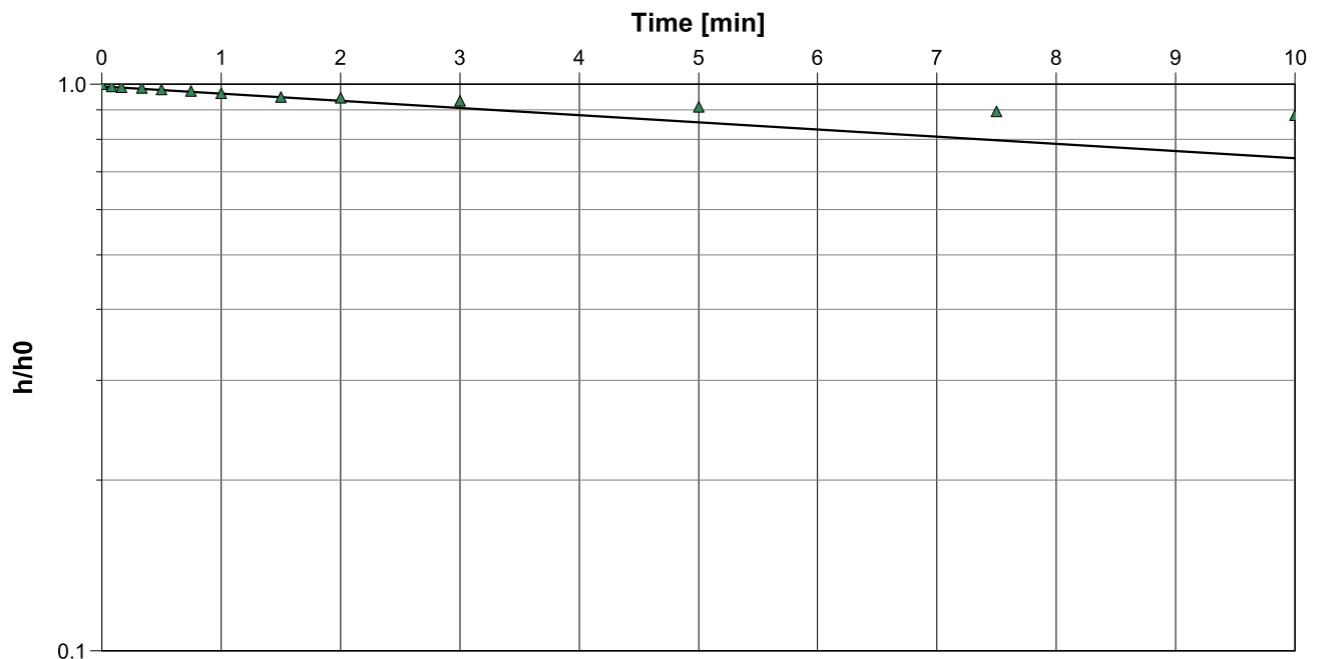
Aquifer Thickness: 12.0 m

Water level at t=0 [m]: 3.48

Static Water Level [m]: 2.55

Water level change at t=0 [m]: 0.93

	Time [min]	Water Level [m]	WL Change [m]
1	0	3.48	0.927
2	0.0833	3.471	0.918
3	0.1667	3.469	0.916
4	0.3333	3.465	0.912
5	0.5	3.459	0.906
6	0.75	3.454	0.901
7	1	3.447	0.894
8	1.5	3.434	0.881
9	2	3.43	0.877
10	3	3.419	0.866
11	5	3.398	0.845
12	7.5	3.382	0.829
13	10	3.37	0.817
14	12.5	3.357	0.804
15	15	3.345	0.792



Calculation using Bouwer & Rice

Observation Well	Hydraulic Conductivity [m/s]
BH 3	1.55×10^{-7}

APPENDIX E

Enclosures



Soil Abbreviations and Terms Used on Record of Borehole Sheets

TERMINOLOGY DESCRIBING COMMON SOIL TYPES:

Topsoil	-	mixture of soil and humus capable of supporting vegetation
Peat	-	mixture of visible and invisible fragments of decayed organic matter
Till	-	unstratified glacial deposit which may range from clay to boulders
Fill	-	soil materials identified as being placed anthropologically

CLASSIFICATION (UNIFIED SYSTEM)

