



# 190-216 Arkell Road Guelph, Ontario

## **Preliminary** **Stormwater Management Report**

### **Project Location:**

190 - 216 Arkell Road  
Guelph, Ontario

### **Prepared for:**

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Waterloo, ON N2V 2A2

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# 1.0 Introduction

## 1.1 Overview

MTE Consultants Inc. (MTE) was retained by Crescent Homes to prepare a Stormwater Management Report in support of a Draft Plan of Subdivision Application. The lands that comprise the Draft Plan of Subdivision are made up of a number of properties, including: 190, 202, 210, and 216 Arkell Road, located in the City of Guelph. These lands are herein referred to as the 'Subject Lands.'

The Subject Lands are approximately 2.58ha. Refer to **Figure 1.1** for the location of the Subject Lands. The proposed development plans for the site include a residential subdivision with 22 townhouse units fronting onto a municipal right-of-way, two stacked townhouse condominium blocks, and a stormwater management (SWM) block. The proposed right-of-way will connect the existing Dawes Avenue northeast of the Subject Lands and to Arkell Road at its intersection with Summerfield Drive. Refer to the Draft Plan of Subdivision prepared by MHBC, dated April 29, 2021, in **Appendix A** for more details. Approximately one third of the site cannot be developed due to the existing wetland and its setbacks.

This report presents stormwater quality, quantity, and erosion control measures that are proposed to be provided for the development. This report should be read in conjunction with the *190-216 Arkell Road – Functional Servicing Report*, prepared by MTE (December 3, 2021).

## 1.2 Background Information

The original Stormwater Management Report, prepared by MTE and dated October 10, 2018, was submitted to the City of Guelph (City) as part of Site Plan approval process. After discussions with City staff, it was determined that the proposed road connections through the site would establish a municipal right-of-way, thereby warranting a Draft Plan of Subdivision Application. As such, various departments within the City have reviewed the original submission and provided comments to be addressed prior to Draft Plan approval.

The revised Functional Servicing Report was submitted dated April 7, 2020. Comments were received from the City on January 4, 2021.

## 1.3 Purpose of Study

The purpose of this report is to address the most recent City comments and develop a comprehensive stormwater management strategy for the current development proposal that is acceptable to the City, the Grand River Conservation Authority (GRCA), and the Ministry of the Environment, Conservation and Parks (MECP).



CITY OF  
GUELPH

VICTORIA ROAD SOUTH



SUBJECT LANDS

ARKELL ROAD

TORRANCE CREEK  
WETLAND COMPLEX

GORDON STREET

FIGURE 1.1

Date: MAR.27/20  
Scale: N.T.S.

LOCATION PLAN



Engineers, Scientists, Surveyors

Project No.: 42063-104

## 1.4 Objectives

The objective of this stormwater management plan is to ensure that the proposed development includes the necessary controls to protect the hydrology and water quality of the receiving water systems. Furthermore, this plan also ensures that the proposed Draft Plan of Subdivision provides the necessary blocks and corridors for stormwater management measures. The primary objectives of this study are as follows:

- Establish criteria for the management of stormwater runoff from the study area;
- Recommend a comprehensive plan for controlling the quality and quantity of stormwater runoff from the study area;
- Perform monthly infiltration and water balances to analyse the effect of the development on local water systems; and
- Prepare preliminary designs for the recommended stormwater management infrastructure.

## 2.0 Existing Conditions and Background Information

### 2.1 Topographical Information

The Subject Lands consist of approximately 2.58ha and are generally bounded by an existing wetland to the northwest, an existing residential development to the northeast, the Arkell Road right-of-way to the southeast, and an existing single family residential property to the southwest. The Subject Lands are legally described as Part of Lot 6, Puslinch Concession 8 in the City of Guelph. They are currently comprised of four residential properties. Municipal addresses for the individual lots are 190, 202, 210, and 216 Arkell Road. The existing homes will be vacated and demolished prior to development.

MTE conducted a detailed topographical survey of the Subject Lands in November 2016. Existing site conditions and topography for the Subject Lands are shown in **Figure 2.1**, as well as the enclosed **MTE Drawing 42063-104-EC1.1**.

The Subject Lands are relatively flat, with slopes generally ranging from 0.5% to 1.5%. Existing elevations within the lands range from 333.3m along the wetland boundary to approximately 335.0m along Arkell Road. Under pre-development conditions, surface runoff from the site flows northwesterly towards the wetland complex.

### 2.2 Pre-Development Conditions

The Subject Lands are located within the Torrance Creek Subwatershed. The western portion of the property is comprised of the Torrance Creek Wetland, which lies at the headwaters of a tributary to Torrance Creek. Approximately one-third of the northerly portion of the site either lies within the wetland complex or within the required 30.0m wetland setback.

As previously mentioned, the majority of the site is internally drained and surface runoff flows northwesterly from Arkell Road to the wetland feature. **Figure 2.2** provides an illustration of the pre-development catchment areas.

Based on existing conditions, the site and surrounding lands were modelled as three separate catchments using the MIDUSS hydrologic modelling program. **Table 2.1** provides a brief description of the catchments and the design parameters used in the hydrologic modelling.

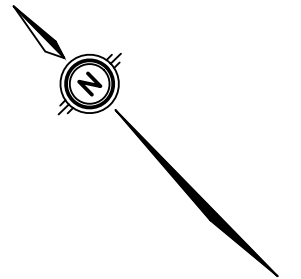
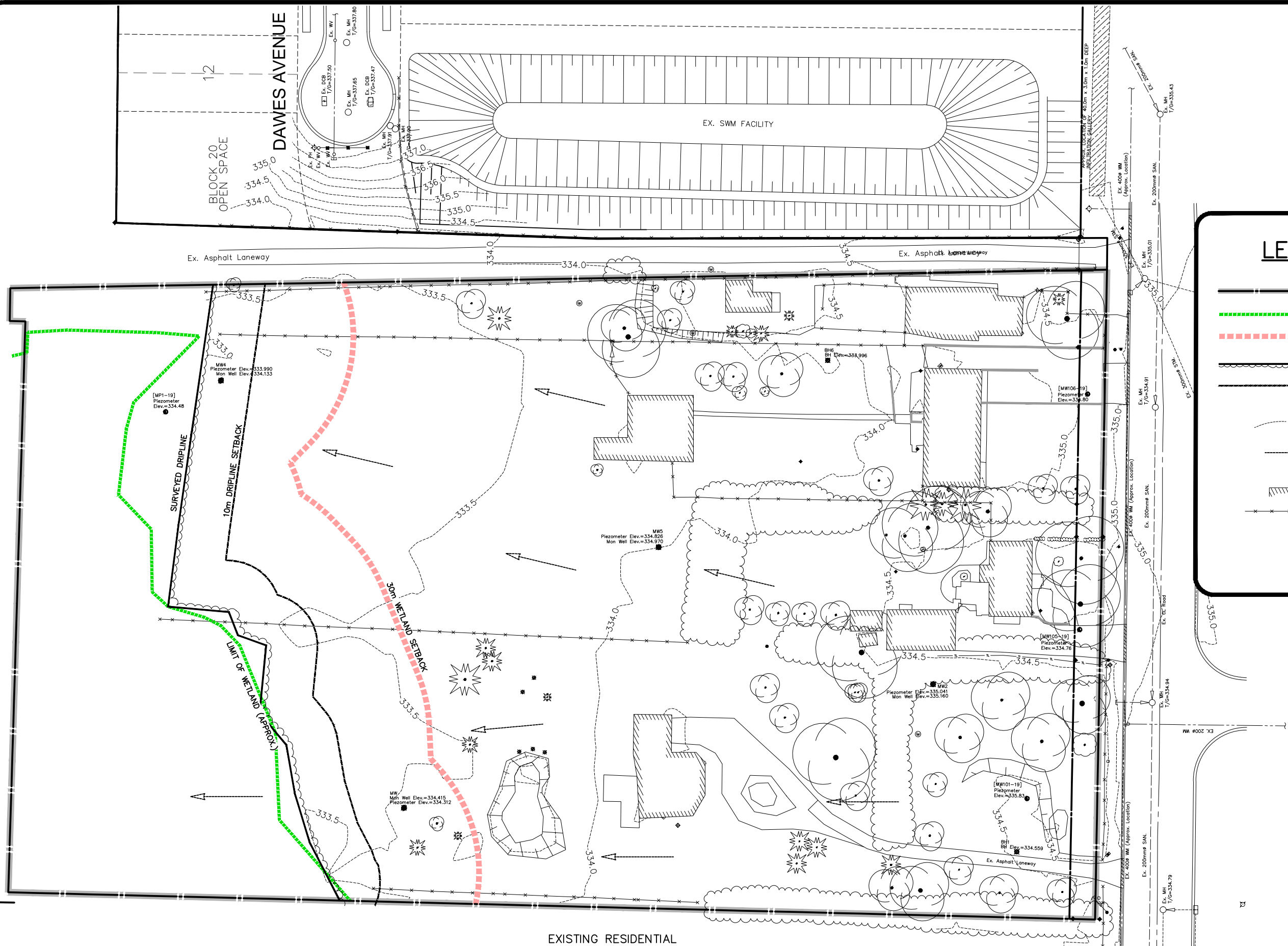


Hydrologic modelling details and results are further discussed in Section 5. A detailed copy of the pre-development catchment parameters and MIDUSS modelling output logs has been included in **Appendix B**.

**Table 2.1 – Pre-Development Catchment Parameters**

Catchment	Description	Area (ha)	% Impervious	Flow Length (m)	Slope (%)
<b>Within Subject Lands</b>					
101	Existing residential properties and Arkell Road boulevard	1.714	16	150	0.5
102	Existing wetland and setbacks	0.863	0	50	0.5
<b>Outside of Subject Lands</b>					
103	Private laneway adjacent to subject lands	0.240	30	225	0.8
104	Existing SWM facility embankments	0.234	0	8	20
105	Driveway aprons and ditch within Arkell right-of-way	0.057	20	125	0.5
	<b>Total</b>	<b>3.108</b>	<b>11.5</b>	<b>-</b>	<b>-</b>

Please note that drainage area 103 is equivalent to drainage area 205 as defined within the Stantec Report *220 Arkell Road Preliminary Servicing, Grading, and Stormwater Management Report* (May 28, 2019). Drainage area 104 represents the existing SWM facility embankment within the adjacent Arkell Meadows Subdivision that is directed towards the subject lands.



# LEGEND

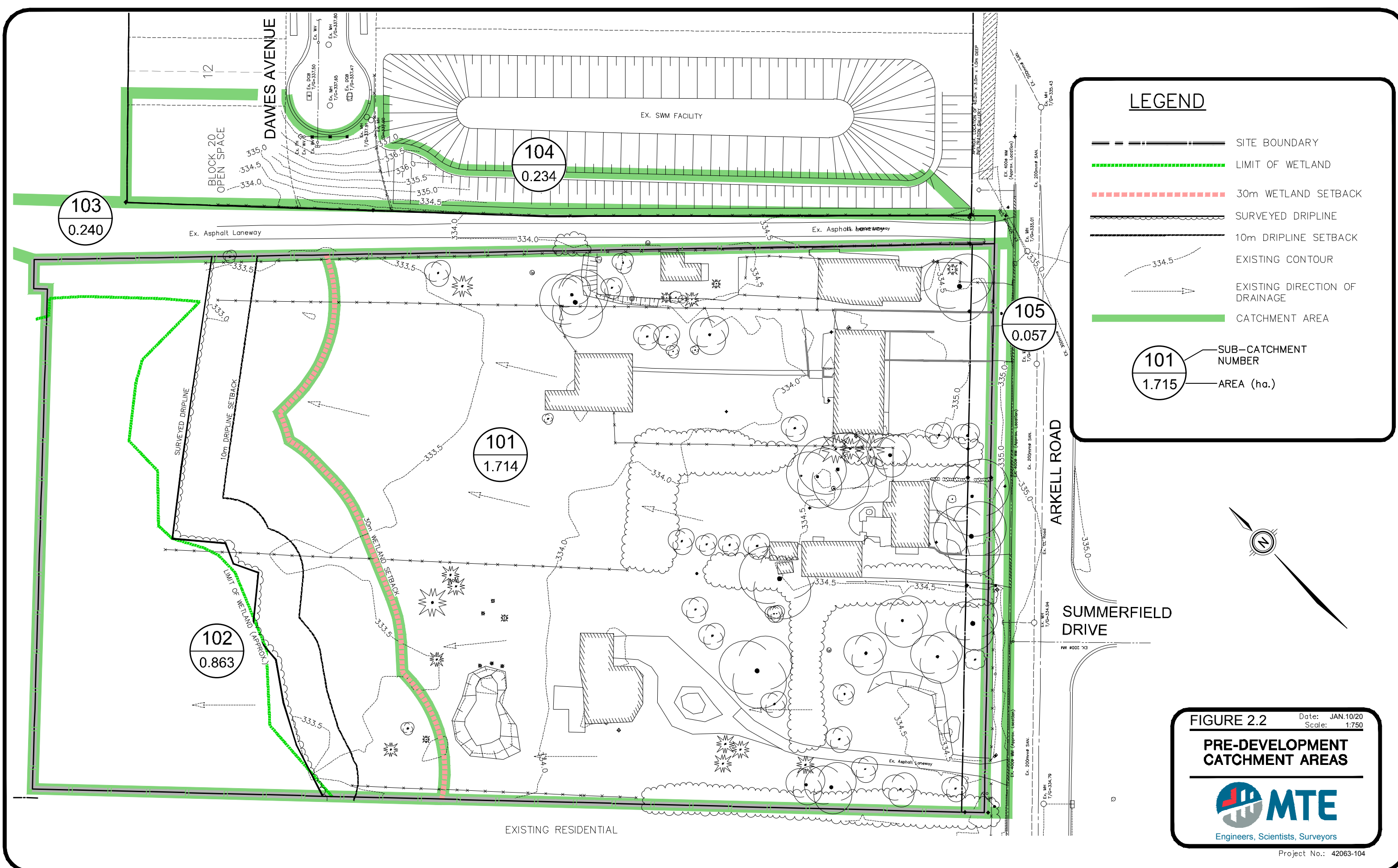
- SITE BOUNDARY
- LIMIT OF WETLAND
- 30m WETLAND SETBACK
- SURVEYED DRIPLINE
- 10m DRIPLINE SETBACK
- EXISTING CONTOUR
- EXISTING DIRECTION OF DRAINAGE
- EXISTING BUILDING
- EXISTING FENCE
- BOREHOLE
- MONITORING WELL

FIGURE 2.1 Date: OCT.29/21  
Scale: 1:750

## EXISTING CONDITIONS PLAN



Project No.: 42063-104



**FIGURE 2.2** Date: JAN.10/20 Scale: 1:750  
**PRE-DEVELOPMENT CATCHMENT AREAS**  
**MTE**  
Engineers, Scientists, Surveyors  
Project No.: 42063-104

## 2.3 Geotechnical Information

A geotechnical investigation for the property was carried out by Peto MacCallum Ltd. (Peto) in 2017. A total of six (6) boreholes were advanced to depths of approximately 6.6m - 8.1m. A final version of their report, entitled *Geotechnical Investigation Proposed Arkell Road Subdivision* dated October 1, 2018, was prepared. The assessment made recommendations pertaining to the site grading, road pavement structure, stormwater management facilities, and general house construction.

Based on the results of Peto's geotechnical investigation, the subsurface stratigraphy of the site generally consists of topsoil and some localized fill overlying native deposits of silt, sand, and gravel. A thick (~3.6m) layer of silt was encountered 2.2m below existing grades on the easterly portion of the site.

The report also provided an infiltration rate of 30mm/hr for typical soils present within the Subject Lands. For further geotechnical information, refer to Peto's geotechnical investigation in **Appendix F**.

## 2.4 Hydrogeological Information

A hydrogeological investigation was conducted by MTE starting in 2017. A supplemental technical memo *Update to Hydrogeological Investigation Report as per City of Guelph Comments* was completed dated January 9, 2020. Monitoring wells were installed in four of the boreholes which were previously advanced by Peto. MTE has conducted continuous groundwater monitoring since March of 2017. The highest water table was observed in May of 2017 and ranged from an elevation of 334.0m in the northern portion of the site (MW4) to 333.2m in the southern portion of the site (MW2). These elevations represent depths of 0.9m above existing grade at MW4 and 1.1m below existing grade at MW2. The measured groundwater elevations indicate that the shallow groundwater flows from the north to the south (i.e. away from the wetland in the north portion of the Site).

A revised Hydrogeological Assessment was completed dated December 3, 2021.

Please note that MTE has set the basement floor elevations above the seasonal high groundwater in order to achieve a minimum 1.0m separation.

Please refer to **Appendix G** for a figure depicting the Subject Lands' seasonal high groundwater contours.

### 2.4.1 In-Situ Infiltration Testing

In-situ infiltration testing was performed as part of the Hydrogeological Investigation.

MTE completed test pit and in-situ infiltration testing at the Subject Lands on November 19, 2021. Four test pits (TP101-21 through TP104-21) were advanced at the Subject Lands for infiltration testing purposes using a mini-excavator operated by Steve Neeb of Neeb Excavating Inc., which was observed by MTE. These test pits are shown on **MTE Drawing 42063-104-EC1.1**.

Infiltration tests were completed using a Soil Moisture 2800 K1 Guelph Permeameter in 0.05 m diameter x 0.16 to 0.20 m deep boreholes which were hand augered through the base of the test pit bottom in native overburden sediments in which the permeameter base tip was placed. Water levels within the combined reservoir of the Guelph Permeameter were recorded at regular time intervals to obtain time-varying infiltration rates of the sediment unit being tested.

The field saturated hydraulic conductivity (Kfs) of the tested materials was calculated using the Guelph Permeameter K-sat calculator, available for download on the soil moisture website (soilmoisture.com).

Based on the field measurements, a Kfs has been calculated for each of the tested locations, summarized in **Table 5.5** below.

**Table 5.5: Field Saturation (Kfs) Summary (mm/hr)**

Test Pit	Depth (mbgs)	Soil Type	Median Kfs <sup>1</sup> (cm/sec)	Median Kfs (mm/hr)
TP101-21	1.0	Silty SAND	$8.9 \times 10^{-5}$	3
TP101-21	1.6	SAND, trace silt, trace gravel	$3.5 \times 10^{-4}$	13
TP102-21	0.8	SAND and GRAVEL	$5.8 \times 10^{-3}$	209
TP103-21	0.5	SAND and GRAVEL	$5.4 \times 10^{-3}$	194
TP104-21	0.9	SAND and GRAVEL	$4.3 \times 10^{-3}$	155

It should be noted that the average infiltration rate across the site (38.3mm/hr with a safety factor of 3) exceeds the 30mm/hr rate provided by Peto. For the purpose of this report, it was determined that a design infiltration rate of 30mm/hr will be utilized for a conservative design scenario. The infiltration rate used can be reevaluated at the time of detailed design if required.



### 3.0 Stormwater Management Criteria

New developments are required to provide stormwater management in accordance with provincial and municipal policies. Relevant documents have been referenced in the design of the stormwater management plan for the Subject Lands; including:

- GRCA Policies for the Administration of Ontario Regulation 150/06 (GRCA, 2015);
- Stormwater Management Planning and Design Manual (MOE, 2003);
- The Torrance Creek Subwatershed Study (TCSS) Management Strategy (GRCA, 1999);
- Design Principles for Stormwater Management (City of Guelph, 1996); and
- The City of Guelph's Development Engineering Manual (City of Guelph, 2019).

Based on the above policies and relevant documents, background reports, agency requirements, and site specific considerations, the following stormwater management criteria have been established for this study area:

- **Water Quality** – Provide an Enhanced (MOE, 2003) level of stormwater quality treatment prior to discharge to surface and groundwater systems.
- **Water Quantity** – Control the peak flow rates for all storms up to and including the 100-year storm event to the allowable pre-development rates; preserve hydraulic and hydrologic functions. Provide erosion control by maintaining existing flow duration characteristics.
- **Erosion Control** – Minimum 12 h extended retention of the 4h 25mm Chicago distribution rainfall event due to small overall drainage area (< 8ha) per MOE Section 4.6.2.
- **Thermal Impact** – Torrance Creek subwatershed supports cold water fish habitats, and therefore thermal impact assessment required with preventive and mitigation measures.
- **Water Balance**
  - Infiltration* – Maintain or exceed target groundwater volume inputs established within the TCSS through active and/or passive infiltration measures.
  - Surface Water* – Maintain existing surface water volume inputs into significant environmental features.

A brief discussion of each of these criteria is included in the following subsections.

#### 3.1 Water Quality Control

The City's Engineering Design Guidelines and the TCSS state that all new developments shall provide an Enhanced (Level 1) level of quality treatment. The requirements for this level of quality control are established in Table 3.2 of the *Stormwater Management Planning and Design Manual* (MOE, 2003). The TCSS also identifies a water management objective of minimizing impacts to the receiving waterbody by establishing additional quality targets for development within the subwatershed, as follows:

- **Nutrients** – Total phosphorus should be 30ug/L or less; the use of the quality wet cell will reduce suspended solids and nutrients;
- **Dissolved Oxygen** – Dissolved oxygen (DO) concentration should not be less than 4mg/L during summer months. Reduction of temperature and nutrient concentrations will improve DO levels. Aeration of direct runoff may also be helpful; and



- **Temperature** – Temperatures within Torrance Creek should be below 25°C. New developments can mitigate temperature increases by maximizing infiltration and using underground drainage elements before discharging to surface water.

### 3.2 Water Quantity and Erosion Control

The primary objective of quantity control is to maintain hydraulic and hydrologic functions from existing conditions with regards to both surface and subsurface flows. As such, the Subwatershed Plan requires future development within the Torrance Creek Subwatershed to maintain post-development peak flows at existing levels for all storms up to and including the 100-year event. For the purpose of this analysis, controls for the Regional storm event will also be included. Furthermore, existing major flow paths are to be maintained to provide overland flow under major flood events.

It should be noted that the TCSS states that controls should provide at least a 24-hour drawdown period for the 25mm storm event. However, the drainage area towards the proposed SWM facility is considered small (< 8ha), so the minimum detention time required is 12 hours as described in Section 4.6.2 of the MOE SWM manual. Controls have been adjusted within the facility to maximize drawdown as much as possible. With a 50mm diameter orifice placed at permanent pool, a drawdown time of 23.0 hours is achieved.

### 3.3 Water Balance

#### 3.3.1 Infiltration to Groundwater

The City requires that Low Impact Design (LID) best management practices be used to mimic pre-development recharge rates. Infiltration galleries are proposed to direct flows from roofs on-site wherever possible. The SWM facility will introduce an infiltration cell to further promote groundwater recharge. Additionally, increasing the amount of pervious landscaped areas throughout the Subject Lands will improve groundwater recharge by means of passive infiltration.

The TCSS divided the subwatershed into three stormwater management areas, with respect to groundwater recharge, and established specific infiltration targets for each. The Subject Lands fall within Area 2 (Arkell Road to Torrance Creek) and a minimum infiltration target of 150mm/year is recommended. Baseflow enhancement is encouraged on lands within this zone, especially if they are close to the creek.

An infiltration balance was performed to determine the pre-to-post development net change in infiltration, provided in **Appendix E**.

#### 3.3.2 Surface Runoff

The City requires a monthly surface runoff water balance analysis to maintain existing surface water volume inputs into significant environmental features (ie. the wetland).