Clay	<0.002mm
Silt	0.002 to .075mm
Sand	0.075 to 4.75mm
	Fine 0.075 to 0.425 mm
	Medium 0.425 to 2.0 mm
	Coarse 2.0 to 4.75 mm
Gravel	4.75 to 75mm
	Fine 4.75 to 19 mm
	Coarse 19 to 75 mm
Cobbles	75 to 300mm
Boulders	>300mm

TERMINOLOGY

Soil Composition	% by Weight
"traces"	<10%
"some"(eg. some silt)	10-20%
Adjective (eg. sandy)	20-35%
"and"(eg. sand and gravel)	35-50%

Standard Penetration Resistance (SPT): Standard Penetration Resistance ('N' Values) refers to the number of blows required to advance a standard (ASTM D1586) 51 mm Ø (2 inch) split-spoon sampler by the use of a free falling, 63.5 Kg (140lbs) hammer. The number of blows from the drop weight is recorded for every 15 cm (6 inches). The hammer is dropped from a distance of 0.76m (30 inches) providing 474.5 Joules per blow. When the sampler is driven a total of 45 cm (18 inches) into the soil, the standard penetration index ('N' Value) is the total number of blows for the last 30 cm (12 inches).

Dynamic Cone Penetration Resistance (DCPT): Dynamic Cone Penetration Resistance is similar to a SPT with the 474.5 Joule/blow impulse provided by the free falling hammer where the split-spoon sampler is replaced by a 51 mm Ø, 60° conical point and the number of blows is recorded continuously for every 30 cm (12 inches).

COHESIVE SOILS CONSISTENCY

	(kPa)	(P.S.F.)	Nominal 'N' Value
Very Soft	<12	<250	0-2
Soft	12-25	250-500	2-4
Firm	25-50	500-1000	4-8
Stiff	50-100	1000-2000	8-15
Very Stiff	100-200	2000-4000	15-30
Hard	>200	>4000	>30

RELATIVE DENSITY OF COHESIONLESS SOIL

	'N' Value
Very Loose	0-4
Loose	4-10
Compact	10-30
Dense	30-50
Very Dense	>50

MOISTURE CONDITIONS:

Cohesive Soil
DTPL- Drier than plastic limit
APL- About plastic limit
WTPL- Wetter than plastic limit
MWTPL- Much wetter than plastic limit

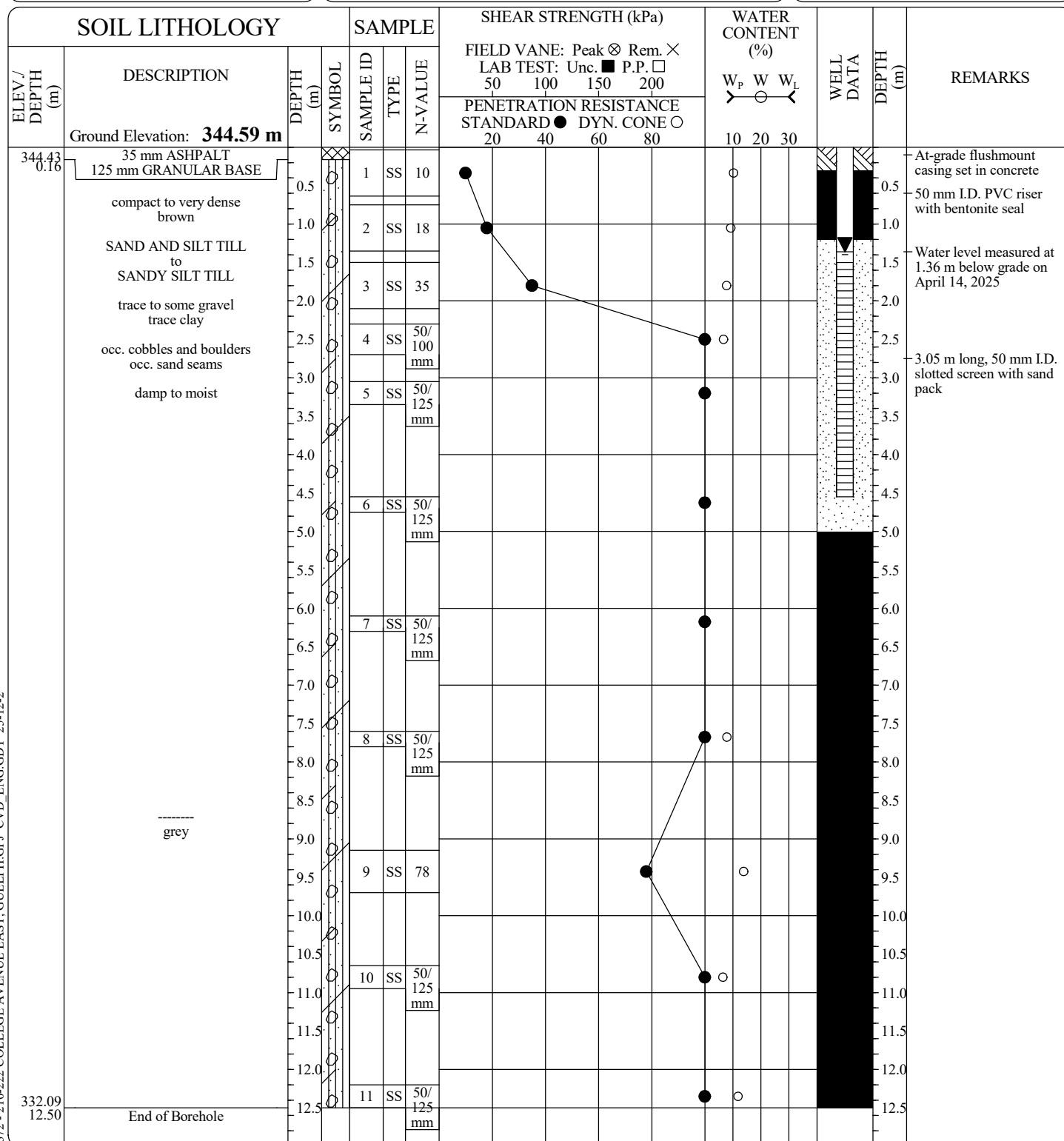
Cohesionless Soil
Damp
Moist
Wet
Saturated