### **3.4 Monitoring**

As per the TCSS, both the performance of the proposed SWM facility and its effect on the adjacent wetland and shallow groundwater table must be monitored.

#### **3.4.1 Facility Monitoring**

Upon completion of construction, the SWM facility is to be inspected to ensure conformance to the design. Upon confirmation from Municipal staff that the facility is acceptable, it will be monitored for a period of 2 years under the ownership of the developer to confirm the performance of the facility meets the quantity and quality targets. Upon completion of the monitoring program, a monitoring report shall be submitted to the Municipality for approval prior to the assumption of the facility.

The SWM facility will be monitored for peak flow rates and flow durations, water levels and drawdown times, pollutant removal efficiency, and the quality/toxicity of the water discharging to the wetland complex. The facility performance should be compared to the theoretical (design) performance and any aberrations should be noted. Remedial action will be required if facility performance does not meet minimum requirements.

#### **3.4.2 Groundwater Monitoring**

Groundwater levels and quality shall be monitored. MTE has completed over 2 years of continuous monitoring of the groundwater levels; which will form the basis against which post-construction levels will be compared. It is recommended that groundwater samples be collected and tested for quality to provide a basis against which post-construction quality results may be compared. Post-construction quality samples should be taken from the shallow groundwater downstream of the SWM facility.

#### **3.4.3 Wellhead Protection**

The subject property is located near the City's Burke Well. Due to its close proximity, the southwestern portion of the Subject Lands fall within the well's 100.0m capture zone. As such, this area has been assigned a Wellhead Protection Area vulnerability score of 10. The remainder of the site; located outside of the 100.0m capture zone, is assigned a vulnerability score of 6 to 8. Therefore, it can be concluded that surface runoff and infiltrated runoff generated from the Subject Lands may need to be properly managed in order to protect the surrounding surface water and groundwater quality. Any on-site infiltration measures will be outside of the 100.0m capture zone.

During the detailed design stage of development, as well as during the respective Site Plan Application processes of the two condominium blocks, adequate measures may need to be implemented to satisfy the criteria set forth by the Drinking Water Source Protection Program. An example of such measures would be to include a liner, either synthetic or clay, to protect the base of the SWM facility. This liner would serve to prevent chloride laden stormwater, present in the WET pond, from infiltrating directly to the groundwater below. Additionally, salt application and snow storage plans may need to include specific strategies to minimize adverse effects to the groundwater supply. In more severe cases, additional measures may include strict restrictions on specific land uses, construction or operational activities, or the use and storage of certain materials.

## 4.0 Proposed Development and SWM Strategy

### 4.1 Proposed Area Grading

The grading design of the site was controlled by many factors, which include: servicing constraints (both sanitary and storm), matching existing and proposed boundary grades around the perimeter of the property, ensuring major storm event overland flows are directed towards the existing road right-of-ways and to the proposed SWM facility, minimizing impacts to the adjacent environmental feature, as well as, reducing the cut/fill deficit for the development. A preliminary finished grade contour plan illustrating site grading, **MTE Drawing 42063-104-AG1.1** is enclosed.

The Draft Plan of Subdivision includes two multi-residential stacked townhome development blocks. For the purpose of this report, their current conceptual designs have been incorporated into the overall SWM strategy for the subject lands. This will allow SWM criteria to be established for the two blocks and provide flow targets to which the blocks' respective SPA processes will have to adhere.

### 4.2 Post-Development Conditions

The proposed SWM plan implements a two-cell SWM facility with a wet cell and infiltration cell designed to accommodate stormwater runoff from the majority of the developed portions of the Subject Lands. The plan has been designed to meet the criteria presented in Section 3.0 of this report. An overview of the stormwater management plan is discussed below.

The proposed development lands are comprised of residential land uses, a road right-of-way and a proposed SWM facility block. Blocks 1 and 2 will be developed through respective Site Plan Application processes and will require SWM Briefs, as well as grading and servicing designs in support of SPA. This information will identify the stormwater management criteria for the block and how the stormwater control measures will adhere to the Draft Plan of Subdivision SWM criteria as established in this report.

The location of the proposed SWM facility, along with contributing drainage areas, is illustrated in **Figure 4.1**. Minor storm runoff from the controlled portions of the contributing areas will be conveyed through the proposed storm sewer system to the proposed SWM facility. Roof areas from Blocks 1 and 2 will be directed to infiltration galleries prior to any overflow being released to the SWM facility. Excess runoff from the major storms will flow overland to the SWM facility via the proposed right-of-way and designated overland flow routes. Refer to MTE's *190-216 Arkell Road Functional Servicing Report*, dated December 3, 2021, for further details of the storm sewer network.

The proposed SWM facility will utilize a wet cell and infiltration cell design. These measures will provide quality and quantity control of runoff prior to discharge into the adjacent Torrance Creek wetland. The contributing drainage area to the SWM facility (1.201ha) is separated into seven catchments, and are described below (catchments 201-1 to 201-7).

Under post-development conditions, the Subject Lands and adjacent areas were delineated into sixteen catchments. **Table 4.1** provides a brief description of each catchment area as well as the design parameters used in the hydrologic modelling. A detailed copy of the post-development catchment parameters and MIDUSS modelling output logs has been included in **Appendix C**.

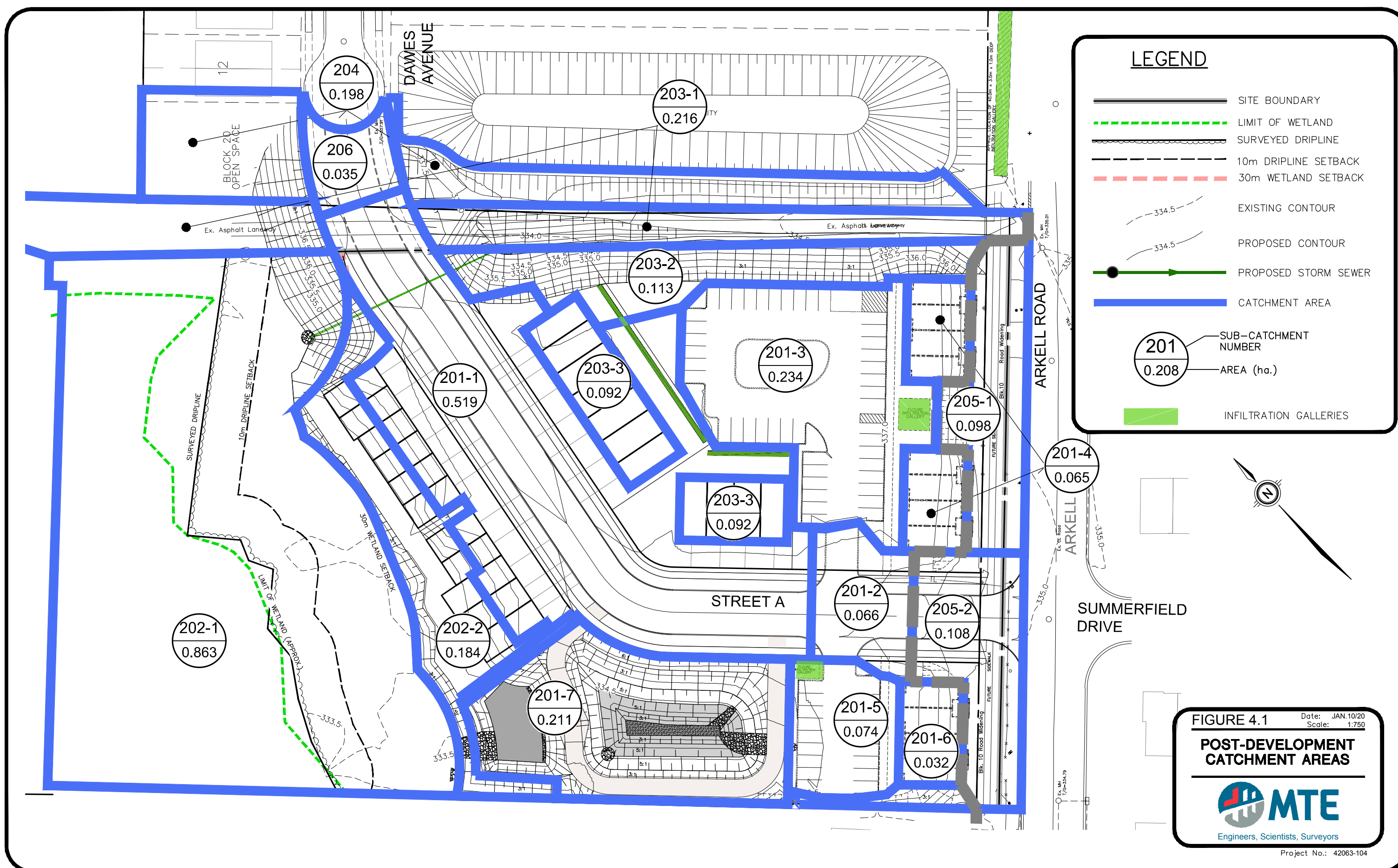
**Table 4.1 – Post-Development Catchment Parameters**

Catchment	Description	Area (ha)	% Impervious	Flow Length (m)	Slope (%)
<b>To SWM Facility</b>					
201-1	Street A, front yards, driveways	0.519	60	60	0.8
201-2	Street A portion	0.066	70	15	0.5
201-3	Block 1 non-roof areas	0.234	75	15	2.0
201-4	Block 1 roofs	0.065	100	10	2.0
201-5	Block 2 non-roof areas	0.074	85	15	2.0
201-6	Block 2 roofs	0.032	100	10	2.0
201-7	SWM Facility	0.211	40	10	10
<b>Directly to Torrance Creek (Within Subject Lands)</b>					
202-1	Wetland and buffers	0.863	0	50	0.5
202-2	Rear yards and roofs to Torrance	0.184	24	10	2.0
<b>To Torrance Creek through future parklands</b>					
203-1	Future parklands trail	0.216	30	130	2.0
203-2	Grassed areas in subject lands to trail	0.113	0	10	33
203-3	Roof areas to galleries, overflow to ditch inlet	0.092	100	10	2.0
204	Ex. adjacent SWM facility embankment	0.198	0	8	20
<b>To Arkell Road (Ultimately to Torrance Creek)</b>					
205-1	Embankments and right-of-way north to existing infiltration gallery	0.098	25	15	2.0
205-2	Embankment and right-of-ways south to existing stone energy dissipators	0.108	40	25	5.0
<b>To Adjacent Ex. SWM Facility</b>					
206	To existing adjacent SWM Facility	0.035	70	20	1.3
	<b>Total</b>	<b>3.108</b>	<b>34.4</b>	<b>-</b>	<b>-</b>

For a majority of the developed lands, stormwater runoff will drain internally through the use of constructed drainage swales and the proposed storm sewer network. However, it should be noted that runoff from small portion of the developed Subject Lands (largely sloped pervious areas) will flow uncontrolled elsewhere.

There is an existing high point along Arkell Road near the driveway entrance to the 202 Arkell Road property. East of the high point (catchment 205-1), flows are directed towards storm sewers connected to an existing 40.0m long x 3.0m wide x 1.0m deep infiltration gallery located in the boulevard adjacent to the Arkell Meadows subdivision SWM facility.

On the western side of the high point (catchment 205-2), flow will be directed to an existing side inlet catchbasin approximately 155.0m away, through a stone energy dissipater, and eventually into the Torrance Creek wetland complex. As such, flow generated from uncontrolled portions of the Subject Lands will ultimately contribute to recharging surface water inputs to the wetland feature and subsurface water inputs to the local groundwater table.





## 5.0 Stormwater Management Design

### 5.1 Hydrologic Modelling

As previously noted, a post-development hydrologic model was constructed, using the MIDUSS modelling software, to reflect the detailed drainage conditions proposed for the Subject Lands. This allows for the quantitative estimate of flows under the proposed development conditions. The proposed development conditions were modelled for the:

- Quality storm event (25mm depth, 4-hour Chicago distribution);
- 2, 5, 10, 25, 50, and 100-year return period rainfall events (3-hour Chicago distribution derived from the City's Intensity-Duration-Frequency (IDF) parameters); and
- Regional storm event (285mm depth, 48-hour Hurricane Hazel).

The IDF parameters, hydrologic parameters, and MIDUSS model output files for each of the pre- and post-development catchment areas are provided in **Appendix B** and **Appendix C**, respectively.

### 5.2 Water Quality

The proposed SWM facility has been designed as a wet pond with a permanent pool of 1.2m, followed by an infiltration cell. A planting scheme will be prepared that carefully selects plant species and their location in and around the pond and swale to stabilize banks, mitigate temperature increases, deter waterfowl from nesting within the area, and provide aesthetics and safety benefits.

Since the majority of annual rainfall occurs in storms less than or equal to a 25mm event, the majority of water borne sediment is also transported to the SWM facility in these less intense events.<sup>1</sup> Therefore, the wet cell is designed as a forebay to target the smaller flows prior to discharging into the infiltration cell. An OGS unit is also proposed immediately upstream of the forebay to provide a 'treatment train' approach that will provide the required Enhanced (Level 1) quality treatment as required by the City of Guelph and Ministry of Environment, Conservation, and Parks.

The OGS unit is designed to treat runoff from minor events (i.e. events  $\leq 25\text{mm}$ ) before releasing flows to the SWM facility. Flows from events greater than the 25mm storm may bypass the OGS unit. Per the City's standards, the proposed OGS (model EF8) unit has been verified by the Canadian Environmental Technology Verification Program and has been sized to provide at least 60% TSS removal. A detailed sizing report for the OGS unit is included in **Appendix D**.

The wetcell/forebay design is based on classic particle settling and flow dispersion equations, as presented in the MOE's 2003 *Stormwater Management Planning and Design Manual*. The methodology presented in that document suggests that the design flow for the forebay should be taken as the peak outflow from the facility. A forebay is typically designed to treat minor storm flows. As such, the design of the forebay should be based on the notion that the flow into the forebay equals the flow through the forebay, which equals the flow out of the forebay. In using this approach, the recommended settling velocity of 0.0003 m/s (from MOE 2003) results in extremely large and unachievable forebay lengths. Therefore, the forebay is designed to satisfy the following four conditions:

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<sup>1</sup> From MOE-1994, Figure C.1: 62% less than 5mm, 78% less than 10mm, 90% less than 15mm, 95% less than 25mm

- A settling length based on a settling velocity of 0.0003 m/s using the main pond's peak discharge from the 25 mm storm event (as per MOE 2003);
- A settling length based on a settling velocity of 0.0055 m/s using the forebay inflow/outflow from the 25 mm storm event;
- A dispersion length such that, based on flow and depth of water, the velocity through the forebay is less than 0.5 m/s; and,
- That velocity, based on flow divided by cross-sectional area, is less than 0.15m/s to prevent scouring.

The 2003 MOE document suggests that the clean-out frequency for a SWM facility be based on the sediment loading within the entire pond, however, it is recommended that the clean-out frequency be based on the loading within the forebay only. While this typically results in more frequent clean-out, it is restricted to the forebay area only and eliminates disturbance of the main pond. The clean-out frequency for the proposed SWM Facility can be found in the forebay design calculations in **Appendix D**.

The total drainage area to the facility is 1.201ha at 64.7% imperviousness. According to Table 3.2 from MOE's 2003 stormwater management guidelines, the infiltration cell requires 32.4 m<sup>3</sup>/ha infiltration storage; the water quality cell requires 212.6 m<sup>3</sup>/ha of storage volume. As described in Section 3.1, 40 m<sup>3</sup>/ha of which is extended detention and the remainder of which is permanent pool. As such, the required extended detention volume is 48.0 m<sup>3</sup>, and a permanent pool volume of 207.3m<sup>3</sup>. Please note that since the infiltration cell is closed for the winter, the attenuation within the main cell is designed for the full drainage area.

The drainage area towards the proposed SWM facility is considered small (< 8ha), so the minimum detention time required is 12 hours. As described in Section 4.6.2 of the MOE SWM manual, a minimum orifice size of 50mm is acceptable when the primary outlet is a perforated CSP riser. With a 50mm diameter orifice placed at permanent pool, a drawdown time of 23.0 hours is achieved.

The proposed SWM facility design characteristics are summarized in **Table 5.1**. Refer to **Appendix D** for the relevant design sheets and calculations (e.g. catchment parameters, imperviousness calculations, stage-storage discharge relationships, drawdown calculations, etc.). Refer to **Figure 5.1** to **Figure 5.3** for details of the proposed SWM facility.



**Table 5.1 – Water Quality Control Details**

General	Facility Characteristics
Stormwater Management Facility Type	Wet Pond/Infiltration Cell
Required MECP Water Quality Protection	Enhanced (Level 1)
Total Contributing Area	1.201ha
Imperviousness	64.7%
Bottom Elevation (Wet Cell)	333.20m
<b>Storage</b>	
<i>Quantity and Erosion Control</i>	
Drawdown Volume (based on 25mm-4hr event)	119m <sup>3</sup>
Approximate Drawdown Time (based on 25mm-4hr event)	22.5 hours
Peak Release Rate (based on 25mm-4hr event)	0.003m <sup>3</sup> /s
<b>Outlet Controls</b>	
<i>1500mm diameter Perforated CSP Riser Manhole</i>	
Orifice 1 Diameter	50mm Vertical
Orifice 1 Elevation	334.40m
Orifice 2 Diameter	200mm Vertical
Orifice 2 Elevation	334.70m
Overflow Weir (Bottom Length / Side Slope)	1.0m / 10:1
Overflow Weir Elevation	335.30m

# LEGEND

- PROPERTY BOUNDARY
- FINISHED GRADE CONTOURS
- PROPOSED SPOT ELEVATIONS
- MAJOR OVERLAND FLOW ROUTE
- MAINTENANCE ACCESS
- 300mm GABION MAT
- PROPOSED STORM SEWER
- 30m WETLAND SETBACK

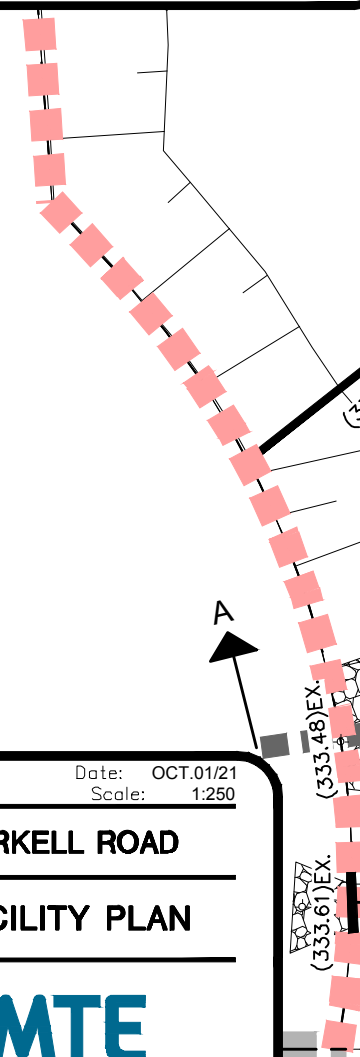


FIGURE 5.1

Date: OCT.01/21  
Scale: 1:250

190-216 ARKELL ROAD

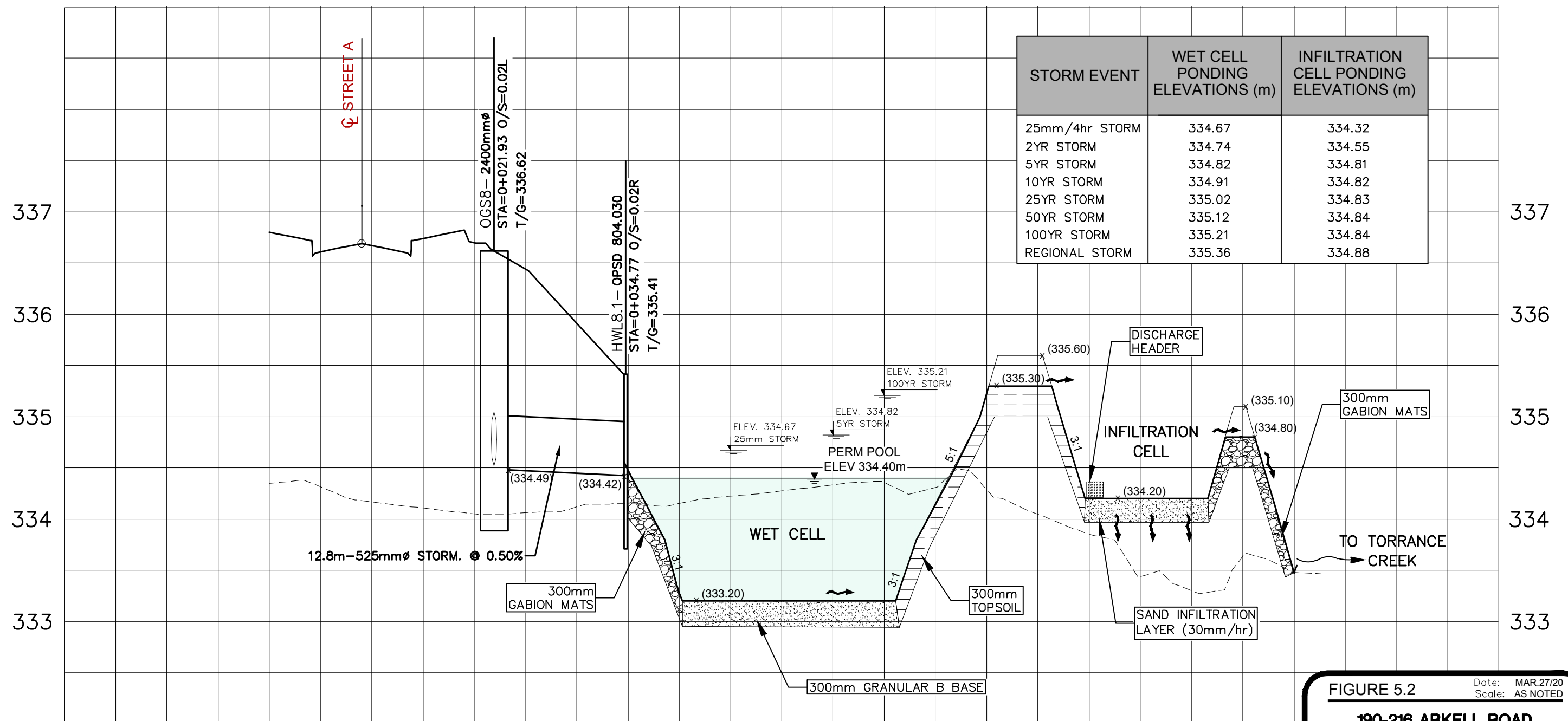
SWM FACILITY PLAN



Project No.: 42063-104

## SWM FACILITY - PLAN VIEW

SCALE 1: 250



# STORM OUTLET PROFILE - SECTION A-A

SCALE H 1:400 V 1:40

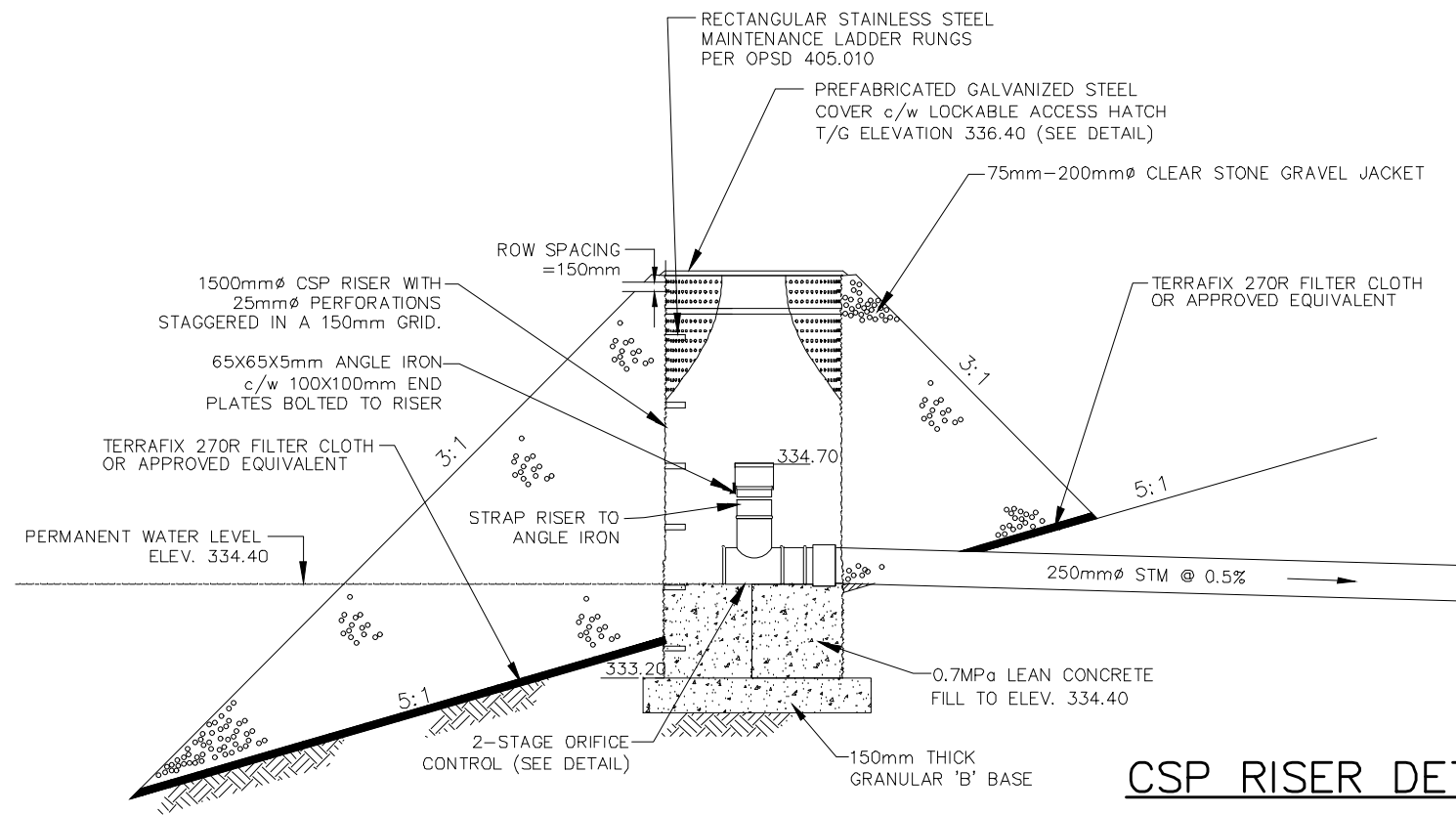
FIGURE 5.2 Date: MAR.27/20  
Scale: AS NOTED

190-216 ARKELL ROAD

SWM FACILITY DETAILS 1

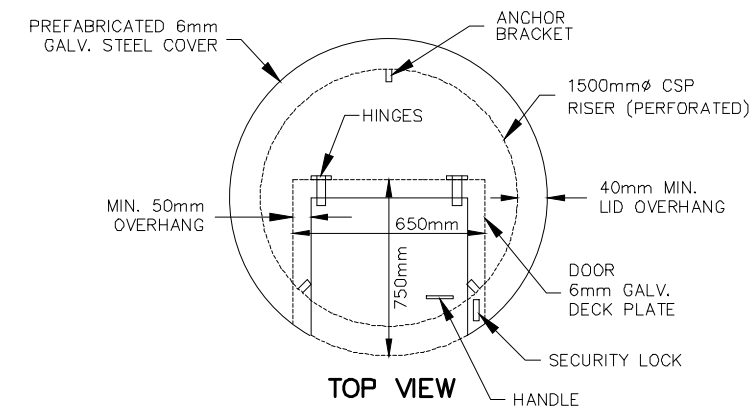


Project No.: 42063-104

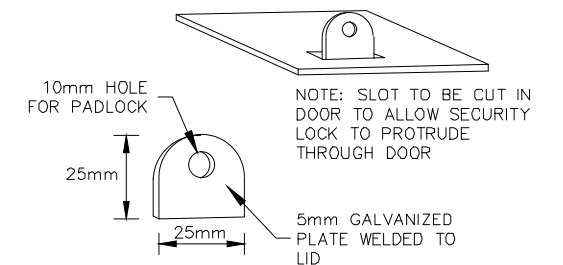
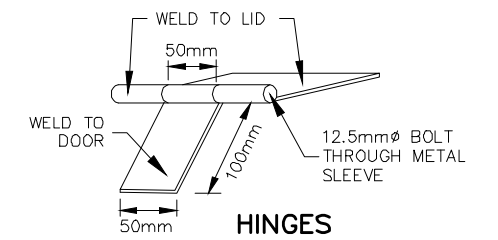
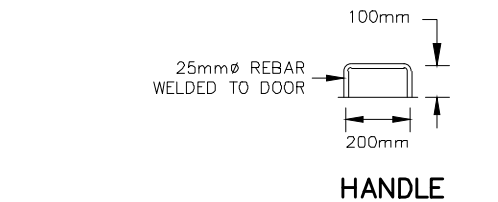


**CSP RISER DETAIL**

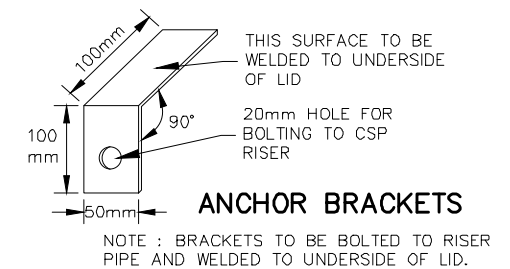
N.T.S.



NOTE: CONTRACTOR SHALL SUPPLY A PAD LOCK ON THE STEEL COVER AND PROVIDE THE KEYS TO THE MUNICIPALITY AT THE TIME OF INSTALLATION

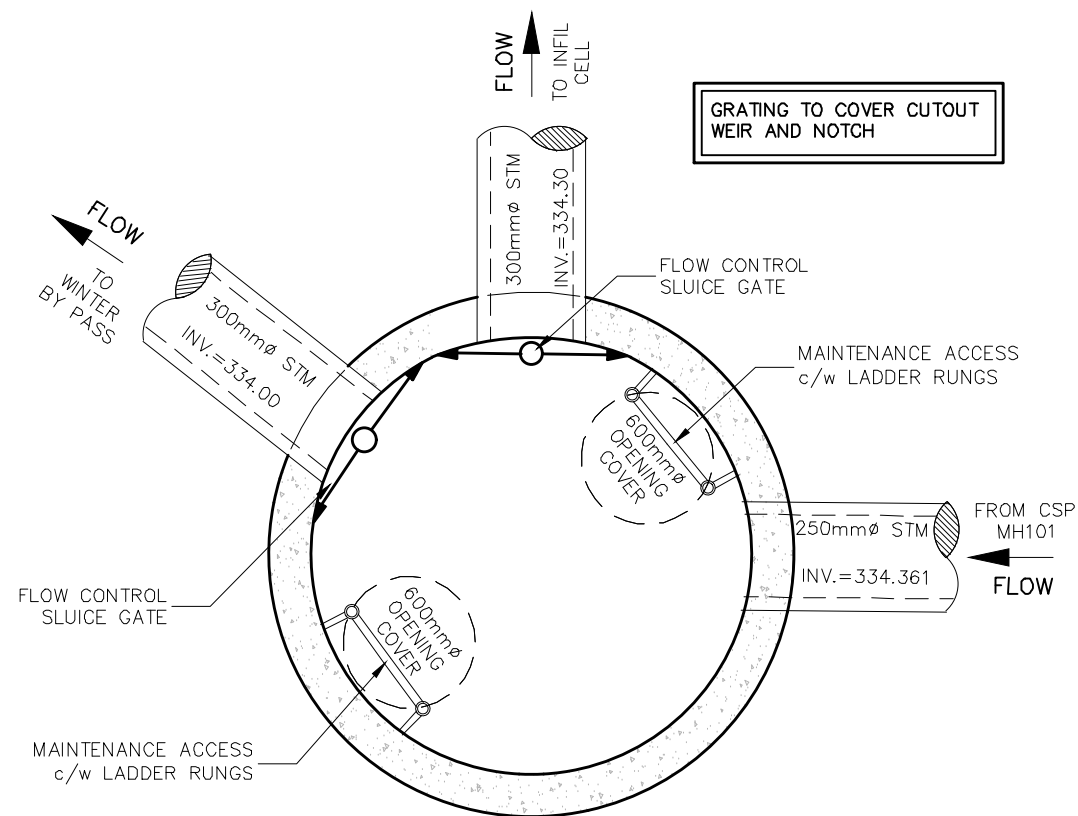


**SECURITY LOCK**



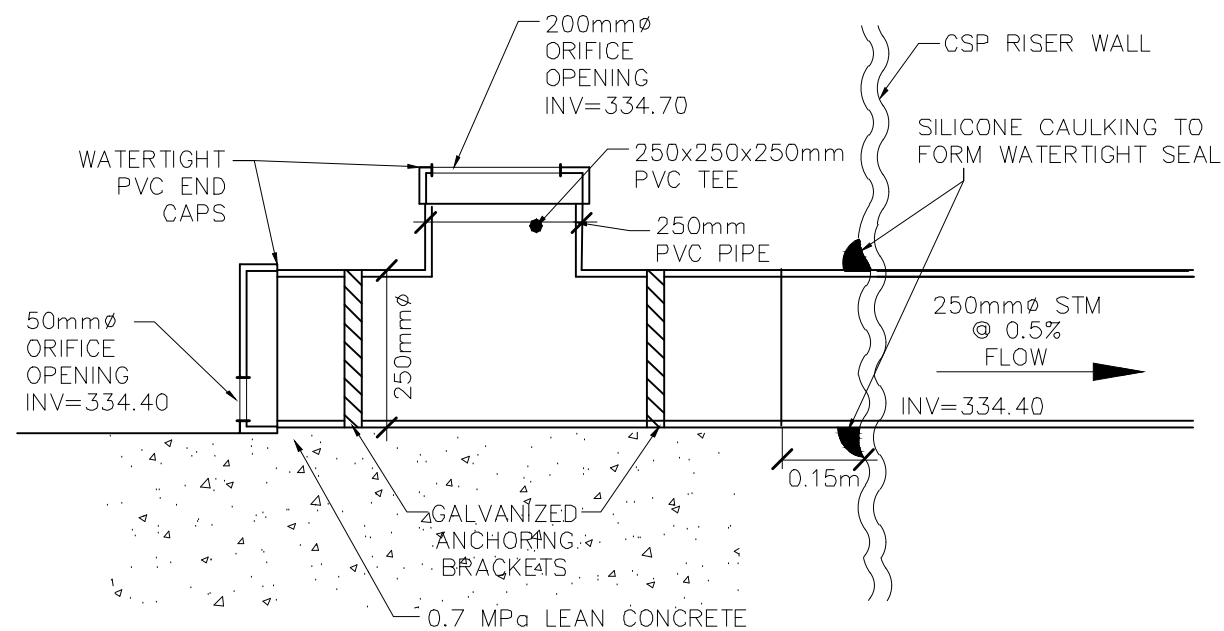
**GALVANIZED STEEL COVER DETAIL FOR 1500mmØ CSP RISER**

N.T.S.



**WINTER BYPASS MH102**

N.T.S.



**2-STAGE ORIFICE DETAIL FOR CSP RISER**

N.T.S.

FIGURE 5.3 Date: MAR.27/20 Scale: N.T.S.

190-216 ARKELL ROAD

**SWM FACILITY DETAILS 3**



Project No.: 42063-104

### 5.3 Water Quantity

Flows for all storm events will be conveyed to the proposed SWM facility by a combination of storm sewers and overland flow routes (road right-of-way and SWM facility access road). The post-development MIDUSS modelling output is included in **Appendix C**.

Discharge from the facility will be controlled via a multi-staged outlet located in a 1500mm diameter perforated CSP riser manhole proposed within the wet pond cell. This structure will house a 450mm cap with multiple orifice controls attached to a 450mm diameter outlet pipe. As illustrated in **Figure 5.4**, the multi-staged outlet consists of a 450mm diameter cap/orifice plate with a 50mm diameter orifice at an elevation of 334.40m, and a 200mm diameter orifice at an elevation of 334.70m.

The infiltration cell downstream of the wet cell is sized to completely infiltrate the 25mm-4hr storm conveyed from the wet cell. Larger storms, up to and including the 100-year events, are infiltrated as much as possible up to an elevation of 334.80, where an overflow weir is provided. These flows are directed through the infiltration cell and ultimately to the Torrance Creek Wetland.

A summary of the preliminary stage-storage-discharge relationships of the proposed wet cell and infiltration cell is shown in **Table 5.2 and 5.3** below.

**Table 5.2 – Wet Cell Stage-Storage-Discharge Summary**

Elevation (m)	Discharge (m <sup>3</sup> /s)	Volume (m3)	Remarks
334.40	0.0000	0	Permanent Pool Elevation / 50mm Orifice Invert
334.50	0.0015	40	Contour
334.60	0.0023	85	Contour
334.70	0.0029	135	200mm Orifice Invert
334.80	0.0311	189	Contour
334.90	0.0430	249	Contour
335.00	0.0522	314	Contour
335.10	0.0599	384	Contour
335.20	0.0668	457	Contour
335.30	0.0730	534	Emergency Overflow Weir
335.40	0.1628	614	Contour
335.50	0.4275	698	Contour
335.60	0.9140	786	Top of Pond

**Table 5.3 – Infiltration Cell Stage-Storage-Discharge Summary**

Elevation (m)	Discharge (m <sup>3</sup> /s)	Volume (m <sup>3</sup> )	Remarks
334.20	0.00000	0	Bottom on Infiltration Cell
334.30	0.00191	18	Contour
334.40	0.00207	38	Contour
334.50	0.00225	60	Contour
334.60	0.00243	83	Contour
334.70	0.00261	109	Contour
334.80	0.00281	136	Overflow Weir
334.90	0.154	165	Contour
335.00	0.433	196	Contour
335.10	0.906	229	Contour

The above discharges include a constant infiltration rate of 30mm/hr across the surface area of the infiltration cell. A summary of the peak flows and associated maximum ponding elevations in the Wet Cell and Infiltration Cell under the post-development conditions is provided in **Table 5.4 and 5.5** below. As previously mentioned, enough volume has been provided to store the 100-year storm event to maximum elevation of 335.21m.

**Table 5.4 – Summary of Peak Flows and Maximum Ponding Elevations in Wet Cell**

Storm Event	Peak Outflow to INF Cell (m <sup>3</sup> /s)	Maximum Ponding Volume (m <sup>3</sup> )	Maximum Ponding Elevation (m)
25mm Storm Event	0.003	119	334.67
2-Year Storm Event	0.013	155	334.74
5-Year Storm Event	0.034	203	334.82
10-Year Storm Event	0.044	255	334.91
25-Year Storm Event	0.054	328	335.02
50-Year Storm Event	0.061	396	335.12
100-Year Storm Event	0.067	464	335.21
Regional Storm Event	0.130	585	335.36

**Table 5.5 – Summary of Peak Flows and Maximum Ponding Elevations in Infiltration Cell**

Storm Event	Peak Infiltration Rate (m <sup>3</sup> /s)	Peak Outflow to Torrance (m <sup>3</sup> /s)	Maximum Ponding Volume (m <sup>3</sup> )	Maximum Ponding Elevation (m)
25mm Storm Event	0.002	-	26	334.34
2-Year Storm Event	0.002	-	71	334.55
5-Year Storm Event	0.003	<b>0.013</b>	138	334.81
10-Year Storm Event	0.003	<b>0.036</b>	142	334.82
25-Year Storm Event	0.003	<b>0.047</b>	145	334.83
50-Year Storm Event	0.003	<b>0.055</b>	147	334.84
100-Year Storm Event	0.003	<b>0.063</b>	148	334.84
Regional Storm Event	0.003	<b>0.126</b>	160	334.88



A summary of the peak flows for the pre- and post-development conditions is summarized in **Table 5.6**. The MIDUSS output for the quantity control can be found in **Appendix C**.

**Table 5.6 – Pre and Post-Development Peak Runoff Rates (m<sup>3</sup>/s)**

Drainage Area	25mm	2-year	5-year	10-year	25-year	50-year	100-year	Regional
<b>Pre-Development</b>								
101	0.029	0.049	0.081	0.104	0.126	0.142	0.165	0.217
102	0.002	0.007	0.020	0.034	0.057	0.077	0.101	0.123
103	0.007	0.012	0.018	0.026	0.032	0.037	0.042	0.029
104	0.002	0.006	0.015	0.025	0.036	0.046	0.057	0.030
105	0.001	0.002	0.003	0.004	0.005	0.006	0.007	0.007
<b>Total to Wetland</b>	<b>0.038</b>	<b>0.069</b>	<b>0.122</b>	<b>0.166</b>	<b>0.213</b>	<b>0.252</b>	<b>0.302</b>	<b>0.392</b>
<b>Post-Development</b>								
<b>SWMF Release (201-1 to 201-7)</b>	-	-	0.013	0.036	0.047	0.055	0.063	0.126
202-1	0.002	0.007	0.020	0.034	0.057	0.077	0.101	0.123
202-2	0.007	0.011	0.016	0.021	0.029	0.037	0.046	0.026
203-1	0.009	0.014	0.019	0.024	0.029	0.033	0.038	0.031
203-2	0.001	0.003	0.007	0.012	0.017	0.022	0.027	0.015
203-3*	-	-	0.003	0.012	0.020	0.029	0.033	0.011
204	0.002	0.005	0.013	0.021	0.031	0.039	0.048	0.026
205-1	0.004	0.006	0.008	0.011	0.015	0.019	0.024	0.014
205-2	0.006	0.010	0.014	0.018	0.022	0.025	0.030	0.015
206	0.004	0.005	0.008	0.009	0.011	0.013	0.015	0.005
<b>Total to Torrance</b>	<b>0.027</b>	<b>0.046</b>	<b>0.083</b>	<b>0.119</b>	<b>0.184</b>	<b>0.237</b>	<b>0.292</b>	<b>0.362</b>

\*Drainage Area 203-3 represent roof areas in Blocks 3 and 4 directed to infiltration galleries prior to overflow

\*\*Note: The sums may not add up arithmetically. They are based on the timing and sum of hydrographs taken directly from MIDUSS.

In order to confirm adequate capacity within the existing receiving infrastructure on Arkell Road, Plan and Profile drawings were received by the City of Guelph. It was assumed that the receiving storm sewers were only receiving flow from within the right-of-way prior to the proposed development.

Under post-development conditions, approximately 0.22 ha is directed west towards the stone energy dissipators. At an approximate imperviousness of 68%, the first length of pipe (a 300mm sewer at 0.26%) is flowing at 93% capacity under the 5-year storm, falling within City standards. Approximately 0.18 ha is directed towards the Arkell Road infiltration gallery. At an approximate imperviousness of 63%, the first length of pipe (a 300mm sewer at 0.61%) is flowing at 46% capacity under the 5-year storm, falling within City standards.

### 5.3.1 Culvert Underneath Street A

Prior to outletting to Torrance Creek, Drainage Area 203-2, and portions of Drainage Areas 203-1 and 204 are directed towards the future parklands block adjacent to the development. In order to provide a legal outlet to Torrance Creek and prevent ponding, a 450mm culvert underneath Street A is proposed. The culvert has been adequately sized to convey the Regional storm event. Under large storm events, overflow from the proposed infiltration galleries within residential blocks 3 and 4 will be directed towards this culvert.

## 5.4 Monthly Water Balance

A monthly water budget calculation has been conducted to assess potential hydrologic impacts the proposed development may have on the existing wetland. Refer to **Figures 2.2 & 4.1** for pre and post-development catchments used in the analyses.

Annual precipitation for the Subject Lands was estimated to be approximately 923.2mm/year, based on data gathered at the Guelph Arboretum weather station between 1971 and 2000. Evapotranspiration, runoff and infiltration/recharge rates for pre- and post-development conditions were estimated using the Thornthwaite and Mather method (1957).

### 5.4.1 Infiltration to Groundwater

Under pre-development conditions, infiltrated water contributes to the shallow groundwater table, which flows southwesterly towards the nearby Burke Well.

As previously mentioned, the TCSS divided the subwatershed into three stormwater management areas, with respect to groundwater recharge, and established specific infiltration targets for each area. The Subject Lands fall within Area 2 (Arkell Road to Torrance Creek); for which a minimum infiltration target of 150mm/year is required for any new development within this area.

Under pre-development conditions, the Subject Lands have a passive infiltration of 7,580m<sup>3</sup>/year. Under post-development conditions, the development area has passive infiltration of 6,721m<sup>3</sup>/year. Through the implementation of lot-level infiltration galleries (wherever possible) and an end-of-pipe infiltration cell operable during non-winter months, the total annual infiltration rate can be increased to 7,816m<sup>3</sup>/year. This equates to an equivalent infiltration rate across the Subject Lands of 251.6 mm/year, exceeding the TCSS criteria and providing an annual 3% volume surplus over existing. Refer to the Water Balance Analysis in **Appendix E** for more details.

### 5.4.2 Surface Runoff to Wetland

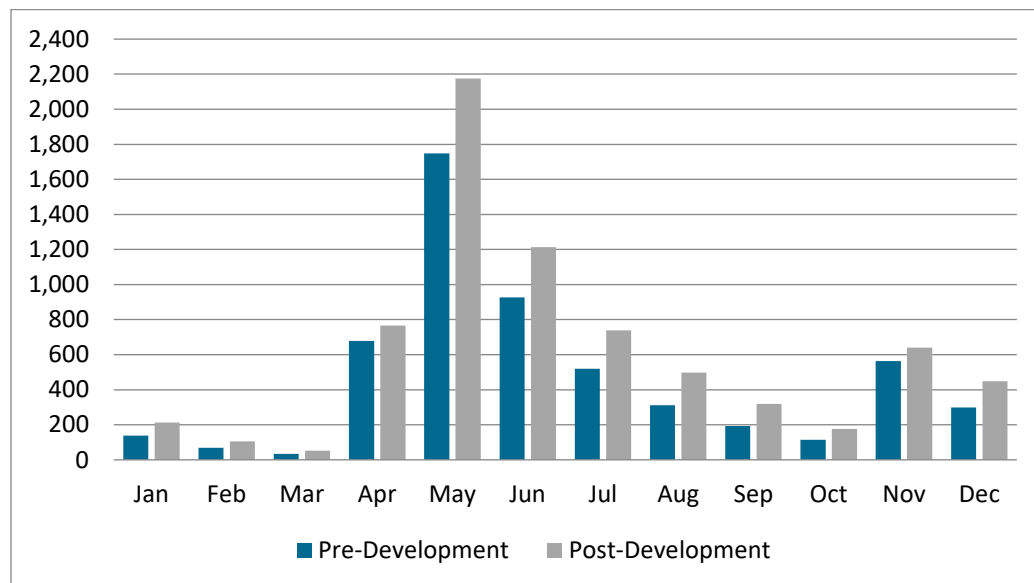
Under pre-development conditions, the Subject Lands drain to the northwest and provide surface water inputs to the adjacent Torrance Creek wetland complex.