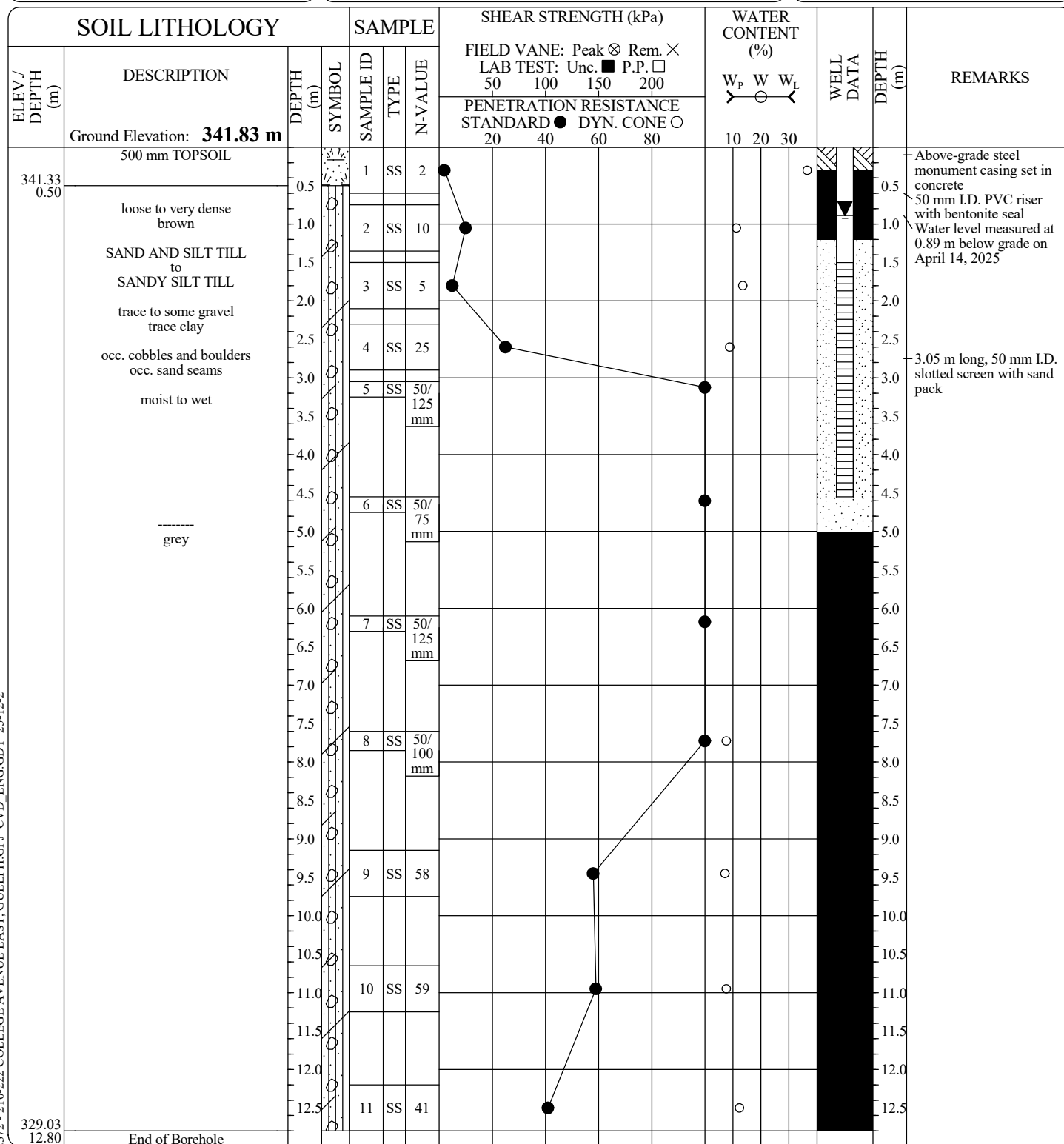
SAMPLE TYPES AND ADDITIONAL FIELD TESTS

SS	Split Spoon Sample (obtained from SPT)	GS	Grab Sample	PP	Pocket Penetrometer
AS	Auger Sample	BS	Bulk Sample	VANE	Peak & Remolded shear
		TW	Thin Wall Sample or Shelby Tube	DMT	Flat Plate Dilatometer

LABORATORY TESTS

SG	Specific Gravity	S	Sieve Analysis	W	Water Content
H	Hydrometer	P	Field Permeability	K	Lab Permeability
W_p	Plastic Limit	W_L	Liquid Limit	I_p	Plasticity Index
GSA	Grain Size Analysis	C	Consolidation	UNC	Unconfined compression

FILE No: 2372**BOREHOLE No. 1**Client: **EM Guelph Developments Limited Partnership**Project: **Proposed Residential Development**Location: **210-222 College Avenue East, Guelph****EQUIPMENT DATA**Machine: **CME 55**Method: **Hollow Stem Auger**Size: **83 mm I.D.**Date: **Apr 10 - 25 TO Apr 10 - 25**PROJECT MANAGER: **NZ****CHUNG & VANDER DOELEN
ENGINEERING LTD.**311 Victoria Street North
Kitchener, Ontario N2H 5E1
ph. (519) 742-8979, fx. (519) 742-7739

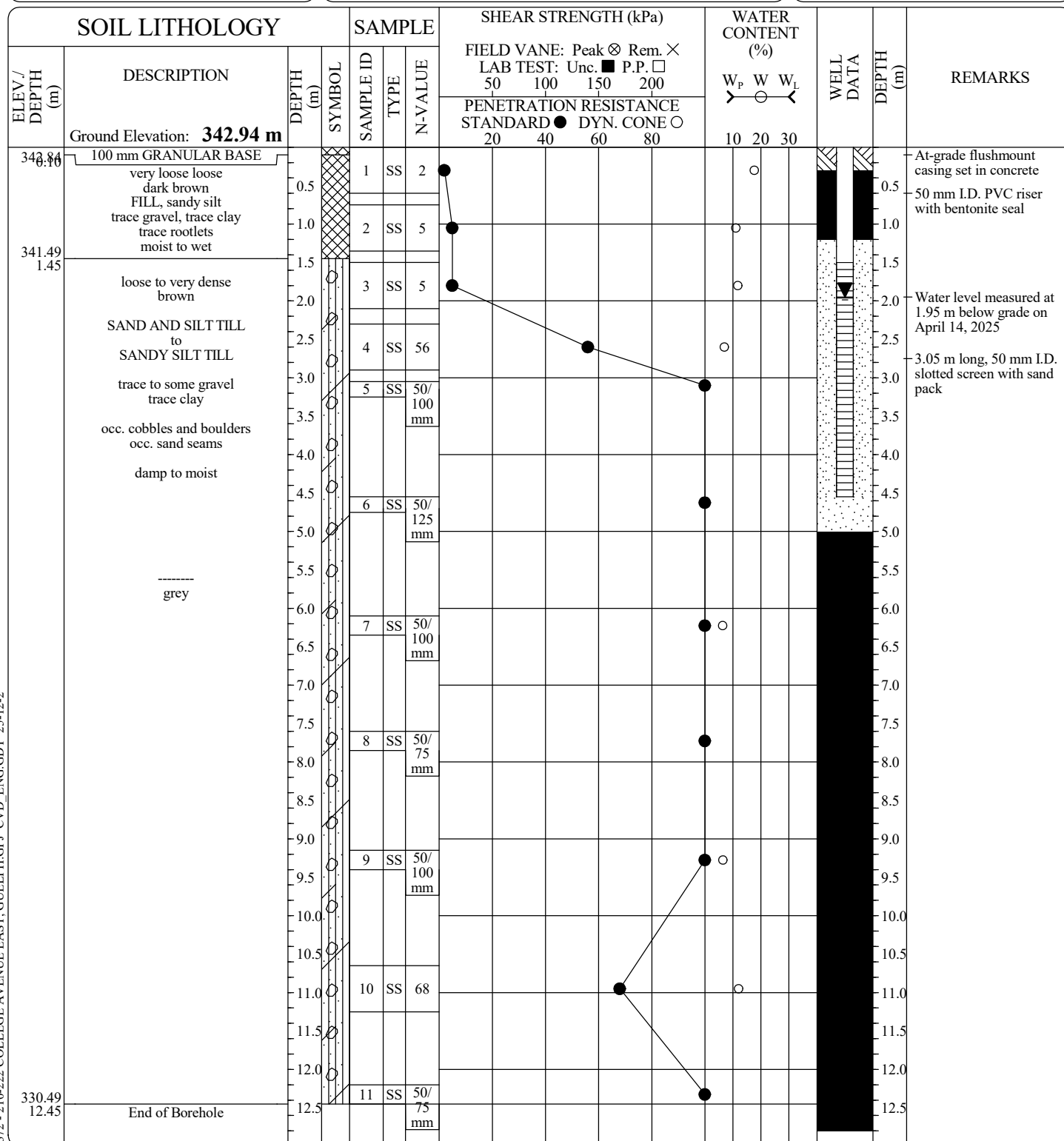
FILE No: 2372**BOREHOLE No. 2**Client: **EM Guelph Developments Limited Partnership**Project: **Proposed Residential Development**Location: **210-222 College Avenue East, Guelph****EQUIPMENT DATA**Machine: **CME 55**Method: **Hollow Stem Auger**Size: **83 mm I.D.**Date: **Apr 09 - 25 TO Apr 10 - 25**PROJECT MANAGER: **NZ****CHUNG & VANDER DOELEN
ENGINEERING LTD.**311 Victoria Street North
Kitchener, Ontario N2H 5E1
ph. (519) 742-8979, fx. (519) 742-7739

FILE No: 2372**BOREHOLE No. 3**

Client: **EM Guelph Developments Limited Partnership**
 Project: **Proposed Residential Development**
 Location: **210-222 College Avenue East, Guelph**