Under pre-development conditions, it was calculated that the Subject Lands (2.97ha) has imperviousness of approximately 11.5%. Approximately 5,595m<sup>3</sup>/year of runoff is generated by the Subject Lands in the pre-development condition.

Under post-development conditions, the total Subject Lands that drain to the wetland is approximately 2.04ha. Catchments 205-1 & 205-2 (0.206ha) will drain to Arkell Road without control. The increased impervious areas under post-development conditions inevitably result in an increased annual runoff volume to the adjacent wetland. Approximately 7,347m<sup>3</sup>/year of runoff is generated by the Subject Lands under post-development conditions, which equates to an annual surplus of 1,752m<sup>3</sup>/year of surface runoff volume to the wetland complex. On a monthly basis, pre-development volumes are sustained and monthly distribution of excess runoff is well balanced as shown in **Figure 5.4**.



**Figure 5.4 – Pre & Post-Development Monthly Runoff Volume to Wetland (m<sup>3</sup>)**



Under post-development conditions, the total Subject Lands that drain to the wetland is approximately 2.04ha. Catchments 205-1 & 205-2 (0.206ha) will drain to Arkell Road without control. The increased impervious areas under post-development conditions inevitably result in an increased annual runoff volume to the adjacent wetland. Approximately 7,347m<sup>3</sup>/year of runoff is generated by the Subject Lands under post-development conditions, which equates to a surplus of 1,752m<sup>3</sup>/year (31% increase) of surface runoff volume to the wetland complex. On a monthly basis, pre-development volumes are sustained and monthly distribution of excess runoff is well balanced as shown in **Figure 5.4**.

## 5.5 Erosion Assessment

The TCSS recommended that any newly proposed development throughout the watershed should implement a SWM solution that provides at least a 24-hour drawdown for the volume generated during the 25mm storm event; to ensure that threshold flow durations do not exceed pre-development levels. However, due to the small catchment size of the SWM facility, this drawdown time cannot be achieved. Using the smallest possible orifice size (50mm) per MECP standards, the proposed SWM facility has been designed to provide approximately 23-hour drawdown time on the 25mm storm event volume. A majority of the outflows from the wet cell are directed towards the infiltration cell prior to be discharged into Torrance Creek, so MTE does not believe this will cause any adverse effect to the downstream watercourse. The drawdown calculations are provided in **Appendix D**.

## 5.6 Landscape Design

A landscape design for the proposed SWM facility will be completed during the final design stage of the development. The reasons for landscaping these types of facilities are aesthetics, erosion protection and long term bank stability, temperature increase mitigation, deterring waterfowl from nesting along their banks, and to limit pedestrian access into the permanent pool components. To that end, the facility will be designed in accordance to the City's stormwater management policies and guidelines for aesthetics, landscaping, and safety of stormwater management facilities.

## 5.7 Temperature Mitigation

The TCSS requires that the monitored temperature of the creek not exceed 25°C.

Detailed temperature mitigation calculations will be performed at the detailed design phase to determine if the current stormwater management design is satisfactory to achieve the required temperatures. If temperature monitoring of the Torrance Creek Wetland indicate that observed temperatures exceed 25°C, additional temperature mitigation measures may be explored and implemented if required (ex. enhanced grass swales, cooling trenches, etc.).

## 6.0 Monitoring Program

A monitoring program will be implemented, which will serve to ensure that the stormwater management plan proposed within this report is implemented and performing at an acceptable level.

### 6.1 During Development Monitoring Program

This stage will begin at the commencement of area grading of the subdivision and will continue until 100% full buildout of the subdivision (i.e. road is urbanized, buildings are constructed, lots are sodded/landscaped, and open spaces are stabilized) of the subdivision. Monitoring of the SWM facility will include:

- Standard inspection of vegetation, structures, and general operation of hydraulic controls (observations of drawdown) within the SWM facility once installed. These inspections are to occur seasonally and typically after a significant rainfall event.
- Regular inspection and maintenance of erosion and sediment control measures around and within the SWM facility.

Standard inspection and maintenance of the SWM facility will be provided throughout the “During Development” period.

### 6.2 Post-Development Monitoring Program

This period of the monitoring will begin following 100% full buildout of the subdivision. The purpose of this stage of the monitoring is to ensure that the SWM facility continues to operate as designed. Monitoring during this stage will include:

- Standard inspection of vegetation, structures, and general operation of hydraulic controls (observations of drawdown) within the SWM facility. These inspections are to occur seasonally and typically after a significant rainfall event; until assumption of the facility by the City.

It is recommended that, following completion of the developer’s portion of the post-development monitoring program and assumption of the SWM facility by the City, the City continues with a post-development inspection and maintenance program to ensure the long term effectiveness of the proposed SWM facility.

A monitoring program will be established within the SWM facility according to the requirements outlined within the *City of Guelph Stormwater Management Master Plan* prepared by AMEC Environment & Infrastructure (February 13, 2012).

## 7.0 Erosion and Sediment Control Measures

Precautions will be taken during construction to limit erosion and sedimentation. Erosion and Sediment Control Plans will be prepared and provided during the detailed design stage. The plans will illustrate the erosion and sediment control measures to be implemented during construction, which will limit impacts associated with site development.

Typically, the recommended construction sequence for erosion and sediment control measures are as follows:

- Placement of all sediment control fencing where required,
- Stripping and strategic placement of topsoil stockpiles. Placement of sediment control fencing around all stockpile areas.

- Construction of temporary sediment control ponds, which will serve as sedimentation basins for the site during construction.
- Construction of temporary swales to direct runoff to sedimentation basins, with rock check dams as required to control velocities.
- Re-vegetation of completed areas as soon as possible after construction, including those areas not slated for construction within 60 days.

Where rock check dams are proposed to promote sedimentation and reduce velocities, clean aggregate is to be placed perpendicular to the direction of flow in the swale, with a small volume of excavation on the upstream side to provide storage for accumulated sediment.

Sediment control fencing shall consist of filter fabric attached to page wire fencing and sealed at ground level. It will be installed at the perimeter of the work areas and intermittently on sloped areas where required. Sediment control fencing will be placed around all topsoil stockpiles.

Storage consistent with the GRCA's requirement of 125m<sup>3</sup>/ha of live and dead storage respectively (total 250m<sup>3</sup>/ha), will be provided. This storage will be provided to ensure that suspended material will have ample time to settle out. In addition, the sediment basin will be sized with sufficient capacity to allow flows to pass without breaching. Once the active construction and grading activities have been completed, the sedimentation basins can be cleaned out.

Access to topsoil or fill storage areas will be located on the upstream side of storage piles. This practice will ensure continuity of the sediment control fencing in the downslope direction; which is most vulnerable to erosion and sediment deposition. Further, topsoil and hydroseed will be placed on all exposed areas following the completion of grading activities.

It is recommended that during construction, monitoring and inspection of the erosion and sediment controls be conducted to ensure the satisfactory performance of these measures. Reporting of the inspection and monitoring results should be distributed to the City and GRCA. If it is found that the erosion and sediment control measures are not working adequately, they shall be augmented to the satisfaction of the City and the GRCA, based on field decisions.

## 8.0 Conclusions and Recommendations

Based on the foregoing analysis, it is concluded that:

- The stormwater management strategy herein outlined will provide the Subject Lands with appropriate levels of quality, quantity, and erosion controls to meet the criteria set out by the *Torrance Creek Subwatershed Study Management Strategy*, the City of Guelph, and the Grand River Conservation Authority;
- Enhanced quality control of stormwater runoff can be provided by the proposed stormwater management strategy; which includes: an OGS, a wet pond cell, and an infiltration cell;
- Quantity control targets for post-development peak flows rates to the adjacent wetland can be achieved in the proposed SWM facility for all storm events up to and including the Regional storm event;
- Infiltration targets defined within the TCSS can be satisfactorily met through the use of passive and active infiltration measures;
- Monthly surface water contributions to the wetland will be maintained or exceeded; and
- Post-development erosion will be mitigated by the use of extended detention of the 25mm storm event.

The findings of this report and the above conclusions lead to the following recommendations:

- Upon completion of detailed design, a quality/quantity control SWM facility be constructed to provide control of stormwater as described in Sections 4.0 and 5.0 of this report; and
- That sediment and erosion controls during construction will be implemented as described in Section 7.0 of this report.

All of which is respectfully submitted,

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[vlazic@mte85.com](mailto:vlazic@mte85.com)

AJC:tmd

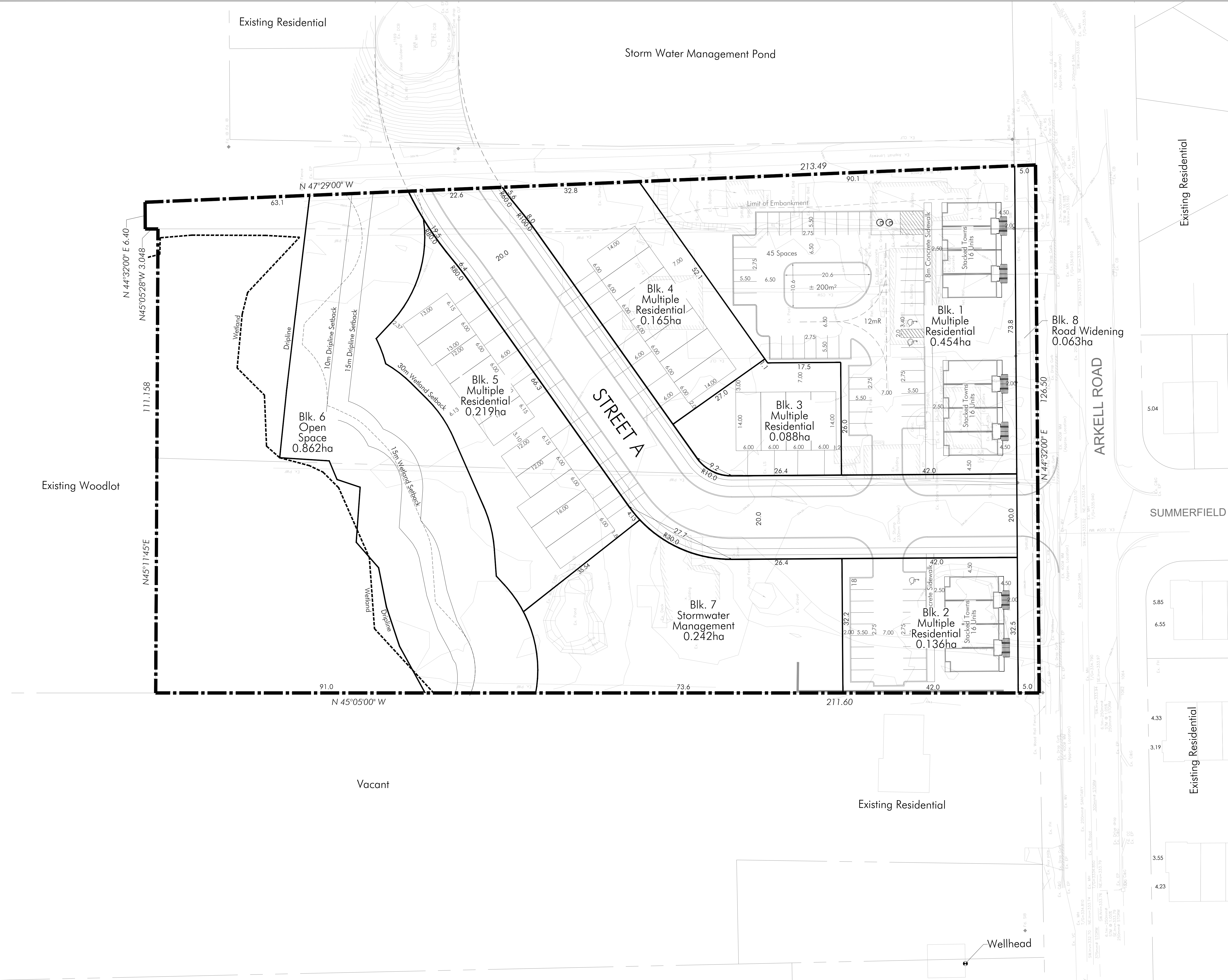
M:\42063\104\02 - Reports\MTE Reports\SWM\November 2021\42063-104\_rpt\_2021-12-03\_Preliminary SWM Report.docx

## Appendix A

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# **Draft Plan of Subdivision (Reduced)**





## CONCEPT PLAN A

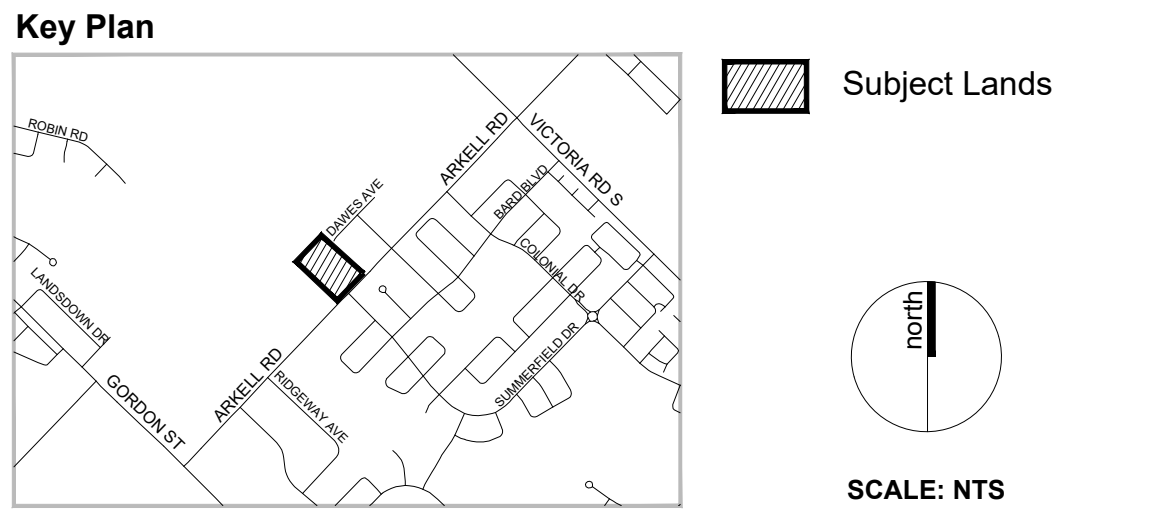
**Legal Description**  
PART OF LOT 6, CONCESSION 8, GEOGRAPHICAL TOWNSHIP OF PUSLINCH.  
CITY OF GUELPH

**Owner's Certificate**  
I HEREBY AUTHORIZE MACNAUGHTON HERMSEN BRITTON CLARKSON PLANNING LIMITED TO SUBMIT THIS PLAN FOR APPROVAL.

DATE: \_\_\_\_\_

**Surveyor's Certificate**  
I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED ON THIS PLAN AND THEIR RELATIONSHIP TO THE ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.

DATE: \_\_\_\_\_



**Additional Information Required Under Section 51(17) of the Planning Act R.S.O. 1990, c.P.13 as Amended**

A. AS SHOWN	B. AS SHOWN	C. AS SHOWN
D. MULTIPLE RESIDENTIAL, OPEN SPACE		
E. AS SHOWN	F. AS SHOWN	G. AS SHOWN
H. MUNICIPAL WATER SUPPLY	I. LOAM	J. AS SHOWN
K. ALL SERVICES AS REQUIRED	L. AS SHOWN	

Area Schedule			61T
Description	Lots/Blocks	Units	Area (ha)
Multiple Residential	1-5	70	1.062
Open Space	6		0.862
Stormwater Management	7		0.242
Road Widening	8		0.063
Roads			0.348
<b>Total</b>	<b>8</b>	<b>70</b>	<b>2.577</b>

- Notes**
- All dimensions are in metres unless otherwise shown.
  - Dripline Limits - Natural Resource Solutions Inc. (NRSI) June, 2016
  - Welland Limits - Natural Resource Solutions Inc. (NRSI) August, 2016
  - Property boundary is approximate, based on MTE Existing Conditions Plan and Wellington County Survey plans 61R-773, 61R-2819
  - Surrounding parcel boundaries are approximate/ taken from Vurnap (First Base Solutions) aerial imagery
  - Unit yield based on Conceptual Site Plan prepared by MHBC Planning
  - Building footprints outside of subject lands gathered from City of Guelph open data

12.	April 28, 2021	Adjusted Stormwater, Residential Blocks;	G.C.
11.	January 28, 2021	Stacked Townhouse Orientation;	G.C.
10.	March 18, 2020	SWM revision;	G.C.
9.	January 6, 2020	Update/ Issued for Review;	G.C.
8.	December 11, 2019	Update/ Issued for Review;	G.C.
7.	October 9, 2019	Update/ Issued for Review;	G.C.
6.	July 8, 2019	Update/ Issued for Review;	G.C.
5.	December 6, 2018	Update/ Issued for Review;	G.C.
3.	September 12, 2018	Update/ Issued for Review;	G.C.
2.	July 4, 2017	Update/ Issued for Review;	G.C.
1.	March 13, 2017	Issued for Review;	G.C.

**Revision No.**   **Date**   **Issued / Revision**   **By**




**PLANNING  
URBAN DESIGN  
& LANDSCAPE  
ARCHITECTURE**

200-540 BINGEWANS CENTRE DR. KITCHENER, ON. N2B 3X9 | P: 519.576.3650 F: 519.576.0121 | WWW.MHBCPLAN.COM

<b>Approval Stamp</b>	<b>Date</b> April 28, 2021
<b>File No.</b> 15246A	
<b>Plan Scale</b> 1:400 (36x24)	
<b>Drawn By</b> G.C.	

<b>Project</b> Arkell Road - Guelph	<b>Checked By</b> D.A.
-------------------------------------	------------------------

**Applicant**



**Crescent Homes**  
151 Curzon Crescent  
Guelph, ON. N1K 0B3  
P: 647.523.6487

<b>File Name</b> <b>CONCEPT PLAN</b>	<b>Dwg No.</b> 1 of 1
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<b>Scale Bar</b>	2.5 0 5 10 25m
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## Appendix B

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# **Existing Conditions Catchment Parameters and MIDUSS Modelling**





**Arkeil Road**  
**STORMWATER MANAGEMENT**  
 Guelph, Ontario

Project Number: 42063-104  
 Date: October 8, 2021  
 Design By: AJC  
 File: Q:\42063\104\SWM\September 2021\42063-104 Master SWM Facility Design Sheet.xlsx

**HYDROLOGIC PARAMETERS**

**Pre-Development Conditions**

Sub-Catchment Number	Area	Overland Slope	Overland Length	SCS Curve Number			Percent Impervious	Land Use	Comment
				Pervious (AMC II)	Pervious (AMC III)	Impervious			
	(ha)	(%)	(m)				(%)		
<b>Within Subject Lands</b>									
101	1.714	0.5	150	74	87	98	16%	Residential	Ex. Residential and Yards
102	0.863	0.5	50	74	87	98	0%	Wetland	Wetland/Forested Area/Torrance Creek
	2.577						10.6%		
<b>Outside of Subject Lands</b>									
103	0.240	0.8	225	74	87	98	30%	Residential	Private laneway
104	0.234	20	8	74	87	98	0%	SWMF	Ex. SWMF + Embankments
105	0.057	0.5	125	74	87	98	20%	Residential	Driveways + Ditch within Right-of-way
	0.531						15.7%		
<b>Total</b>	<b>3.108</b>						<b>11.5%</b>		

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"      Licensee name:          A"
"      Company                  Microsoft"
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"      240.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
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"      1 Chicago storm"
"      509.000 Coefficient A"
"      6.000 Constant B"
"      0.799 Exponent C"
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"      1 Equal length"
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"      1.714 Total Area"
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"      0.500 Overland Slope"
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"      150.000 Pervious length"
"      0.500 Pervious slope"
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"      150.000 Impervious length"
"      0.500 Impervious slope"
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"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.806 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.029 0.000 0.000 0.000 c.m/sec"
"      Catchment 101 Pervious Impervious Total Area "
"      Surface Area 1.440 0.274 1.714 hectare"
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"      Time to Centroid 347.290 133.825 217.179 minutes"
"      Rainfall depth 25.028 25.028 25.028 mm"
"      Rainfall volume 360.34 68.64 428.97 c.m"
"      Rainfall losses 22.566 4.855 19.732 mm"
"      Runoff depth 2.462 20.172 5.295 mm"
"      Runoff volume 35.44 55.32 90.76 c.m"
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"      1 SCS method"
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"      50.000 Flow length"
"      0.500 Overland Slope"
"      0.863 Pervious Area"
"      50.000 Pervious length"
"      0.500 Pervious slope"
"      0.000 Impervious Area"
"      50.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
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"      Surface Area 0.863 0.000 0.863 hectare"
"      Time of concentration 90.470 5.452 90.469 minutes"
"      Time to Centroid 248.117 125.802 248.116 minutes"
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"      Rainfall volume 215.99 0.00 215.99 c.m"
"      Rainfall losses 22.566 4.865 22.566 mm"
"      Runoff depth 2.461 20.162 2.461 mm"
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" 30.000 % Impervious"

" 0.240 Total Area"

" 225.000 Flow length"

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" 0.168 Pervious Area"

" 225.000 Pervious length"

" 0.800 Pervious slope"

" 0.072 Impervious Area"

" 225.000 Impervious length"

" 0.800 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.098 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.807 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.007 0.000 0.002 0.029 c.m/sec"

" Catchment 103 Pervious Impervious Total Area "

" Surface Area 0.168 0.072 0.240 hectare"

" Time of concentration 193.730 11.674 51.980 minutes"

" Time to Centroid 369.414 135.621 187.381 minutes"

" Rainfall depth 25.028 25.028 25.028 mm"

" Rainfall volume 42.05 18.02 60.07 c.m"

" Rainfall losses 22.566 4.828 17.245 mm"

" Runoff depth 2.462 20.200 7.783 mm"

" Runoff volume 4.14 14.54 18.68 c.m"

" Runoff coefficient 0.098 0.807 0.311 "

" Maximum flow 0.000 0.007 0.007 c.m/sec"

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" 4 Add Runoff "

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" 1 SCS method"

" 104 Catchment 104 - Ex. SWMF+Embankment"

" 0.000 % Impervious"

" 0.234 Total Area"

" 8.000 Flow length"

" 20.000 Overland Slope"

" 0.234 Pervious Area"

" 8.000 Pervious length"

" 20.000 Pervious slope"

" 0.000 Impervious Area"

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" 20.000 Impervious slope"

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" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.000 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.002 0.000 0.007 0.007 c.m/sec"

" Catchment 104 Pervious Impervious Total Area "

" Surface Area 0.234 0.000 0.234 hectare"

" Time of concentration 9.962 0.600 9.962 minutes"

" Time to Centroid 153.643 118.857 153.642 minutes"

" Rainfall depth 25.028 25.028 25.028 mm"

" Rainfall volume 58.56 0.00 58.56 c.m"

" Rainfall losses 22.580 6.253 22.580 mm"