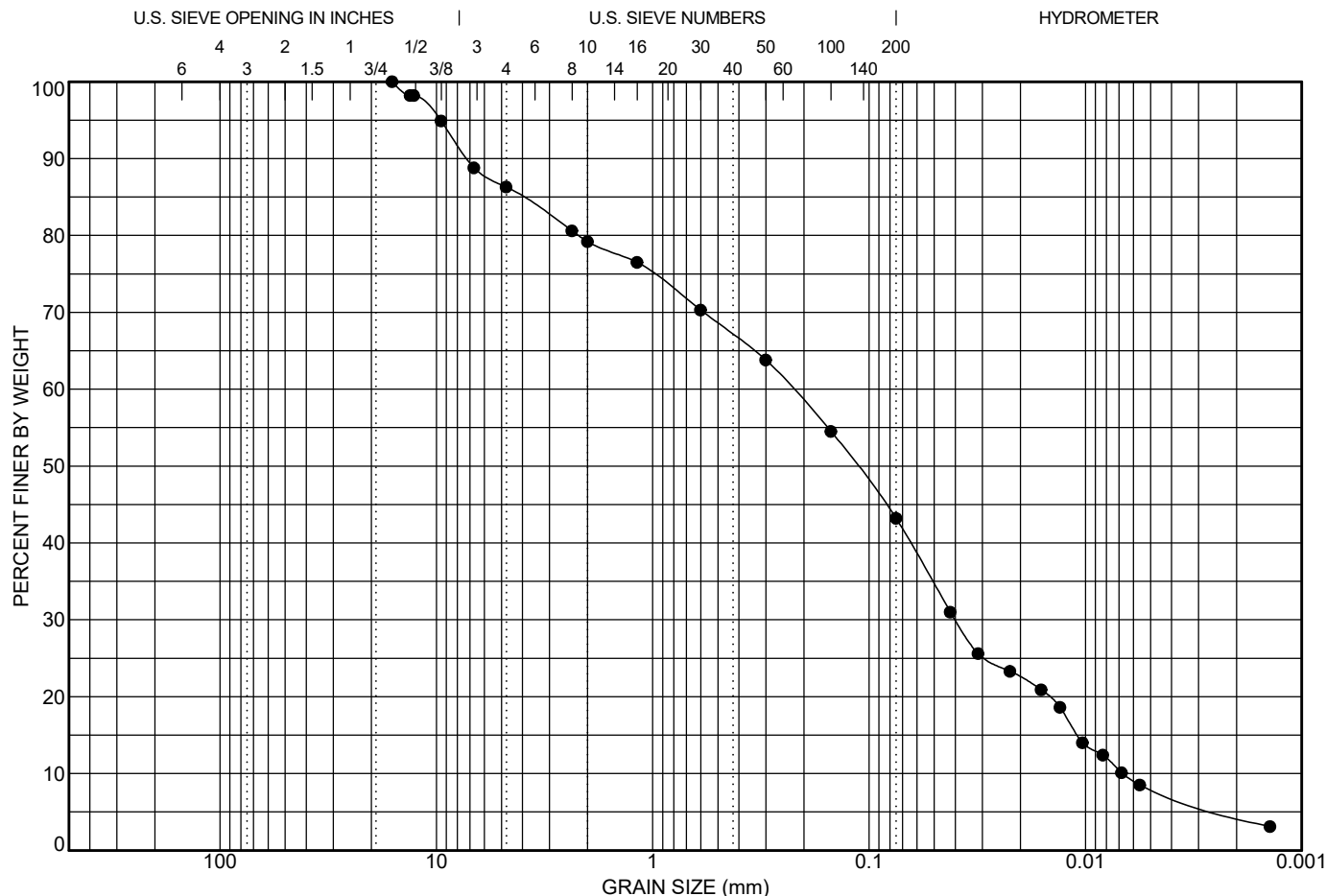
EQUIPMENT DATA

Machine: **CME 55**
 Method: **Hollow Stem Auger**
 Size: **83 mm I.D.**
 Date: **Apr 10 - 25 TO Apr 10 - 25**

PROJECT MANAGER: **NZ**

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COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

LL	PL	PI	Cc	Cu	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
			1.05	33.64	16	0.226	0.04	0.007	13.7	43.1		43.2

Date: May. 09 - 2025
Client: EM Guelph Developments Limited Partnership
Contractor:
Source:
Sampled From: BH1 - SA4; 2.3 to 2.9 m depth
Sample No.: 1-4
Date Sampled: Apr. 10 - 2025
Sampled By: JR
Lab No.: 407
Date Tested: May. 07 - 2025
Type of Material: Sand and Silt Till, some gravel, trace clay