" Runoff depth 2.447 18.775 2.447 mm"

" Runoff volume 5.73 0.00 5.73 c.m"

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" Maximum flow 0.002 0.000 0.002 c.m/sec"

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" External"

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" Hydrograph volume 24.406 c.m"

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" 1 Equal length"

" 1 SCS method"

" 105 Catchment 105 - Driveways + Ditch Within RoW"

" 20.000 % Impervious"

" 0.057 Total Area"

" 125.000 Flow length"

" 0.500 Overland Slope"

" 0.046 Pervious Area"

" 125.000 Pervious length"

" 0.500 Pervious slope"

" 0.011 Impervious Area"

" 125.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.098 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.804 Impervious Runoff coefficient"

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"      0.100 Impervious Ia/S coefficient"
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"      Catchment 105      Pervious      Impervious Total Area  "
"      Surface Area      0.046      0.011      0.057      hectare"
"      Time of concentration 156.772      9.447      57.847      minutes"
"      Time to Centroid 326.000      132.108      195.806      minutes"
"      Rainfall depth 25.028      25.028      25.028      mm"
"      Rainfall volume 11.41      2.85      14.27      c.m"
"      Rainfall losses 22.566      4.903      19.034      mm"
"      Runoff depth 2.461      20.125      5.994      mm"
"      Runoff volume 1.12      2.29      3.42      c.m"
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"      Torrence Creek"
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" 900 Node #"
"      Torrence Creek"
"      Maximum flow      0.038      c.m/sec"
"      Hydrograph volume 139.825      c.m"
"      0.001      0.038      0.010      0.000"
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"      3 Runoff Totals on EXIT"
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"      Total Impervious area      0.358      hectare"
"      Total % impervious      11.507"
" 19 EXIT"

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"      Company      Microsoft"
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"      150.000 Flow length"
"      0.500 Overland Slope"
"      1.440 Pervious Area"
"      150.000 Pervious length"
"      0.500 Pervious slope"
"      0.274 Impervious Area"
"      150.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.163 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.852 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.049      0.000      0.000      0.000 c.m/sec"
"      Catchment 101      Pervious      Impervious Total Area  "
"      Surface Area 1.440      0.274      1.714      hectare"
"      Time of concentration 109.757      8.796      59.477      minutes"
"      Time to Centroid 235.201      101.169      168.451      minutes"
"      Rainfall depth 34.259      34.259      34.259      mm"
"      Rainfall volume 493.24      93.95      587.19      c.m"
"      Rainfall losses 28.657      5.085      24.886      mm"
"      Runoff depth 5.601      29.174      9.373      mm"
"      Runoff volume 80.64      80.01      160.65      c.m"
"      Runoff coefficient 0.163      0.852      0.274      "
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"      0.049      0.049      0.000      0.000"

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"      0.500      Overland Slope"
"      0.863      Pervious Area"
"      50.000      Pervious length"
"      0.500      Pervious slope"
"      0.000      Impervious Area"
"      50.000      Impervious length"
"      0.500      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.163      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.000      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.007      0.000      0.049      0.049 c.m/sec"
"      Catchment 102      Pervious      Impervious      Total Area "
"      Surface Area      0.863      0.000      0.863      hectare"
"      Time of concentration      56.775      4.550      56.775      minutes"
"      Time to Centroid      171.819      94.883      171.819      minutes"
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"      Rainfall volume      295.65      0.00      295.65      c.m"
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"      Runoff depth      5.600      28.978      5.600      mm"
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"      2      Start - New Tributary"

```

```

"      0.007      0.000      0.007      0.050"
" 33      CATCHMENT 103"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      103      Catchment 103 - Laneway"
"      30.000      % Impervious"
"      0.240      Total Area"
"      225.000      Flow length"
"      0.800      Overland Slope"
"      0.168      Pervious Area"
"      225.000      Pervious length"
"      0.800      Pervious slope"
"      0.072      Impervious Area"
"      225.000      Impervious length"
"      0.800      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.164      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.849      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.012      0.000      0.007      0.050 c.m/sec"
"      Catchment 103      Pervious      Impervious      Total Area "
"      Surface Area      0.168      0.072      0.240      hectare"
"      Time of concentration      121.577      9.744      44.428      minutes"
"      Time to Centroid      249.343      102.633      148.134      minutes"
"      Rainfall depth      34.259      34.259      34.259      mm"
"      Rainfall volume      57.55      24.67      82.22      c.m"
"      Rainfall losses      28.657      5.187      21.616      mm"
"      Runoff depth      5.601      29.072      12.643      mm"
"      Runoff volume      9.41      20.93      30.34      c.m"
"      Runoff coefficient      0.164      0.849      0.369      "
"      Maximum flow      0.001      0.012      0.012      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.012      0.012      0.007      0.050"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.012      0.012      0.012      0.050"
" 40      HYDROGRAPH Combine 800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.012      c.m/sec"
"      Hydrograph volume      30.342      c.m"
"      0.012      0.012      0.012      0.012"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.012      0.000      0.012      0.012"
" 33      CATCHMENT 104"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      104      Catchment 104 - Ex. SWMF+Embankment"
"      0.000      % Impervious"
"      0.234      Total Area"
"      8.000      Flow length"
"      20.000      Overland Slope"
"      0.234      Pervious Area"
"      8.000      Pervious length"

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"      20.000 Pervious slope"
"      0.000 Impervious Area"
"      8.000 Impervious length"
"      20.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.162 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.006      0.000      0.012      0.012 c.m/sec"
"      Catchment 104      Pervious      Impervious      Total Area      "
"      Surface Area      0.234      0.000      0.234      hectare"
"      Time of concentration      6.252      0.501      6.252      minutes"
"      Time to Centroid      111.541      89.345      111.541      minutes"
"      Rainfall depth      34.259      34.259      34.259      mm"
"      Rainfall volume      80.16      0.00      80.17      c.m"
"      Rainfall losses      28.719      7.755      28.719      mm"
"      Runoff depth      5.540      26.504      5.540      mm"
"      Runoff volume      12.96      0.00      12.96      c.m"
"      Runoff coefficient      0.162      0.000      0.162      "
"      Maximum flow      0.006      0.000      0.006      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.006      0.006      0.012      0.012"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.006      0.006      0.006      0.012"
" 40      HYDROGRAPH Combine      800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.017      c.m/sec"
"      Hydrograph volume      43.306      c.m"
"      0.006      0.006      0.006      0.017"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.006      0.000      0.006      0.017"
" 33      CATCHMENT 105"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      105      Catchment 105 - Driveways + Ditch Within RoW"
"      20.000      % Impervious"
"      0.057      Total Area"
"      125.000      Flow length"
"      0.500      Overland Slope"
"      0.046      Pervious Area"
"      125.000      Pervious length"
"      0.500      Pervious slope"
"      0.011      Impervious Area"
"      125.000      Impervious length"
"      0.500      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.164      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.852      Impervious Runoff coefficient"

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"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.002      0.000      0.006      0.017 c.m/sec"
"      Catchment 105      Pervious      Impervious      Total Area      "
"      Surface Area      0.046      0.011      0.057      hectare"
"      Time of concentration      98.384      7.885      47.187      minutes"
"      Time to Centroid      221.598      99.893      152.747      minutes"
"      Rainfall depth      34.259      34.259      34.259      mm"
"      Rainfall volume      15.62      3.91      19.53      c.m"
"      Rainfall losses      28.657      5.072      23.940      mm"
"      Runoff depth      5.601      29.187      10.318      mm"
"      Runoff volume      2.55      3.33      5.88      c.m"
"      Runoff coefficient      0.164      0.852      0.301      "
"      Maximum flow      0.000      0.002      0.002      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.002      0.002      0.006      0.017"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.002      0.002      0.002      0.017"
" 40      HYDROGRAPH Combine      800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.020      c.m/sec"
"      Hydrograph volume      49.187      c.m"
"      0.002      0.002      0.002      0.020"
" 40      HYDROGRAPH Confluence      800"
"      7      Confluence "
"      800      Node #"
"      External"
"      Maximum flow      0.020      c.m/sec"
"      Hydrograph volume      49.187      c.m"
"      0.002      0.020      0.002      0.000"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.002      0.020      0.020      0.000"
" 40      HYDROGRAPH Combine      900"
"      6      Combine "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.069      c.m/sec"
"      Hydrograph volume      258.166      c.m"
"      0.002      0.020      0.020      0.069"
" 40      HYDROGRAPH Confluence      900"
"      7      Confluence "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.069      c.m/sec"
"      Hydrograph volume      258.166      c.m"
"      0.002      0.069      0.020      0.000"
" 38      START/RE-START TOTALS 900"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      3.108      hectare"
"      Total Impervious area      0.358      hectare"
"      Total % impervious      11.507"
" 19      EXIT"

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"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:             Q:\42063\104\SWM\September 2021\MIDUSS\
"                                PRE"
"      Output filename:        5yrPRE.in"
"      Licensee name:          A"
"      Company                  Microsoft"
"      Date & Time last used:   9/24/2021 at 2:05:29 PM"
"  31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
"  32 STORM Chicago storm"
"      1 Chicago storm"
"      1593.000 Coefficient A"
"      11.000 Constant B"
"      0.879 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity        139.250 mm/hr"
"      Total depth              47.240 mm"
"      6 005hyd Hydrograph extension used in this file"
"  33 CATCHMENT 101"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      101 Catchment 101"
"      16.000 % Impervious"
"      1.714 Total Area"
"      150.000 Flow length"
"      0.500 Overland Slope"
"      1.440 Pervious Area"
"      150.000 Pervious length"
"      0.500 Pervious slope"
"      0.274 Impervious Area"
"      150.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.244 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.887 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.081 0.000 0.000 0.000 c.m/sec"
"      Catchment 101 Pervious Impervious Total Area "
"      Surface Area 1.440 0.274 1.714 hectare"
"      Time of concentration 78.588 7.885 49.638 minutes"
"      Time to Centroid 194.743 97.376 154.875 minutes"
"      Rainfall depth 47.240 47.240 47.240 mm"
"      Rainfall volume 680.14 129.55 809.69 c.m"
"      Rainfall losses 35.733 5.352 30.872 mm"
"      Runoff depth 11.507 41.888 16.368 mm"
"      Runoff volume 165.67 114.87 280.55 c.m"
"      Runoff coefficient 0.244 0.887 0.346 "
"      Maximum flow 0.020 0.079 0.081 c.m/sec"
"  40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.081 0.081 0.000 0.000"

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"  40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.081 0.081 0.081 0.000"
"  40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.081 c.m/sec"
"      Hydrograph volume 280.548 c.m"
"      0.081 0.081 0.081 0.081"
"  40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.081 0.000 0.081 0.081"
"  33 CATCHMENT 102"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      102 Catchment 102"
"      0.000 % Impervious"
"      0.863 Total Area"
"      50.000 Flow length"
"      0.500 Overland Slope"
"      0.863 Pervious Area"
"      50.000 Pervious length"
"      0.500 Pervious slope"
"      0.000 Impervious Area"
"      50.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.243 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.020 0.000 0.081 0.081 c.m/sec"
"      Catchment 102 Pervious Impervious Total Area "
"      Surface Area 0.863 0.000 0.863 hectare"
"      Time of concentration 40.652 4.079 40.652 minutes"
"      Time to Centroid 148.337 91.940 148.336 minutes"
"      Rainfall depth 47.240 47.240 47.240 mm"
"      Rainfall volume 407.68 0.00 407.68 c.m"
"      Rainfall losses 35.737 5.719 35.737 mm"
"      Runoff depth 11.503 41.521 11.503 mm"
"      Runoff volume 99.27 0.00 99.27 c.m"
"      Runoff coefficient 0.243 0.000 0.243 "
"      Maximum flow 0.020 0.000 0.020 c.m/sec"
"  40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.020 0.020 0.081 0.081"
"  40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.020 0.020 0.020 0.081"
"  40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.084 c.m/sec"
"      Hydrograph volume 379.819 c.m"
"      0.020 0.020 0.020 0.084"
"  40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"

```

" 0.020 0.000 0.020 0.084"

" 33 CATCHMENT 103"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 103 Catchment 103 - Laneway"

" 30.000 % Impervious"

" 0.240 Total Area"

" 225.000 Flow length"

" 0.800 Overland Slope"

" 0.168 Pervious Area"

" 225.000 Pervious length"

" 0.800 Pervious slope"

" 0.072 Impervious Area"

" 225.000 Impervious length"

" 0.800 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.244 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.889 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.018 0.000 0.020 0.084 c.m/sec"

" Catchment 103 Pervious Impervious Total Area "

" Surface Area 0.168 0.072 0.240 hectare"

" Time of concentration 87.051 8.734 39.283 minutes"

" Time to Centroid 205.095 98.522 140.093 minutes"

" Rainfall depth 47.240 47.240 47.240 mm"

" Rainfall volume 79.36 34.01 113.38 c.m"

" Rainfall losses 35.732 5.255 26.589 mm"

" Runoff depth 11.508 41.985 20.651 mm"

" Runoff volume 19.33 30.23 49.56 c.m"

" Runoff coefficient 0.244 0.889 0.437 "

" Maximum flow 0.002 0.018 0.018 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.018 0.018 0.020 0.084"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.018 0.018 0.018 0.084"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" External"

" Maximum flow 0.018 c.m/sec"

" Hydrograph volume 49.562 c.m"

" 0.018 0.018 0.018 0.018"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.018 0.000 0.018 0.018"

" 33 CATCHMENT 104"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 104 Catchment 104 - Ex. SWMF+Embankment"

" 0.000 % Impervious"

" 0.234 Total Area"

" 8.000 Flow length"

" 20.000 Overland Slope"

" 0.234 Pervious Area"

" 8.000 Pervious length"

" 20.000 Pervious slope"

" 0.000 Impervious Area"

" 8.000 Impervious length"

" 20.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.242 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.000 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.015 0.000 0.018 0.018 c.m/sec"

" Catchment 104 Pervious Impervious Total Area "

" Surface Area 0.234 0.000 0.234 hectare"

" Time of concentration 4.476 0.449 4.476 minutes"

" Time to Centroid 104.105 87.097 104.105 minutes"

" Rainfall depth 47.240 47.240 47.240 mm"

" Rainfall volume 110.54 0.00 110.54 c.m"

" Rainfall losses 35.825 9.778 35.825 mm"

" Runoff depth 11.415 37.462 11.415 mm"

" Runoff volume 26.71 0.00 26.71 c.m"

" Runoff coefficient 0.242 0.000 0.242 "

" Maximum flow 0.015 0.000 0.015 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.015 0.015 0.018 0.018"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.015 0.015 0.015 0.018"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" External"

" Maximum flow 0.034 c.m/sec"

" Hydrograph volume 76.272 c.m"

" 0.015 0.015 0.015 0.034"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.015 0.000 0.015 0.034"

" 33 CATCHMENT 105"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 105 Catchment 105 - Driveways + Ditch Within RoW"

" 20.000 % Impervious"

" 0.057 Total Area"

" 125.000 Flow length"

" 0.500 Overland Slope"

" 0.046 Pervious Area"

" 125.000 Pervious length"

" 0.500 Pervious slope"

" 0.011 Impervious Area"

" 125.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.244 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.882 Impervious Runoff coefficient"

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"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.003      0.000      0.015      0.034 c.m/sec"
"      Catchment 105      Pervious      Impervious Total Area  "
"      Surface Area      0.046      0.011      0.057      hectare"
"      Time of concentration      70.444      7.068      40.326      minutes"
"      Time to Centroid      184.778      96.234      142.700      minutes"
"      Rainfall depth      47.240      47.240      47.240      mm"
"      Rainfall volume      21.54      5.39      26.93      c.m"
"      Rainfall losses      35.732      5.553      29.696      mm"
"      Runoff depth      11.508      41.687      17.544      mm"
"      Runoff volume      5.25      4.75      10.00      c.m"
"      Runoff coefficient      0.244      0.882      0.371      "
"      Maximum flow      0.001      0.003      0.003      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.003      0.003      0.015      0.034"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.003      0.003      0.003      0.034"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      External"
"      Maximum flow      0.037      c.m/sec"
"      Hydrograph volume      86.272      c.m"
"      0.003      0.003      0.003      0.037"
" 40 HYDROGRAPH Confluence 800"
"      7 Confluence "
"      800 Node #"
"      External"
"      Maximum flow      0.037      c.m/sec"
"      Hydrograph volume      86.272      c.m"
"      0.003      0.037      0.003      0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.003      0.037      0.037      0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow      0.122      c.m/sec"
"      Hydrograph volume      466.091      c.m"
"      0.003      0.037      0.037      0.122"
" 40 HYDROGRAPH Confluence 900"
"      7 Confluence "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow      0.122      c.m/sec"
"      Hydrograph volume      466.091      c.m"
"      0.003      0.122      0.037      0.000"
" 38 START/RE-START TOTALS 900"
"      3 Runoff Totals on EXIT"
"      Total Catchment area      3.108      hectare"
"      Total Impervious area      0.358      hectare"
"      Total % impervious      11.507"
" 19 EXIT"

```

```

"      MIDUSS Output ----->"
"      MIDUSS version      Version 2.25 rev. 473"
"      MIDUSS created      Sunday, February 7, 2010"
"      10 Units used:      ie METRIC"
"      Job folder:      Q:\42063\104\SWM\September 2021\MIDUSS\
"      PRE"
"      Output filename:      10yrPRE.in"
"      Licensee name:      A"
"      Company      Microsoft"
"      Date & Time last used:      9/24/2021 at 2:06:02 PM"
" 31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      2221.000 Coefficient A"
"      12.000 Constant B"
"      0.908 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity      169.551      mm/hr"
"      Total depth      56.290      mm"
"      6 010hyd Hydrograph extension used in this file"
" 33 CATCHMENT 101"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      101 Catchment 101"
"      16.000 % Impervious"
"      1.714 Total Area"
"      150.000 Flow length"
"      0.500 Overland Slope"
"      1.440 Pervious Area"
"      150.000 Pervious length"
"      0.500 Pervious slope"
"      0.274 Impervious Area"
"      150.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.292 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.900 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.104      0.000      0.000      0.000 c.m/sec"
"      Catchment 101      Pervious      Impervious Total Area  "
"      Surface Area      1.440      0.274      1.714      hectare"
"      Time of concentration      65.907      7.253      44.199      minutes"
"      Time to Centroid      178.477      95.467      147.755      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      810.44      154.37      964.81      c.m"
"      Rainfall losses      39.871      5.643      34.395      mm"
"      Runoff depth      16.419      50.647      21.896      mm"
"      Runoff volume      236.40      138.89      375.29      c.m"
"      Runoff coefficient      0.292      0.900      0.389      "
"      Maximum flow      0.035      0.100      0.104      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.104      0.104      0.000      0.000"

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" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.104      0.104      0.104      0.000"
" 40      HYDROGRAPH Combine 900"
"      6      Combine "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.104      c.m/sec"
"      Hydrograph volume      375.289      c.m"
"      0.104      0.104      0.104      0.104"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.104      0.000      0.104      0.104"
" 33      CATCHMENT 102"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      102      Catchment 102"
"      0.000      % Impervious"
"      0.863      Total Area"
"      50.000      Flow length"
"      0.500      Overland Slope"
"      0.863      Pervious Area"
"      50.000      Pervious length"
"      0.500      Pervious slope"
"      0.000      Impervious Area"
"      50.000      Impervious length"
"      0.500      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.292      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.000      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.034      0.000      0.104      0.104 c.m/sec"
"      Catchment 102      Pervious      Impervious      Total Area "
"      Surface Area      0.863      0.000      0.863      hectare"
"      Time of concentration      34.093      3.752      34.093      minutes"
"      Time to Centroid      138.712      90.510      138.712      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      485.78      0.00      485.78      c.m"
"      Rainfall losses      39.876      6.201      39.876      mm"
"      Runoff depth      16.415      50.089      16.415      mm"
"      Runoff volume      141.66      0.00      141.66      c.m"
"      Runoff coefficient      0.292      0.000      0.292      "
"      Maximum flow      0.034      0.000      0.034      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.034      0.034      0.104      0.104"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.034      0.034      0.034      0.104"
" 40      HYDROGRAPH Combine 900"
"      6      Combine "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.111      c.m/sec"
"      Hydrograph volume      516.947      c.m"
"      0.034      0.034      0.034      0.111"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"

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"      0.034      0.000      0.034      0.111"
" 33      CATCHMENT 103"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      103      Catchment 103 - Laneway"
"      30.000      % Impervious"
"      0.240      Total Area"
"      225.000      Flow length"
"      0.800      Overland Slope"
"      0.168      Pervious Area"
"      225.000      Pervious length"
"      0.800      Pervious slope"
"      0.072      Impervious Area"
"      225.000      Impervious length"
"      0.800      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.292      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.904      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.026      0.000      0.034      0.111 c.m/sec"
"      Catchment 103      Pervious      Impervious      Total Area "
"      Surface Area      0.168      0.072      0.240      hectare"
"      Time of concentration      73.005      8.034      35.946      minutes"
"      Time to Centroid      187.348      96.576      135.573      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      94.57      40.53      135.10      c.m"
"      Rainfall losses      39.869      5.420      29.535      mm"
"      Runoff depth      16.421      50.870      26.755      mm"
"      Runoff volume      27.59      36.63      64.21      c.m"
"      Runoff coefficient      0.292      0.904      0.475      "
"      Maximum flow      0.004      0.026      0.026      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.026      0.026      0.034      0.111"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.026      0.026      0.026      0.111"
" 40      HYDROGRAPH Combine 800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.026      c.m/sec"
"      Hydrograph volume      64.213      c.m"
"      0.026      0.026      0.026      0.026"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.026      0.000      0.026      0.026"
" 33      CATCHMENT 104"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      104      Catchment 104 - Ex. SWMF+Embankment"
"      0.000      % Impervious"
"      0.234      Total Area"
"      8.000      Flow length"
"      20.000      Overland Slope"
"      0.234      Pervious Area"
"      8.000      Pervious length"

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"      20.000 Pervious slope"
"      0.000 Impervious Area"
"      8.000 Impervious length"
"      20.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.286 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.025      0.000      0.026      0.026 c.m/sec"
"      Catchment 104      Pervious      Impervious      Total Area
"      Surface Area      0.234      0.000      0.234      hectare"
"      Time of concentration      3.754      0.413      3.754      minutes"
"      Time to Centroid      101.003      85.977      101.003      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      131.72      0.00      131.72      c.m"
"      Rainfall losses      40.210      11.286      40.210      mm"
"      Runoff depth      16.080      45.004      16.080      mm"
"      Runoff volume      37.63      0.00      37.63      c.m"
"      Runoff coefficient      0.286      0.000      0.286
"      Maximum flow      0.025      0.000      0.025      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.025      0.025      0.026      0.026"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.025      0.025      0.025      0.026"
" 40      HYDROGRAPH Combine      800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.051      c.m/sec"
"      Hydrograph volume      101.840      c.m"
"      0.025      0.025      0.025      0.051"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.025      0.000      0.025      0.051"
" 33      CATCHMENT 105"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      105      Catchment 105 - Driveways + Ditch Within RoW"
"      20.000      % Impervious"
"      0.057      Total Area"
"      125.000      Flow length"
"      0.500      Overland Slope"
"      0.046      Pervious Area"
"      125.000      Pervious length"
"      0.500      Pervious slope"
"      0.011      Impervious Area"
"      125.000      Impervious length"
"      0.500      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.292      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.898      Impervious Runoff coefficient"

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"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.004      0.000      0.025      0.051 c.m/sec"
"      Catchment 105      Pervious      Impervious      Total Area
"      Surface Area      0.046      0.011      0.057      hectare"
"      Time of concentration      59.078      6.501      36.207      minutes"
"      Time to Centroid      169.938      94.384      137.072      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      25.67      6.42      32.09      c.m"
"      Rainfall losses      39.871      5.725      33.042      mm"
"      Runoff depth      16.419      50.565      23.248      mm"
"      Runoff volume      7.49      5.76      13.25      c.m"
"      Runoff coefficient      0.292      0.898      0.413
"      Maximum flow      0.001      0.004      0.004      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.004      0.004      0.025      0.051"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.004      0.004      0.004      0.051"
" 40      HYDROGRAPH Combine      800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.055      c.m/sec"
"      Hydrograph volume      115.092      c.m"
"      0.004      0.004      0.004      0.055"
" 40      HYDROGRAPH Confluence      800"
"      7      Confluence "
"      800      Node #"
"      External"
"      Maximum flow      0.055      c.m/sec"
"      Hydrograph volume      115.092      c.m"
"      0.004      0.055      0.004      0.000"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.004      0.055      0.055      0.000"
" 40      HYDROGRAPH Combine      900"
"      6      Combine "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.166      c.m/sec"
"      Hydrograph volume      632.039      c.m"
"      0.004      0.055      0.055      0.166"
" 40      HYDROGRAPH Confluence      900"
"      7      Confluence "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.166      c.m/sec"
"      Hydrograph volume      632.039      c.m"
"      0.004      0.166      0.055      0.000"
" 38      START/RE-START TOTALS 900"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      3.108      hectare"
"      Total Impervious area      0.358      hectare"
"      Total % impervious      11.507"
" 19      EXIT"

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"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:             Q:\42063\104\SWM\September 2021\MIDUSS\
"                                PRE"
"      Output filename:        25yrPRE.in"
"      Licensee name:          A"
"      Company                  Microsoft"
"      Date & Time last used:   9/24/2021 at 2:06:44 PM"
" 31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      3158.000 Coefficient A"
"      15.000 Constant B"
"      0.936 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity        191.271 mm/hr"
"      Total depth              68.087 mm"
"      6 025hyd Hydrograph extension used in this file"
" 33 CATCHMENT 101"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      101 Catchment 101"
"      16.000 % Impervious"
"      1.714 Total Area"
"      150.000 Flow length"
"      0.500 Overland Slope"
"      1.440 Pervious Area"
"      150.000 Pervious length"
"      0.500 Pervious slope"
"      0.274 Impervious Area"
"      150.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.346 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.912 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.126 0.000 0.000 0.000 c.m/sec"
"      Catchment 101 Pervious Impervious Total Area "
"      Surface Area 1.440 0.274 1.714 hectare"
"      Time of concentration 57.570 6.884 40.635 minutes"
"      Time to Centroid 166.282 94.277 142.223 minutes"
"      Rainfall depth 68.087 68.087 68.087 mm"
"      Rainfall volume 980.28 186.72 1167.00 c.m"
"      Rainfall losses 44.506 5.967 38.340 mm"
"      Runoff depth 23.580 62.119 29.746 mm"
"      Runoff volume 339.50 170.36 509.85 c.m"
"      Runoff coefficient 0.346 0.912 0.437 "
"      Maximum flow 0.056 0.119 0.126 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.126 0.126 0.000 0.000"

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" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.126 0.126 0.126 0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.126 c.m/sec"
"      Hydrograph volume 509.854 c.m"
"      0.126 0.126 0.126 0.126"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.126 0.000 0.126 0.126"
" 33 CATCHMENT 102"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      102 Catchment 102"
"      0.000 % Impervious"
"      0.863 Total Area"
"      50.000 Flow length"
"      0.500 Overland Slope"
"      0.863 Pervious Area"
"      50.000 Pervious length"
"      0.500 Pervious slope"
"      0.000 Impervious Area"
"      50.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.346 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.057 0.000 0.126 0.126 c.m/sec"
"      Catchment 102 Pervious Impervious Total Area "
"      Surface Area 0.863 0.000 0.863 hectare"
"      Time of concentration 29.780 3.561 29.780 minutes"
"      Time to Centroid 131.824 89.674 131.824 minutes"
"      Rainfall depth 68.087 68.087 68.087 mm"
"      Rainfall volume 587.59 0.00 587.59 c.m"
"      Rainfall losses 44.508 6.651 44.508 mm"
"      Runoff depth 23.579 61.435 23.579 mm"
"      Runoff volume 203.48 0.00 203.48 c.m"
"      Runoff coefficient 0.346 0.000 0.346 "
"      Maximum flow 0.057 0.000 0.057 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.057 0.057 0.126 0.126"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.057 0.057 0.057 0.126"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.140 c.m/sec"
"      Hydrograph volume 713.339 c.m"
"      0.057 0.057 0.057 0.140"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"

```



" 0.057 0.000 0.057 0.140"

" 33 CATCHMENT 103"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 103 Catchment 103 - Laneway"

" 30.000 % Impervious"

" 0.240 Total Area"

" 225.000 Flow length"

" 0.800 Overland Slope"

" 0.168 Pervious Area"

" 225.000 Pervious length"

" 0.800 Pervious slope"

" 0.072 Impervious Area"

" 225.000 Impervious length"

" 0.800 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.346 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.917 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.032 0.000 0.057 0.140 c.m/sec"

" Catchment 103 Pervious Impervious Total Area "

" Surface Area 0.168 0.072 0.240 hectare"

" Time of concentration 63.770 7.625 33.932 minutes"

" Time to Centroid 173.965 95.304 132.161 minutes"

" Rainfall depth 68.087 68.087 68.087 mm"

" Rainfall volume 114.39 49.02 163.41 c.m"

" Rainfall losses 44.504 5.673 32.855 mm"

" Runoff depth 23.582 62.414 35.232 mm"

" Runoff volume 39.62 44.94 84.56 c.m"

" Runoff coefficient 0.346 0.917 0.517 "

" Maximum flow 0.006 0.031 0.032 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.032 0.032 0.057 0.140"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.032 0.032 0.032 0.140"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" External"

" Maximum flow 0.032 c.m/sec"

" Hydrograph volume 84.556 c.m"

" 0.032 0.032 0.032 0.032"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.032 0.000 0.032 0.032"

" 33 CATCHMENT 104"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 104 Catchment 104 - Ex. SWMF+Embankment"

" 0.000 % Impervious"

" 0.234 Total Area"

" 8.000 Flow length"

" 20.000 Overland Slope"

" 0.234 Pervious Area"

" 8.000 Pervious length"

" 20.000 Pervious slope"

" 0.000 Impervious Area"

" 8.000 Impervious length"

" 20.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.337 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.000 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.036 0.000 0.032 0.032 c.m/sec"

" Catchment 104 Pervious Impervious Total Area "

" Surface Area 0.234 0.000 0.234 hectare"

" Time of concentration 3.279 0.392 3.279 minutes"

" Time to Centroid 99.107 85.405 99.107 minutes"

" Rainfall depth 68.087 68.087 68.087 mm"

" Rainfall volume 159.32 0.00 159.32 c.m"

" Rainfall losses 45.109 13.152 45.109 mm"

" Runoff depth 22.977 54.935 22.977 mm"

" Runoff volume 53.77 0.00 53.77 c.m"

" Runoff coefficient 0.337 0.000 0.337 "

" Maximum flow 0.036 0.000 0.036 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.036 0.036 0.032 0.032"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.036 0.036 0.036 0.032"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" External"

" Maximum flow 0.068 c.m/sec"

" Hydrograph volume 138.323 c.m"

" 0.036 0.036 0.036 0.068"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.036 0.000 0.036 0.068"

" 33 CATCHMENT 105"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 105 Catchment 105 - Driveways + Ditch Within RoW"

" 20.000 % Impervious"

" 0.057 Total Area"

" 125.000 Flow length"

" 0.500 Overland Slope"

" 0.046 Pervious Area"

" 125.000 Pervious length"

" 0.500 Pervious slope"

" 0.011 Impervious Area"

" 125.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.346 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.916 Impervious Runoff coefficient"

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"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.005      0.000      0.036      0.068 c.m/sec"
"      Catchment 105      Pervious      Impervious Total Area  "
"      Surface Area      0.046      0.011      0.057      hectare"
"      Time of concentration 51.604      6.171      33.519      minutes"
"      Time to Centroid 158.885      93.272      132.767      minutes"
"      Rainfall depth 68.087      68.087      68.087      mm"
"      Rainfall volume 31.05      7.76      38.81      c.m"
"      Rainfall losses 44.507      5.714      36.748      mm"
"      Runoff depth 23.579      62.373      31.338      mm"
"      Runoff volume 10.75      7.11      17.86      c.m"
"      Runoff coefficient 0.346      0.916      0.460      "
"      Maximum flow 0.002      0.005      0.005      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.005      0.005      0.036      0.068"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.005      0.005      0.005      0.068"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      External"
"      Maximum flow      0.073      c.m/sec"
"      Hydrograph volume 156.187      c.m"
"      0.005      0.005      0.005      0.073"
" 40 HYDROGRAPH Confluence 800"
"      7 Confluence "
"      800 Node #"
"      External"
"      Maximum flow      0.073      c.m/sec"
"      Hydrograph volume 156.187      c.m"
"      0.005      0.073      0.005      0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.005      0.073      0.073      0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow      0.213      c.m/sec"
"      Hydrograph volume 869.525      c.m"
"      0.005      0.073      0.073      0.213"
" 40 HYDROGRAPH Confluence 900"
"      7 Confluence "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow      0.213      c.m/sec"
"      Hydrograph volume 869.525      c.m"
"      0.005      0.213      0.073      0.000"
" 38 START/RE-START TOTALS 900"
"      3 Runoff Totals on EXIT"
"      Total Catchment area      3.108      hectare"
"      Total Impervious area      0.358      hectare"
"      Total % impervious      11.507"
" 19 EXIT"

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"      MIDUSS Output ----->"
"      MIDUSS version      Version 2.25 rev. 473"
"      MIDUSS created      Sunday, February 7, 2010"
"      10 Units used:      ie METRIC"
"      Job folder:      Q:\42063\104\SWM\September 2021\MIDUSS\
"      PRE"
"      Output filename:      50yrPRE.in"
"      Licensee name:      A"
"      Company      Microsoft"
"      Date & Time last used:      9/24/2021 at 2:07:27 PM"
" 31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      3886.000 Coefficient A"
"      16.000 Constant B"
"      0.950 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity 215.474 mm/hr"
"      Total depth 77.443 mm"
"      6 050hyd Hydrograph extension used in this file"
" 33 CATCHMENT 101"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      101 Catchment 101"
"      16.000 % Impervious"
"      1.714 Total Area"
"      150.000 Flow length"
"      0.500 Overland Slope"
"      1.440 Pervious Area"
"      150.000 Pervious length"
"      0.500 Pervious slope"
"      0.274 Impervious Area"
"      150.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.384 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.923 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.142      0.000      0.000      0.000 c.m/sec"
"      Catchment 101      Pervious      Impervious Total Area  "
"      Surface Area 1.440      0.274      1.714      hectare"
"      Time of concentration 52.113      6.550      37.809      minutes"
"      Time to Centroid 158.861      93.407      138.312      minutes"
"      Rainfall depth 77.443      77.443      77.443      mm"
"      Rainfall volume 1114.99      212.38      1327.37      c.m"
"      Rainfall losses 47.687      5.956      41.010      mm"
"      Runoff depth 29.756      71.486      36.433      mm"
"      Runoff volume 428.41      196.04      624.46      c.m"
"      Runoff coefficient 0.384      0.923      0.470      "
"      Maximum flow 0.079      0.131      0.142      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.142      0.142      0.000      0.000"

```

```

" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.142      0.142      0.142      0.000"
" 40      HYDROGRAPH Combine 900"
"      6      Combine "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.142      c.m/sec"
"      Hydrograph volume      624.456      c.m"
"      0.142      0.142      0.142      0.142"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.142      0.000      0.142      0.142"
" 33      CATCHMENT 102"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      102      Catchment 102"
"      0.000      % Impervious"
"      0.863      Total Area"
"      50.000      Flow length"
"      0.500      Overland Slope"
"      0.863      Pervious Area"
"      50.000      Pervious length"
"      0.500      Pervious slope"
"      0.000      Impervious Area"
"      50.000      Impervious length"
"      0.500      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.384      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.000      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.077      0.000      0.142      0.142 c.m/sec"
"      Catchment 102      Pervious      Impervious      Total Area "
"      Surface Area      0.863      0.000      0.863      hectare"
"      Time of concentration      26.957      3.388      26.957      minutes"
"      Time to Centroid      127.483      89.006      127.483      minutes"
"      Rainfall depth      77.443      77.443      77.443      mm"
"      Rainfall volume      668.33      0.00      668.33      c.m"
"      Rainfall losses      47.692      6.765      47.692      mm"
"      Runoff depth      29.750      70.677      29.750      mm"
"      Runoff volume      256.74      0.00      256.75      c.m"
"      Runoff coefficient      0.384      0.000      0.384      "
"      Maximum flow      0.077      0.000      0.077      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.077      0.077      0.142      0.142"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.077      0.077      0.077      0.142"
" 40      HYDROGRAPH Combine 900"
"      6      Combine "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.170      c.m/sec"
"      Hydrograph volume      881.201      c.m"
"      0.077      0.077      0.077      0.170"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"

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"      0.077      0.000      0.077      0.170"
" 33      CATCHMENT 103"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      103      Catchment 103 - Laneway"
"      30.000      % Impervious"
"      0.240      Total Area"
"      225.000      Flow length"
"      0.800      Overland Slope"
"      0.168      Pervious Area"
"      225.000      Pervious length"
"      0.800      Pervious slope"
"      0.072      Impervious Area"
"      225.000      Impervious length"
"      0.800      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.384      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.924      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.037      0.000      0.077      0.170 c.m/sec"
"      Catchment 103      Pervious      Impervious      Total Area "
"      Surface Area      0.168      0.072      0.240      hectare"
"      Time of concentration      57.725      7.255      32.116      minutes"
"      Time to Centroid      165.863      94.365      129.585      minutes"
"      Rainfall depth      77.443      77.443      77.443      mm"
"      Rainfall volume      130.10      55.76      185.86      c.m"
"      Rainfall losses      47.687      5.924      35.158      mm"
"      Runoff depth      29.756      71.519      42.285      mm"
"      Runoff volume      49.99      51.49      101.48      c.m"
"      Runoff coefficient      0.384      0.924      0.546      "
"      Maximum flow      0.008      0.036      0.037      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.037      0.037      0.077      0.170"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.037      0.037      0.037      0.170"
" 40      HYDROGRAPH Combine 800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.037      c.m/sec"
"      Hydrograph volume      101.483      c.m"
"      0.037      0.037      0.037      0.037"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.037      0.000      0.037      0.037"
" 33      CATCHMENT 104"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      104      Catchment 104 - Ex. SWMF+Embankment"
"      0.000      % Impervious"
"      0.234      Total Area"
"      8.000      Flow length"
"      20.000      Overland Slope"
"      0.234      Pervious Area"
"      8.000      Pervious length"

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"      20.000 Pervious slope"
"      0.000 Impervious Area"
"      8.000 Impervious length"
"      20.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.376 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.046      0.000      0.037      0.037 c.m/sec"
"      Catchment 104      Pervious      Impervious      Total Area
"      Surface Area      0.234      0.000      0.234      hectare"
"      Time of concentration      2.968      0.373      2.968      minutes"
"      Time to Centroid      97.626      84.920      97.626      minutes"
"      Rainfall depth      77.443      77.443      77.443      mm"
"      Rainfall volume      181.22      0.00      181.22      c.m"
"      Rainfall losses      48.309      14.568      48.309      mm"
"      Runoff depth      29.134      62.875      29.134      mm"
"      Runoff volume      68.17      0.00      68.17      c.m"
"      Runoff coefficient      0.376      0.000      0.376
"      Maximum flow      0.046      0.000      0.046      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.046      0.046      0.037      0.037"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.046      0.046      0.046      0.037"
" 40      HYDROGRAPH Combine      800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.083      c.m/sec"
"      Hydrograph volume      169.657      c.m"
"      0.046      0.046      0.046      0.083"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.046      0.000      0.046      0.083"
" 33      CATCHMENT 105"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      105      Catchment 105 - Driveways + Ditch Within RoW"
"      20.000      % Impervious"
"      0.057      Total Area"
"      125.000      Flow length"
"      0.500      Overland Slope"
"      0.046      Pervious Area"
"      125.000      Pervious length"
"      0.500      Pervious slope"
"      0.011      Impervious Area"
"      125.000      Impervious length"
"      0.500      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.384      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.925      Impervious Runoff coefficient"

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"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.006      0.000      0.046      0.083 c.m/sec"
"      Catchment 105      Pervious      Impervious      Total Area
"      Surface Area      0.046      0.011      0.057      hectare"
"      Time of concentration      46.713      5.871      31.363      minutes"
"      Time to Centroid      152.126      92.469      129.704      minutes"
"      Rainfall depth      77.443      77.443      77.443      mm"
"      Rainfall volume      35.31      8.83      44.14      c.m"
"      Rainfall losses      47.689      5.779      39.307      mm"
"      Runoff depth      29.754      71.664      38.136      mm"
"      Runoff volume      13.57      8.17      21.74      c.m"
"      Runoff coefficient      0.384      0.925      0.492
"      Maximum flow      0.003      0.006      0.006      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.006      0.006      0.046      0.083"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.006      0.006      0.006      0.083"
" 40      HYDROGRAPH Combine      800"
"      6      Combine "
"      800      Node #"
"      External"
"      Maximum flow      0.089      c.m/sec"
"      Hydrograph volume      191.394      c.m"
"      0.006      0.006      0.006      0.089"
" 40      HYDROGRAPH Confluence      800"
"      7      Confluence "
"      800      Node #"
"      External"
"      Maximum flow      0.089      c.m/sec"
"      Hydrograph volume      191.394      c.m"
"      0.006      0.089      0.006      0.000"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.006      0.089      0.089      0.000"
" 40      HYDROGRAPH Combine      900"
"      6      Combine "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.252      c.m/sec"
"      Hydrograph volume      1072.595      c.m"
"      0.006      0.089      0.089      0.252"
" 40      HYDROGRAPH Confluence      900"
"      7      Confluence "
"      900      Node #"
"      Torrence Creek"
"      Maximum flow      0.252      c.m/sec"
"      Hydrograph volume      1072.595      c.m"
"      0.006      0.252      0.089      0.000"
" 38      START/RE-START TOTALS 900"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      3.108      hectare"
"      Total Impervious area      0.358      hectare"
"      Total % impervious      11.507"
" 19      EXIT"

```

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:              Q:\42063\104\SWM\September 2021\MIDUSS\
"                                PRE"
"      Output filename:         100yrPRE.in"
"      Licensee name:           A"
"      Company                  Microsoft"
"      Date & Time last used:    9/24/2021 at 2:08:03 PM"
" 31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      4688.000 Coefficient A"
"      17.000 Constant B"
"      0.962 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity          239.650 mm/hr"
"      Total depth                87.263 mm"
"      6 100hyd Hydrograph extension used in this file"
" 33 CATCHMENT 101"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      101 Catchment 101"
"      16.000 % Impervious"
"      1.714 Total Area"
"      150.000 Flow length"
"      0.500 Overland Slope"
"      1.440 Pervious Area"
"      150.000 Pervious length"
"      0.500 Pervious slope"
"      0.274 Impervious Area"
"      150.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.419 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.932 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.165 0.000 0.000 0.000 c.m/sec"
"      Catchment 101 Pervious Impervious Total Area "
"      Surface Area 1.440 0.274 1.714 hectare"
"      Time of concentration 47.820 6.267 35.467 minutes"
"      Time to Centroid 152.839 92.686 134.956 minutes"
"      Rainfall depth 87.263 87.263 87.263 mm"
"      Rainfall volume 1256.38 239.31 1495.70 c.m"
"      Rainfall losses 50.658 5.957 43.506 mm"
"      Runoff depth 36.606 81.307 43.758 mm"
"      Runoff volume 527.04 222.98 750.01 c.m"
"      Runoff coefficient 0.419 0.932 0.501 "
"      Maximum flow 0.104 0.148 0.165 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.165 0.165 0.000 0.000"

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" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.165 0.165 0.165 0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.165 c.m/sec"
"      Hydrograph volume 750.010 c.m"
"      0.165 0.165 0.165 0.165"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.165 0.000 0.165 0.165"
" 33 CATCHMENT 102"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      102 Catchment 102"
"      0.000 % Impervious"
"      0.863 Total Area"
"      50.000 Flow length"
"      0.500 Overland Slope"
"      0.863 Pervious Area"
"      50.000 Pervious length"
"      0.500 Pervious slope"
"      0.000 Impervious Area"
"      50.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.419 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.101 0.000 0.165 0.165 c.m/sec"
"      Catchment 102 Pervious Impervious Total Area "
"      Surface Area 0.863 0.000 0.863 hectare"
"      Time of concentration 24.737 3.242 24.736 minutes"
"      Time to Centroid 124.000 88.466 124.000 minutes"
"      Rainfall depth 87.263 87.263 87.263 mm"
"      Rainfall volume 753.08 0.00 753.08 c.m"
"      Rainfall losses 50.668 7.034 50.668 mm"
"      Runoff depth 36.595 80.229 36.595 mm"
"      Runoff volume 315.82 0.00 315.82 c.m"
"      Runoff coefficient 0.419 0.000 0.419 "
"      Maximum flow 0.101 0.000 0.101 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.101 0.101 0.165 0.165"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.101 0.101 0.101 0.165"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.211 c.m/sec"
"      Hydrograph volume 1065.829 c.m"
"      0.101 0.101 0.101 0.211"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"

```



" 0.101 0.000 0.101 0.211"

" 33 CATCHMENT 103"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 103 Catchment 103 - Laneway"

" 30.000 % Impervious"

" 0.240 Total Area"

" 225.000 Flow length"

" 0.800 Overland Slope"

" 0.168 Pervious Area"

" 225.000 Pervious length"

" 0.800 Pervious slope"

" 0.072 Impervious Area"

" 225.000 Impervious length"

" 0.800 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.420 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.930 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.042 0.000 0.101 0.211 c.m/sec"

" Catchment 103 Pervious Impervious Total Area "

" Surface Area 0.168 0.072 0.240 hectare"

" Time of concentration 52.970 6.942 30.550 minutes"

" Time to Centroid 159.276 93.571 127.271 minutes"

" Rainfall depth 87.263 87.263 mm"

" Rainfall volume 146.60 62.83 209.43 c.m"

" Rainfall losses 50.650 6.128 37.293 mm"

" Runoff depth 36.614 81.136 49.971 mm"

" Runoff volume 61.51 58.42 119.93 c.m"

" Runoff coefficient 0.420 0.930 0.573 "

" Maximum flow 0.011 0.040 0.042 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.042 0.042 0.101 0.211"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.042 0.042 0.042 0.211"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" External"

" Maximum flow 0.042 c.m/sec"

" Hydrograph volume 119.929 c.m"

" 0.042 0.042 0.042 0.042"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.042 0.000 0.042 0.042"

" 33 CATCHMENT 104"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 104 Catchment 104 - Ex. SWMF+Embankment"