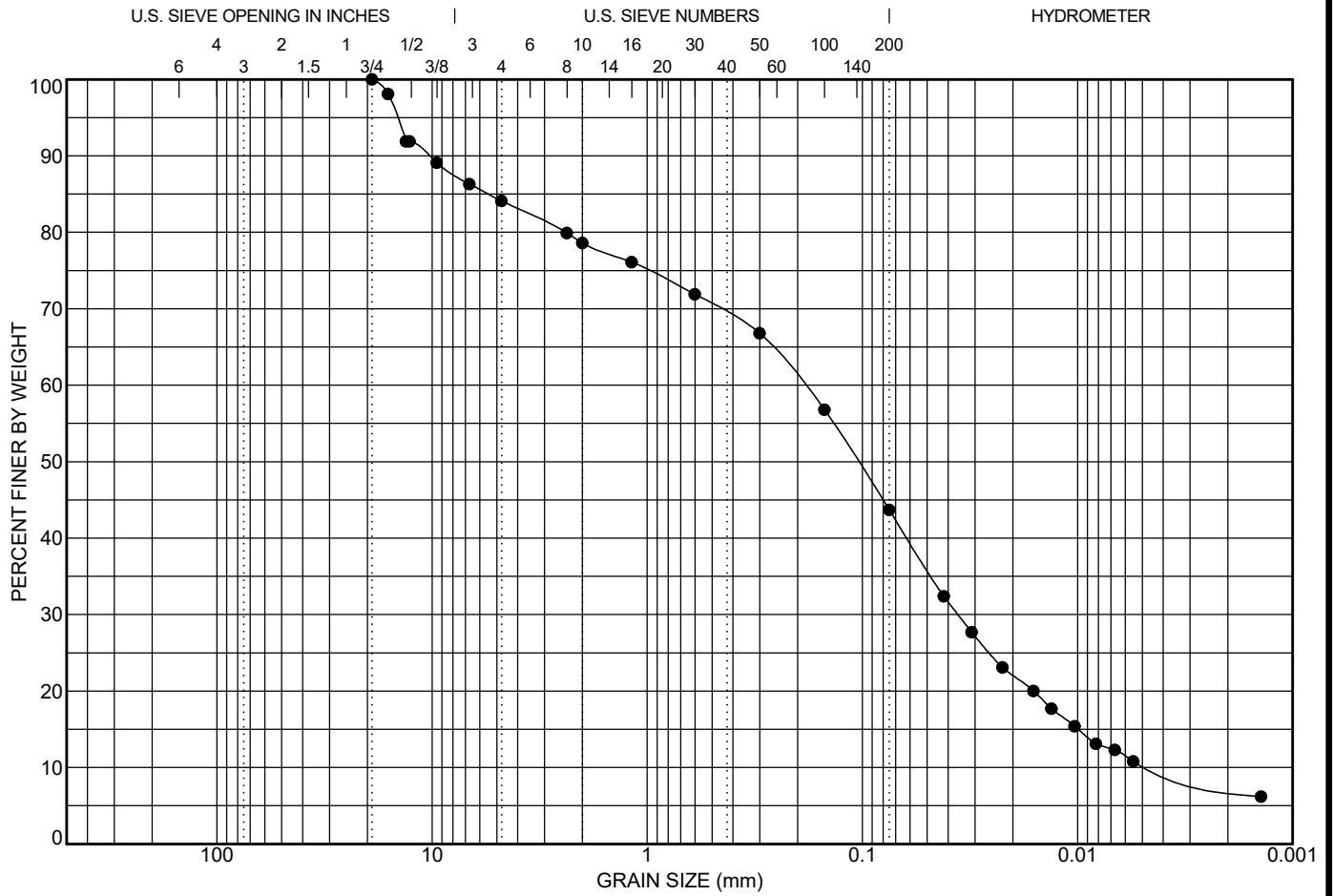
Sieve Size (mm)	Percent Passing	No Specifications



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 Fax: 519-742-7739
 e-mail: info@cvdengineering.com

GRAIN SIZE DISTRIBUTION

Project: Proposed Residential Development
Location: 210-222 College Avenue East, Guelph
File No.: 2372
Enclosure No.: 4



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

LL	PL	PI	Cc	Cu	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
			1.59	43.19	19	0.187	0.036	0.004	15.9	40.4	43.7	

Date: May. 09 - 2025
Client: EM Guelph Developments Limited Partnership
Contractor:
Source:
Sampled From: BH2 - SA3; 1.5 to 2.1 m depth
Sample No.: 2-3
Date Sampled: Apr. 10 - 2025
Sampled By: JR
Lab No.: 408
Date Tested: May. 07 - 2025
Type of Material: Sand and Silt Till, some gravel, trace clay

Sieve Size (mm)	Percent Passing	No Specifications



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 e-mail: info@cvdengineering.com

GRAIN SIZE DISTRIBUTION

Project: Proposed Residential Development
Location: 210-222 College Avenue East, Guelph
File No.: 2372
Enclosure No.: 5