" 0.000 % Impervious"

" 0.234 Total Area"

" 8.000 Flow length"

" 20.000 Overland Slope"

" 0.234 Pervious Area"

" 8.000 Pervious length"

" 20.000 Pervious slope"

" 0.000 Impervious Area"

" 8.000 Impervious length"

" 20.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.411 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.000 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.057 0.000 0.042 0.042 c.m/sec"

" Catchment 104 Pervious Impervious Total Area "

" Surface Area 0.234 0.000 0.234 hectare"

" Time of concentration 2.724 0.357 2.724 minutes"

" Time to Centroid 96.470 84.532 96.470 minutes"

" Rainfall depth 87.263 87.263 mm"

" Rainfall volume 204.20 0.00 204.20 c.m"

" Rainfall losses 51.380 16.052 51.380 mm"

" Runoff depth 35.883 71.212 35.883 mm"

" Runoff volume 83.97 0.00 83.97 c.m"

" Runoff coefficient 0.411 0.000 0.411 "

" Maximum flow 0.057 0.000 0.057 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.057 0.057 0.042 0.042"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.057 0.057 0.057 0.042"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" External"

" Maximum flow 0.098 c.m/sec"

" Hydrograph volume 203.896 c.m"

" 0.057 0.057 0.057 0.098"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.057 0.000 0.057 0.098"

" 33 CATCHMENT 105"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 105 Catchment 105 - Driveways + Ditch Within RoW"

" 20.000 % Impervious"

" 0.057 Total Area"

" 125.000 Flow length"

" 0.500 Overland Slope"

" 0.046 Pervious Area"

" 125.000 Pervious length"

" 0.500 Pervious slope"

" 0.011 Impervious Area"

" 125.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.420 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.931 Impervious Runoff coefficient"

```

"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.007      0.000      0.057      0.098 c.m/sec"
"      Catchment 105      Pervious      Impervious Total Area  "
"      Surface Area      0.046      0.011      0.057      hectare"
"      Time of concentration 42.865      5.618      29.576      minutes"
"      Time to Centroid 146.645      91.757      127.062      minutes"
"      Rainfall depth 87.263      87.263      87.263      mm"
"      Rainfall volume 39.79      9.95      49.74      c.m"
"      Rainfall losses 50.653      6.034      41.729      mm"
"      Runoff depth 36.611      81.229      45.535      mm"
"      Runoff volume 16.69      9.26      25.95      c.m"
"      Runoff coefficient 0.420      0.931      0.522      "
"      Maximum flow 0.004      0.006      0.007      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.007      0.007      0.057      0.098"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.007      0.007      0.007      0.098"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      External"
"      Maximum flow 0.105      c.m/sec"
"      Hydrograph volume 229.851      c.m"
"      0.007      0.007      0.007      0.105"
" 40 HYDROGRAPH Confluence 800"
"      7 Confluence "
"      800 Node #"
"      External"
"      Maximum flow 0.105      c.m/sec"
"      Hydrograph volume 229.851      c.m"
"      0.007      0.105      0.007      0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.007      0.105      0.105      0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.302      c.m/sec"
"      Hydrograph volume 1295.680      c.m"
"      0.007      0.105      0.105      0.302"
" 40 HYDROGRAPH Confluence 900"
"      7 Confluence "
"      900 Node #"
"      Torrence Creek"
"      Maximum flow 0.302      c.m/sec"
"      Hydrograph volume 1295.680      c.m"
"      0.007      0.302      0.105      0.000"
" 38 START/RE-START TOTALS 900"
"      3 Runoff Totals on EXIT"
"      Total Catchment area 3.108      hectare"
"      Total Impervious area 0.358      hectare"
"      Total % impervious 11.507"
" 19 EXIT"

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"      MIDUSS created      Sunday, February 7, 2010"
"      10 Units used:      ie METRIC"
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"      PRE"
"      Output filename:      RegPRE.in"
"      Licensee name:      A"
"      Company      Microsoft"
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"      5.000 Time Step"
"      2880.000 Max. Storm length"
"      9000.000 Max. Hydrograph"
" 32 STORM Mass Curve"
"      3 Mass Curve"
"      285.000 Rainfall depth"
"      2880.000 Duration"
"      69 Q:\42063\104\SWM\September 2021\MIDUSS\POST\Hazel entire 48 hours.mrd Hurricane
Hazel (entire 48 h)"
"      Maximum intensity 53.012      mm/hr"
"      Total depth 285.000      mm"
"      8 99999hyd Hydrograph extension used in this file"
" 33 CATCHMENT 101"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      101 Catchment 101"
"      16.000 % Impervious"
"      1.714 Total Area"
"      150.000 Flow length"
"      0.500 Overland Slope"
"      1.440 Pervious Area"
"      150.000 Pervious length"
"      0.500 Pervious slope"
"      0.274 Impervious Area"
"      150.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.732 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.977 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.217      0.000      0.000      0.000 c.m/sec"
"      Catchment 101      Pervious      Impervious Total Area  "
"      Surface Area 1.440      0.274      1.714      hectare"
"      Time of concentration 63.712      11.386      53.101      minutes"
"      Time to Centroid 2575.587      2281.703      2515.991      minutes"
"      Rainfall depth 285.000      285.000      285.000      mm"
"      Rainfall volume 4103.32      781.58      4884.90      c.m"
"      Rainfall losses 76.397      6.423      65.202      mm"
"      Runoff depth 208.603      278.577      219.798      mm"
"      Runoff volume 3003.38      763.97      3767.35      c.m"
"      Runoff coefficient 0.732      0.977      0.771      "
"      Maximum flow 0.188      0.042      0.217      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.217      0.217      0.000      0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"

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"          0.217      0.217      0.217      0.000"
" 40      HYDROGRAPH Combine 900"
" 6      Combine "
" 900     Node #"
"      Torrence Creek"
"      Maximum flow          0.217      c.m/sec"
"      Hydrograph volume      3767.346      c.m"
"          0.217      0.217      0.217      0.217"
" 40      HYDROGRAPH Start - New Tributary"
" 2      Start - New Tributary"
"          0.217      0.000      0.217      0.217"
" 33      CATCHMENT 102"
" 1      Triangular SCS"
" 1      Equal length"
" 1      SCS method"
" 102     Catchment 102"
" 0.000   % Impervious"
" 0.863   Total Area"
" 50.000   Flow length"
" 0.500   Overland Slope"
" 0.863   Pervious Area"
" 50.000   Pervious length"
" 0.500   Pervious slope"
" 0.000   Impervious Area"
" 50.000   Impervious length"
" 0.500   Impervious slope"
" 0.250   Pervious Manning 'n'"
" 74.000   Pervious SCS Curve No."
" 0.732   Pervious Runoff coefficient"
" 0.100   Pervious Ia/S coefficient"
" 8.924   Pervious Initial abstraction"
" 0.015   Impervious Manning 'n'"
" 98.000   Impervious SCS Curve No."
" 0.000   Impervious Runoff coefficient"
" 0.100   Impervious Ia/S coefficient"
" 0.518   Impervious Initial abstraction"
"          0.123      0.000      0.217      0.217 c.m/sec"
"      Catchment 102      Pervious      Impervious      Total Area "
"      Surface Area      0.863      0.000      0.863      hectare"
"      Time of concentration      32.957      5.890      32.957      minutes"
"      Time to Centroid      2532.306      2271.773      2532.307      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      2459.55      0.00      2459.55      c.m"
"      Rainfall losses      76.445      8.151      76.445      mm"
"      Runoff depth      208.555      276.849      208.555      mm"
"      Runoff volume      1799.83      0.00      1799.83      c.m"
"      Runoff coefficient      0.732      0.000      0.732      "
"      Maximum flow      0.123      0.000      0.123      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
" 4      Add Runoff "
"          0.123      0.123      0.217      0.217"
" 40      HYDROGRAPH Copy to Outflow"
" 8      Copy to Outflow"
"          0.123      0.123      0.123      0.217"
" 40      HYDROGRAPH Combine 900"
" 6      Combine "
" 900     Node #"
"      Torrence Creek"
"      Maximum flow          0.331      c.m/sec"
"      Hydrograph volume      5567.176      c.m"
"          0.123      0.123      0.123      0.331"
" 40      HYDROGRAPH Start - New Tributary"
" 2      Start - New Tributary"
"          0.123      0.000      0.123      0.331"
" 33      CATCHMENT 103"

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" 1      Triangular SCS"
" 1      Equal length"
" 1      SCS method"
" 103     Catchment 103 - Laneway"
" 30.000   % Impervious"
" 0.240   Total Area"
" 225.000   Flow length"
" 0.800   Overland Slope"
" 0.168   Pervious Area"
" 225.000   Pervious length"
" 0.800   Pervious slope"
" 0.072   Impervious Area"
" 225.000   Impervious length"
" 0.800   Impervious slope"
" 0.250   Pervious Manning 'n'"
" 74.000   Pervious SCS Curve No."
" 0.732   Pervious Runoff coefficient"
" 0.100   Pervious Ia/S coefficient"
" 8.924   Pervious Initial abstraction"
" 0.015   Impervious Manning 'n'"
" 98.000   Impervious SCS Curve No."
" 0.978   Impervious Runoff coefficient"
" 0.100   Impervious Ia/S coefficient"
" 0.518   Impervious Initial abstraction"
"          0.029      0.000      0.123      0.331 c.m/sec"
"      Catchment 103      Pervious      Impervious      Total Area "
"      Surface Area      0.168      0.072      0.240      hectare"
"      Time of concentration      70.573      12.612      49.473      minutes"
"      Time to Centroid      2585.268      2283.863      2475.546      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      478.80      205.20      684.00      c.m"
"      Rainfall losses      76.375      6.354      55.369      mm"
"      Runoff depth      208.625      278.646      229.631      mm"
"      Runoff volume      350.49      200.62      551.12      c.m"
"      Runoff coefficient      0.732      0.978      0.806      "
"      Maximum flow      0.021      0.011      0.029      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
" 4      Add Runoff "
"          0.029      0.029      0.123      0.331"
" 40      HYDROGRAPH Copy to Outflow"
" 8      Copy to Outflow"
"          0.029      0.029      0.029      0.331"
" 40      HYDROGRAPH Combine 800"
" 6      Combine "
" 800     Node #"
"      External"
"      Maximum flow          0.029      c.m/sec"
"      Hydrograph volume      551.115      c.m"
"          0.029      0.029      0.029      0.029"
" 40      HYDROGRAPH Start - New Tributary"
" 2      Start - New Tributary"
"          0.029      0.000      0.029      0.029"
" 33      CATCHMENT 104"
" 1      Triangular SCS"
" 1      Equal length"
" 1      SCS method"
" 104     Catchment 104 - Ex. SWMF+Embankment"
" 0.000   % Impervious"
" 0.234   Total Area"
" 8.000   Flow length"
" 20.000   Overland Slope"
" 0.234   Pervious Area"
" 8.000   Pervious length"
" 20.000   Pervious slope"
" 0.000   Impervious Area"

```

"	8.000	Impervious length"			
"	20.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	74.000	Pervious SCS Curve No."			
"	0.712	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n' "			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.030	0.000	0.029	0.029 c.m/sec"	
"	Catchment 104	Pervious	Impervious	Total Area	"
"	Surface Area	0.234	0.000	0.234	hectare"
"	Time of concentration	3.629	0.649	3.629	minutes"
"	Time to Centroid	2485.855	2238.670	2485.855	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	666.90	0.00	666.90	c.m"
"	Rainfall losses	82.021	24.612	82.021	mm"
"	Runoff depth	202.979	260.388	202.979	mm"
"	Runoff volume	474.97	0.00	474.97	c.m"
"	Runoff coefficient	0.712	0.000	0.712	"
"	Maximum flow	0.030	0.000	0.030	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.030	0.030	0.029	0.029"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.030	0.030	0.030	0.029"	
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	External"				
"	Maximum flow		0.057	c.m/sec"	
"	Hydrograph volume		1026.085	c.m"	
"	0.030	0.030	0.030	0.057"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.030	0.000	0.030	0.057"	
" 33	CATCHMENT 105"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	105 Catchment 105 - Driveways + Ditch Within RoW"				
"	% Impervious"				
"	0.057 Total Area"				
"	125.000 Flow length"				
"	0.500 Overland Slope"				
"	0.046 Pervious Area"				
"	125.000 Pervious length"				
"	0.500 Pervious slope"				
"	0.011 Impervious Area"				
"	125.000 Impervious length"				
"	0.500 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	74.000 Pervious SCS Curve No."				
"	0.732 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No."				
"	0.977 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				

"	0.007	0.000	0.030	0.057 c.m/sec"	
"	Catchment 105	Pervious	Impervious	Total Area	"
"	Surface Area	0.046	0.011	0.057	hectare"
"	Time of concentration	57.110	10.206	45.374	minutes"
"	Time to Centroid	2566.321	2280.284	2494.753	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	129.96	32.49	162.45	c.m"
"	Rainfall losses	76.391	6.548	62.422	mm"
"	Runoff depth	208.609	278.452	222.578	mm"
"	Runoff volume	95.13	31.74	126.87	c.m"
"	Runoff coefficient	0.732	0.977	0.781	"
"	Maximum flow	0.006	0.002	0.007	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.007	0.007	0.030	0.057"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.007	0.007	0.007	0.057"	
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	External"				
"	Maximum flow		0.065	c.m/sec"	
"	Hydrograph volume		1152.954	c.m"	
"	0.007	0.007	0.007	0.065"	
" 40	HYDROGRAPH Confluence	800"			
"	7 Confluence "				
"	800 Node #"				
"	External"				
"	Maximum flow		0.065	c.m/sec"	
"	Hydrograph volume		1152.954	c.m"	
"	0.007	0.065	0.007	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.007	0.065	0.065	0.000"	
" 40	HYDROGRAPH Combine	900"			
"	6 Combine "				
"	900 Node #"				
"	Torrence Creek"				
"	Maximum flow		0.392	c.m/sec"	
"	Hydrograph volume		6720.128	c.m"	
"	0.007	0.065	0.065	0.392"	
" 40	HYDROGRAPH Confluence	900"			
"	7 Confluence "				
"	900 Node #"				
"	Torrence Creek"				
"	Maximum flow		0.392	c.m/sec"	
"	Hydrograph volume		6720.128	c.m"	
"	0.007	0.392	0.065	0.000"	
" 38	START/RE-START TOTALS 900"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area		3.108	hectare"	
"	Total Impervious area		0.358	hectare"	
"	Total % impervious		11.507"		
" 19	EXIT"				

## Appendix C

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# **Proposed Conditions Catchment Parameters and MIDUSS Modelling**



**Arkell Road**  
**STORMWATER MANAGEMENT**  
 Guelph, Ontario

Project Number: 42063-104  
 Date: October 8, 2021  
 Design By: AJC  
 File: Q:\42063\104\SWM\September 2021\42063-104 Master SWM Facility Design Sheet.xlsx

**HYDROLOGIC PARAMETERS**  
 Post-Development Conditions

Sub-Catchment Number	Area	Overland Slope	Overland Length	SCS Curve Number			Percent Impervious	Land Use	Comment
				Pervious (AMC II)	Pervious (AMC III)	Impervious			
	(ha)	(%)	(m)				(%)		
<b>To SWMF</b>									
201-1	0.519	0.75	60	74	87	98	60%	Residential/RoW	Street A, Front Yards, Driveways, Roofs to SWMF
201-2	0.066	0.5	15	74	87	98	70%	RoW	Minor to SWMF, Emergency to Arkell Rd
201-3	0.234	2.0	15	74	87	98	75%	Multi-Residential	Block 1 to SWMF
201-4	0.065	2.0	10	74	87	98	100%	Multi-Residential	Block 1 Roofs to Gallery
201-5	0.074	2.0	15	74	87	98	85%	Multi-Residential	Block 2 to SWMF
201-6	0.032	2.0	10	74	87	98	100%	Multi-Residential	Block 2 Roofs to Gallery
201-7	0.211	10	10	74	87	98	40%	SWMF	Proposed SWMF
	<u>1.201</u>						<u>64.7%</u>		
<b>Directly to Torrance Creek</b>									
202-1	0.863	0.5	50	74	87	98	0%	Wetland	Wetland/Forested Area/Torrance Creek
202-2	0.184	2.0	10	74	87	98	24%	Residential	Rear Yards/Roofs to Torrance Creek
	<u>1.047</u>						<u>4.2%</u>		
<b>Directed to Future Parklands/Torrance</b>									
203-1	0.216	2	130	74	87	98	30%	Park	Future Park Trail
203-2	0.113	33	10	74	87	98	0%	Residential	Embankments to Trail
203-3	0.092	2	10	74	87	98	100%	Residential	Roofs Areas to Galleries prior to Torrance
204	0.198	20	8	74	87	98	0%	SWMF	Ex. SWMF + Embankments
	<u>0.619</u>						<u>25.3%</u>		
<b>To Arkell Road</b>									
205-1	0.098	2	15	74	87	98	25%	Residential/RoW	Flows to Arkell Road Infil Gallery
205-2	0.108	5	25	74	87	98	40%	Residential/RoW	Flows to Arkell Road Stone Energy Dissipators
	<u>0.206</u>						<u>32.9%</u>		
<b>To Adjacent Ex. SWMF</b>									
206	0.035	1.25	20	74	87	98	70.0%	Residential/RoW	Dawes Avenue to adjacent SWMF
<b>Total</b>	<u><u>3.108</u></u>						<u><u>34.4%</u></u>		

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:             Q:\42063\104\SWM\September 2021\MIDUSS\
"                                POST"
"      Output filename:        25mm4hrPOSTREV.in"
"      Licensee name:          A"
"      Company                 Microsoft"
"      Date & Time last used:   10/5/2021 at 1:30:54 PM"
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"      5.000 Time Step"
"      240.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      509.000 Coefficient A"
"      6.000 Constant B"
"      0.799 Exponent C"
"      0.400 Fraction R"
"      240.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity        71.966 mm/hr"
"      Total depth              25.028 mm"
"      6 025hyd Hydrograph extension used in this file"
" 33 CATCHMENT 2011"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2011 Catchment 201-1"
"      60.000 % Impervious"
"      0.519 Total Area"
"      60.000 Flow length"
"      0.750 Overland Slope"
"      0.208 Pervious Area"
"      60.000 Pervious length"
"      0.750 Pervious slope"
"      0.311 Impervious Area"
"      60.000 Impervious length"
"      0.750 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.806 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.042 0.000 0.000 0.000 c.m/sec"
"      Catchment 2011 Pervious Impervious Total Area "
"      Surface Area 0.208 0.311 0.519 hectare"
"      Time of concentration 89.369 5.385 11.707 minutes"
"      Time to Centroid 246.824 125.697 134.814 minutes"
"      Rainfall depth 25.028 25.028 25.028 mm"
"      Rainfall volume 51.96 77.94 129.89 c.m"
"      Rainfall losses 22.566 4.867 11.947 mm"
"      Runoff depth 2.461 20.161 13.081 mm"
"      Runoff volume 5.11 62.78 67.89 c.m"
"      Runoff coefficient 0.098 0.806 0.523 "
"      Maximum flow 0.000 0.042 0.042 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.042 0.042 0.000 0.000"

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" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.042 0.042 0.042 0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.042 c.m/sec"
"      Hydrograph volume 67.891 c.m"
"      0.042 0.042 0.042 0.042"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.042 0.000 0.042 0.042"
" 33 CATCHMENT 2012"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2012 Catchment 201-2"
"      70.000 % Impervious"
"      0.066 Total Area"
"      15.000 Flow length"
"      0.500 Overland Slope"
"      0.020 Pervious Area"
"      15.000 Pervious length"
"      0.500 Pervious slope"
"      0.046 Impervious Area"
"      15.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.793 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.007 0.000 0.042 0.042 c.m/sec"
"      Catchment 2012 Pervious Impervious Total Area "
"      Surface Area 0.020 0.046 0.066 hectare"
"      Time of concentration 43.932 2.647 4.729 minutes"
"      Time to Centroid 193.449 121.411 125.044 minutes"
"      Rainfall depth 25.028 25.028 25.028 mm"
"      Rainfall volume 4.96 11.56 16.52 c.m"
"      Rainfall losses 22.567 5.170 10.389 mm"
"      Runoff depth 2.461 19.858 14.639 mm"
"      Runoff volume 0.49 9.17 9.66 c.m"
"      Runoff coefficient 0.098 0.793 0.585 "
"      Maximum flow 0.000 0.007 0.007 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.007 0.007 0.042 0.042"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.007 0.007 0.007 0.042"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.047 c.m/sec"
"      Hydrograph volume 77.552 c.m"
"      0.007 0.007 0.007 0.047"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"

```



" 0.007 0.000 0.007 0.047"

" 33 CATCHMENT 2013"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2013 Catchment 201-3"

" 75.000 % Impervious"

" 0.234 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.058 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.176 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.098 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.797 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.026 0.000 0.007 0.047 c.m/sec"

" Catchment 2013 Pervious Impervious Total Area "

" Surface Area 0.058 0.176 0.234 hectare"

" Time of concentration 28.984 1.747 2.822 minutes"

" Time to Centroid 175.889 119.783 121.998 minutes"

" Rainfall depth 25.028 25.028 25.028 mm"

" Rainfall volume 14.64 43.92 58.56 c.m"

" Rainfall losses 22.568 5.082 9.454 mm"

" Runoff depth 2.460 19.946 15.574 mm"

" Runoff volume 1.44 35.00 36.44 c.m"

" Runoff coefficient 0.098 0.797 0.622 "

" Maximum flow 0.000 0.026 0.026 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.026 0.026 0.007 0.047"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.026 0.026 0.026 0.047"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.066 c.m/sec"

" Hydrograph volume 113.995 c.m"

" 0.026 0.026 0.026 0.066"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.026 0.000 0.026 0.066"

" 33 CATCHMENT 2014"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2014 Catchment 201-4"

" 100.000 % Impervious"

" 0.065 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.000 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.000 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.794 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.010 0.000 0.026 0.066 c.m/sec"

" Catchment 2014 Pervious Impervious Total Area "

" Surface Area 0.000 0.065 0.065 hectare"

" Time of concentration 22.725 1.369 1.369 minutes"

" Time to Centroid 168.546 119.217 119.217 minutes"

" Rainfall depth 25.028 25.028 25.028 mm"

" Rainfall volume 0.00 16.27 16.27 c.m"

" Rainfall losses 22.568 5.156 5.156 mm"

" Runoff depth 2.459 19.872 19.872 mm"

" Runoff volume 0.00 12.92 12.92 c.m"

" Runoff coefficient 0.000 0.794 0.794 "

" Maximum flow 0.000 0.010 0.010 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.010 0.010 0.026 0.066"

" 57 TRENCH Design d/s of 2014"

" 0.010 Peak inflow"

" 12.917 Hydrograph volume"

" 337.000 Ground elevation"

" 334.200 Downstream trench invert"

" 1.000 Trench height"

" 333.200 Water table elevation"

" 5.000 Trench top width"

" 5.000 Trench bottom width"

" 40.000 Voids ratio (%) "

" 30.000 Hydraulic conductivity"

" 0.000 Trench gradient (%) "

" 10.000 Trench length"

" 1.000 Include base width"

" 29. Number of stages"

" Level Discharge Volume"

" 334.200 0.000 0.0"

" 334.300 0.000 2.0"

" 334.400 0.000 4.0"

" 334.500 0.000 6.0"

" 334.600 0.000 8.0"

" 334.700 0.000 10.0"

" 334.800 0.000 12.0"

" 334.900 0.000 14.0"

" 335.000 0.000 16.0"

" 335.100 0.000 18.0"

" 335.200 0.000 20.0"

" 335.300 0.004 20.1"

" 335.400 0.006 20.2"

" 335.500 0.008 20.3"

" 335.600 0.010 20.5"

" 335.700 0.011 20.6"

" 335.800 0.013 20.7"

" 335.900 0.014 20.8"

" 336.000 0.015 20.9"

" 336.100 0.016 21.0"

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"          336.200      0.017      21.1"
"          336.300      0.018      21.2"
"          336.400      0.019      21.4"
"          336.500      0.020      21.5"
"          336.600      0.020      21.6"
"          336.700      0.021      21.7"
"          336.800      0.022      21.8"
"          336.900      0.023      21.9"
"          337.000      0.023      22.0"
"
"      1.  TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      20.000      0.000      1.000      1.000      0.000"
"
"      1.  MANHOLE"
"      Access"
"      diameter"
"      1.200"
"
"      1.  OUTFLOW PIPE"
"      0.  Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      20.000      0.100      0.013      0.500"
"
"      Peak outflow      0.000      c.m/sec"
"      Outflow volume      0.002      c.m"
"      Peak exfiltration      0.001      c.m/sec"
"      Exfiltration volume      12.913      c.m"
"      Maximum level      334.633      metre"
"      Maximum storage      8.658      c.m"
"      Centroidal lag      4.967      hours"
"      Infiltration area 2 sides      8.658      sq.metre"
"      Infiltration Base area      50.000      sq.metre"
"      0.010      0.010      0.000      0.001 c.m/sec"
"
"  40      HYDROGRAPH Combine 900"
"
"      6  Combine "
"
"  900  Node #"
"      SWMF"
"
"      Maximum flow      0.066      c.m/sec"
"      Hydrograph volume      113.998      c.m"
"      0.010      0.010      0.000      0.066"
"
"  40      HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.010      0.000      0.000      0.066"
"
"  33      CATCHMENT 2015"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"
"      2015  Catchment 201-5"
"
"      85.000  % Impervious"
"      0.074  Total Area"
"      15.000  Flow length"
"      2.000  Overland Slope"
"      0.011  Pervious Area"
"      15.000  Pervious length"
"      2.000  Pervious slope"
"      0.063  Impervious Area"
"      15.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.098  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.797  Impervious Runoff coefficient"

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"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.009      0.000      0.000      0.066 c.m/sec"
"
"      Catchment 2015      Pervious      Impervious Total Area "
"      Surface Area      0.011      0.063      0.074      hectare"
"      Time of concentration      28.984      1.747      2.327      minutes"
"      Time to Centroid      175.889      119.783      120.977      minutes"
"      Rainfall depth      25.028      25.028      25.028      mm"
"      Rainfall volume      2.78      15.74      18.52      c.m"
"      Rainfall losses      22.568      5.082      7.705      mm"
"      Runoff depth      2.460      19.946      17.323      mm"
"      Runoff volume      0.27      12.55      12.82      c.m"
"      Runoff coefficient      0.098      0.797      0.692      "
"      Maximum flow      0.000      0.009      0.009      c.m/sec"
"
"  40      HYDROGRAPH Add Runoff "
"
"      4  Add Runoff "
"      0.009      0.009      0.000      0.066"
"
"  40      HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.009      0.009      0.009      0.066"
"
"  40      HYDROGRAPH Combine 900"
"      6  Combine "
"
"  900  Node #"
"      SWMF"
"
"      Maximum flow      0.075      c.m/sec"
"      Hydrograph volume      126.817      c.m"
"      0.009      0.009      0.009      0.075"
"
"  40      HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.009      0.000      0.009      0.075"
"
"  33      CATCHMENT 2016"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"
"      2016  Catchment 201-6"
"
"      100.000  % Impervious"
"      0.032  Total Area"
"      10.000  Flow length"
"      2.000  Overland Slope"
"      0.000  Pervious Area"
"      10.000  Pervious length"
"      2.000  Pervious slope"
"      0.032  Impervious Area"
"      10.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.000  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.794  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"
"      0.518  Impervious Initial abstraction"
"      0.005      0.000      0.009      0.075 c.m/sec"
"
"      Catchment 2016      Pervious      Impervious Total Area "
"      Surface Area      0.000      0.032      0.032      hectare"
"      Time of concentration      22.725      1.369      1.369      minutes"
"      Time to Centroid      168.546      119.217      119.217      minutes"
"      Rainfall depth      25.028      25.028      25.028      mm"
"      Rainfall volume      0.00      8.01      8.01      c.m"
"      Rainfall losses      22.568      5.156      5.156      mm"
"      Runoff depth      2.459      19.872      19.872      mm"
"      Runoff volume      0.00      6.36      6.36      c.m"

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"      Runoff coefficient      0.000      0.794      0.794      "
"      Maximum flow           0.000      0.005      0.005      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.005      0.005      0.009      0.075"
" 57  TRENCH Design d/s of 2016"
"      0.005  Peak inflow"
"      6.359  Hydrograph volume"
"      337.000  Ground elevation"
"      334.200  Downstream trench invert"
"      1.000  Trench height"
"      333.200  Water table elevation"
"      5.000  Trench top width"
"      5.000  Trench bottom width"
"      40.000  Voids ratio (%)"
"      30.000  Hydraulic conductivity"
"      0.000  Trench gradient (%)"
"      5.000  Trench length"
"      1.000  Include base width"
"      29.  Number of stages"
"          Level Discharge      Volume"
"          334.200      0.000      0.0"
"          334.300      0.000      1.0"
"          334.400      0.000      2.0"
"          334.500      0.000      3.0"
"          334.600      0.000      4.0"
"          334.700      0.000      5.0"
"          334.800      0.000      6.0"
"          334.900      0.000      7.0"
"          335.000      0.000      8.0"
"          335.100      0.000      9.0"
"          335.200      0.000      10.0"
"          335.300      0.002      10.1"
"          335.400      0.006      10.2"
"          335.500      0.008      10.3"
"          335.600      0.010      10.5"
"          335.700      0.011      10.6"
"          335.800      0.013      10.7"
"          335.900      0.014      10.8"
"          336.000      0.015      10.9"
"          336.100      0.016      11.0"
"          336.200      0.017      11.1"
"          336.300      0.018      11.2"
"          336.400      0.019      11.4"
"          336.500      0.020      11.5"
"          336.600      0.020      11.6"
"          336.700      0.021      11.7"
"          336.800      0.022      11.8"
"          336.900      0.023      11.9"
"          337.000      0.023      12.0"
" 1.  TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      10.000      0.000      2.000      1.000      0.000"
" 1.  MANHOLE"
"      Access"
"      diameter"
"      1.200"
" 1.  OUTFLOW PIPE"
" 0.  Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      10.000      0.100      0.013      0.500"
"      Peak outflow      0.000      c.m/sec"
"      Outflow volume      0.002      c.m"

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"      Peak exfiltration      0.000      c.m/sec"
"      Exfiltration volume      6.356      c.m"
"      Maximum level      334.625      metre"
"      Maximum storage      4.252      c.m"
"      Centroidal lag      4.929      hours"
"      Infiltration area 2 sides      4.252      sq.metre"
"      Infiltration Base area      25.000      sq.metre"
"          0.005      0.005      0.000      0.000 c.m/sec"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"
"      Maximum flow      0.075      c.m/sec"
"      Hydrograph volume      126.819      c.m"
"          0.005      0.005      0.000      0.075"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.005      0.000      0.000      0.075"
" 33  CATCHMENT 2017"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      2017  Catchment 201-7 - SWMF"
"      40.000  % Impervious"
"      0.211  Total Area"
"      10.000  Flow length"
"      10.000  Overland Slope"
"      0.127  Pervious Area"
"      10.000  Pervious length"
"      10.000  Pervious slope"
"      0.084  Impervious Area"
"      10.000  Impervious length"
"      10.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.098  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.772  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.013      0.000      0.000      0.075 c.m/sec"
"      Catchment 2017      Pervious      Impervious      Total Area "
"      Surface Area      0.127      0.084      0.211      hectare"
"      Time of concentration      14.022      0.845      2.956      minutes"
"      Time to Centroid      158.323      118.369      124.770      minutes"
"      Rainfall depth      25.028      25.028      25.028      mm"
"      Rainfall volume      31.69      21.12      52.81      c.m"
"      Rainfall losses      22.570      5.703      15.823      mm"
"      Runoff depth      2.458      19.325      9.205      mm"
"      Runoff volume      3.11      16.31      19.42      c.m"
"      Runoff coefficient      0.098      0.772      0.368      "
"      Maximum flow      0.001      0.013      0.013      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.013      0.013      0.000      0.075"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.013      0.013      0.013      0.075"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"

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"	Maximum flow	0.088	c.m/sec"
"	Hydrograph volume	146.241	c.m"
"	0.013 0.013 0.013 0.088"		
" 40	HYDROGRAPH Confluence	900"	
"	7 Confluence "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.088	c.m/sec"
"	Hydrograph volume	146.241	c.m"
"	0.013 0.088 0.013 0.000"		
" 54	POND DESIGN"		
"	0.088 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	146.2 Hydrograph volume	c.m"	
"	13. Number of stages"		
"	334.400 Minimum water level	metre"	
"	335.600 Maximum water level	metre"	
"	334.400 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.400 0.000	0.000"	
"	334.500 0.00150	40.000"	
"	334.600 0.00230	85.000"	
"	334.700 0.00290	135.000"	
"	334.800 0.03110	189.000"	
"	334.900 0.04300	249.000"	
"	335.000 0.05220	314.000"	
"	335.100 0.05990	384.000"	
"	335.200 0.06680	457.000"	
"	335.300 0.07300	534.000"	
"	335.400 0.1628	614.000"	
"	335.500 0.4275	698.000"	
"	335.600 0.9140	786.000"	
"	Peak outflow	0.003	c.m/sec"
"	Maximum level	334.669	metre"
"	Maximum storage	119.302	c.m"
"	Centroidal lag	11.547	hours"
"	0.013 0.088 0.003 0.000 c.m/sec"		
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.013 0.003 0.003 0.000"		
" 54	POND DESIGN"		
"	0.003 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	135.9 Hydrograph volume	c.m"	
"	10. Number of stages"		
"	334.200 Minimum water level	metre"	
"	335.100 Maximum water level	metre"	
"	334.200 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.200 0.000	0.000"	
"	334.300 0.00191	18.000"	
"	334.400 0.00207	38.000"	
"	334.500 0.00225	60.000"	
"	334.600 0.00243	83.000"	
"	334.700 0.00261	109.000"	
"	334.800 0.00281	136.000"	
"	334.900 0.1543	165.000"	
"	335.000 0.4628	196.000"	
"	335.100 0.9060	229.000"	
"	Peak outflow	0.002	c.m/sec"
"	Maximum level	334.339	metre"
"	Maximum storage	25.757	c.m"
"	Centroidal lag	13.146	hours"

"	0.013 0.003 0.002 0.000 c.m/sec"		
" 40	HYDROGRAPH Combine	800"	
"	6 Combine "		
"	800 Node #"		
"	TORRANCE CREEK"		
"	Maximum flow	0.002	c.m/sec"
"	Hydrograph volume	130.201	c.m"
"	0.013 0.003 0.002 0.002"		
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.013 0.000 0.002 0.002"		
" 33	CATCHMENT 2021"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2021 Catchment 202-1"		
"	0.000 % Impervious"		
"	0.863 Total Area"		
"	50.000 Flow length"		
"	0.500 Overland Slope"		
"	0.863 Pervious Area"		
"	50.000 Pervious length"		
"	0.500 Pervious slope"		
"	0.000 Impervious Area"		
"	50.000 Impervious length"		
"	0.500 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	74.000 Pervious SCS Curve No."		
"	0.098 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.000 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.002 0.000 0.002 0.002 c.m/sec"		
"	Catchment 2021 Pervious Impervious Total Area "		
"	Surface Area 0.863 0.000 0.863	hectare"	
"	Time of concentration 90.470 5.452 90.469	minutes"	
"	Time to Centroid 248.117 125.802 248.116	minutes"	
"	Rainfall depth 25.028 25.028 25.028	mm"	
"	Rainfall volume 215.99 0.00 215.99	c.m"	
"	Rainfall losses 22.566 4.865 22.566	mm"	
"	Runoff depth 2.461 20.162 2.461	mm"	
"	Runoff volume 21.24 0.00 21.24	c.m"	
"	Runoff coefficient 0.098 0.000 0.098	"	
"	Maximum flow 0.002 0.000 0.002	c.m/sec"	
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.002 0.002 0.002 0.002"		
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.002 0.002 0.002 0.002"		
" 40	HYDROGRAPH Combine	800"	
"	6 Combine "		
"	800 Node #"		
"	TORRANCE CREEK"		
"	Maximum flow	0.003	c.m/sec"
"	Hydrograph volume	151.443	c.m"
"	0.002 0.002 0.002 0.003"		
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.002 0.000 0.002 0.003"		
" 33	CATCHMENT 2022"		

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"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2022 Catchment 202-2"
" 24.000 % Impervious"
"      0.184 Total Area"
" 10.000 Flow length"
"      2.000 Overland Slope"
"      0.140 Pervious Area"
" 10.000 Pervious length"
"      2.000 Pervious slope"
"      0.044 Impervious Area"
" 10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
"      0.794 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.007      0.000      0.002      0.003 c.m/sec"
"      Catchment 2022      Pervious      Impervious Total Area
"      Surface Area      0.140      0.044      0.184      hectare"
"      Time of concentration      22.725      1.369      7.382      minutes"
"      Time to Centroid      168.546      119.217      133.106      minutes"
"      Rainfall depth      25.028      25.028      25.028      mm"
"      Rainfall volume      35.00      11.05      46.05      c.m"
"      Rainfall losses      22.568      5.156      18.389      mm"
"      Runoff depth      2.459      19.872      6.638      mm"
"      Runoff volume      3.44      8.78      12.21      c.m"
"      Runoff coefficient      0.098      0.794      0.265      "
"      Maximum flow      0.001      0.007      0.007      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.007      0.007      0.002      0.003"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.007      0.007      0.007      0.003"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.007      c.m/sec"
"      Hydrograph volume      163.658      c.m"
"      0.007      0.007      0.007      0.007"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.007      0.000      0.007      0.007"
" 33 CATCHMENT 2031"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2031 Catchment 203-1"
" 30.000 % Impervious"
"      0.216 Total Area"
" 130.000 Flow length"
"      2.000 Overland Slope"
"      0.151 Pervious Area"
" 130.000 Pervious length"
"      2.000 Pervious slope"
"      0.065 Impervious Area"

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" 130.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.802 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.009      0.000      0.007      0.007 c.m/sec"
"      Catchment 2031      Pervious      Impervious Total Area
"      Surface Area      0.151      0.065      0.216      hectare"
"      Time of concentration      105.894      6.381      28.521      minutes"
"      Time to Centroid      266.233      127.297      158.207      minutes"
"      Rainfall depth      25.028      25.028      25.028      mm"
"      Rainfall volume      37.84      16.22      54.06      c.m"
"      Rainfall losses      22.566      4.956      17.283      mm"
"      Runoff depth      2.461      20.072      7.745      mm"
"      Runoff volume      3.72      13.01      16.73      c.m"
"      Runoff coefficient      0.098      0.802      0.309      "
"      Maximum flow      0.000      0.009      0.009      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.009      0.009      0.007      0.007"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.009      0.009      0.009      0.007"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow      0.009      c.m/sec"
"      Hydrograph volume      16.728      c.m"
"      0.009      0.009      0.009      0.009"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.009      0.000      0.009      0.009"
" 33 CATCHMENT 2032"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2032 Catchment 203-2"
"      0.000 % Impervious"
"      0.113 Total Area"
"      10.000 Flow length"
"      33.000 Overland Slope"
"      0.113 Pervious Area"
"      10.000 Pervious length"
"      33.000 Pervious slope"
"      0.000 Impervious Area"
"      10.000 Impervious length"
"      33.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"

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"	0.001	0.000	0.009	0.009 c.m/sec"
"	Catchment 2032	Pervious	Impervious	Total Area "
"	Surface Area	0.113	0.000	0.113 hectare"
"	Time of concentration	9.801	0.591	9.801 minutes"
"	Time to Centroid	153.463	118.873	153.462 minutes"
"	Rainfall depth	25.028	25.028	25.028 mm"
"	Rainfall volume	28.28	0.00	28.28 c.m"
"	Rainfall losses	22.581	6.287	22.581 mm"
"	Runoff depth	2.446	18.741	2.446 mm"
"	Runoff volume	2.76	0.00	2.76 c.m"
"	Runoff coefficient	0.098	0.000	0.098 "
"	Maximum flow	0.001	0.000	0.001 c.m/sec"
" 40	HYDROGRAPH Add Runoff "			
"	4 Add Runoff "			
"	0.001	0.001	0.009	0.009"
" 40	HYDROGRAPH Copy to Outflow"			
"	8 Copy to Outflow"			
"	0.001	0.001	0.001	0.009"
" 40	HYDROGRAPH Combine	700"		
"	6 Combine "			
"	700 Node #"			
"	CULVERT"			
"	Maximum flow		0.010	c.m/sec"
"	Hydrograph volume		19.493	c.m"
"	0.001	0.001	0.001	0.010"
" 40	HYDROGRAPH Start - New Tributary"			
"	2 Start - New Tributary"			
"	0.001	0.000	0.001	0.010"
" 33	CATCHMENT 2033"			
"	1 Triangular SCS"			
"	1 Equal length"			
"	1 SCS method"			
"	2033 Catchment 203-3"			
"	100.000 % Impervious"			
"	0.092 Total Area"			
"	10.000 Flow length"			
"	2.000 Overland Slope"			
"	0.000 Pervious Area"			
"	10.000 Pervious length"			
"	2.000 Pervious slope"			
"	0.092 Impervious Area"			
"	10.000 Impervious length"			
"	2.000 Impervious slope"			
"	0.250 Pervious Manning 'n' "			
"	74.000 Pervious SCS Curve No. "			
"	0.000 Pervious Runoff coefficient"			
"	0.100 Pervious Ia/S coefficient"			
"	8.924 Pervious Initial abstraction"			
"	0.015 Impervious Manning 'n' "			
"	98.000 Impervious SCS Curve No. "			
"	0.794 Impervious Runoff coefficient"			
"	0.100 Impervious Ia/S coefficient"			
"	0.518 Impervious Initial abstraction"			
"	0.014	0.000	0.001	0.010 c.m/sec"
"	Catchment 2033	Pervious	Impervious	Total Area "
"	Surface Area	0.000	0.092	0.092 hectare"
"	Time of concentration	22.725	1.369	1.369 minutes"
"	Time to Centroid	168.546	119.217	119.217 minutes"
"	Rainfall depth	25.028	25.028	25.028 mm"
"	Rainfall volume	0.00	23.03	23.03 c.m"
"	Rainfall losses	22.568	5.156	5.156 mm"
"	Runoff depth	2.459	19.872	19.872 mm"
"	Runoff volume	0.00	18.28	18.28 c.m"
"	Runoff coefficient	0.000	0.794	0.794 "
"	Maximum flow	0.000	0.014	0.014 c.m/sec"

" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.014	0.014	0.001	0.010"	
" 57	TRENCH Design d/s of 2033"				
"	0.014 Peak inflow"				
"	18.282 Hydrograph volume"				
"	337.000 Ground elevation"				
"	334.500 Downstream trench invert"				
"	1.000 Trench height"				
"	333.500 Water table elevation"				
"	1.000 Trench top width"				
"	1.000 Trench bottom width"				
"	40.000 Voids ratio (%) "				
"	30.000 Hydraulic conductivity"				
"	0.000 Trench gradient (%) "				
"	60.000 Trench length"				
"	1.000 Include base width"				
"	26. Number of stages"				
"	Level Discharge	Volume"			
"	334.500	0.000	0.0"		
"	334.600	0.000	2.4"		
"	334.700	0.000	4.8"		
"	334.800	0.000	7.2"		
"	334.900	0.000	9.6"		
"	335.000	0.000	12.0"		
"	335.100	0.000	14.4"		
"	335.200	0.000	16.8"		
"	335.300	0.000	19.2"		
"	335.400	0.000	21.6"		
"	335.500	0.000	24.0"		
"	335.600	0.004	24.1"		
"	335.700	0.006	24.2"		
"	335.800	0.008	24.3"		
"	335.900	0.010	24.5"		
"	336.000	0.011	24.6"		
"	336.100	0.013	24.7"		
"	336.200	0.014	24.8"		
"	336.300	0.015	24.9"		
"	336.400	0.016	25.0"		
"	336.500	0.017	25.1"		
"	336.600	0.018	25.2"		
"	336.700	0.019	25.4"		
"	336.800	0.020	25.5"		
"	336.900	0.020	25.6"		
"	337.000	0.021	25.7"		
"	1. TRENCH PIPES"				
"	Downstream	Pipe	Pipe	Pipe Perf'ted?	Offset"
"	Invert	length	diam.	grade%	0=Yes distance"
"	335.500	20.000	0.000	1.000	1.000 0.000"
"	1. MANHOLE"				
"	Access				
"	diameter"				
"	1.200"				
"	1. OUTFLOW PIPE"				
"	0. Inflow at upstream end of trench: 1=True; 0=False"				
"	Upstream Downstr'm	Pipe	Pipe	Manning	Entry"
"	invert	invert	Length	Diameter	'n' loss Ke"
"	335.500	335.300	20.000	0.100	0.013 0.500"
"	Peak outflow		0.000	c.m/sec"	
"	Outflow volume		0.002	c.m"	
"	Peak exfiltration		0.001	c.m/sec"	
"	Exfiltration volume		18.286	c.m"	
"	Maximum level		334.975	metre"	
"	Maximum storage		11.388	c.m"	
"	Centroidal lag		4.361	hours"	

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"      Infiltration area 2 sides 56.942 sq.metre"
"      Infiltration Base area 60.000 sq.metre"
"      0.014 0.014 0.000 0.001 c.m/sec"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.010 c.m/sec"
"      Hydrograph volume 19.495 c.m"
"      0.014 0.014 0.000 0.010"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.014 0.000 0.000 0.010"
" 33 CATCHMENT 204"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      204 Catchment 204"
"      0.000 % Impervious"
"      0.198 Total Area"
"      8.000 Flow length"
"      20.000 Overland Slope"
"      0.198 Pervious Area"
"      8.000 Pervious length"
"      20.000 Pervious slope"
"      0.000 Impervious Area"
"      8.000 Impervious length"
"      20.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.002 0.000 0.000 0.010 c.m/sec"
"      Catchment 204 Pervious Impervious Total Area "
"      Surface Area 0.198 0.000 0.198 hectare"
"      Time of concentration 9.962 0.600 9.962 minutes"
"      Time to Centroid 153.643 118.857 153.643 minutes"
"      Rainfall depth 25.028 25.028 25.028 mm"
"      Rainfall volume 49.55 0.00 49.55 c.m"
"      Rainfall losses 22.580 6.253 22.580 mm"
"      Runoff depth 2.447 18.775 2.447 mm"
"      Runoff volume 4.85 0.00 4.85 c.m"
"      Runoff coefficient 0.098 0.000 0.098 "
"      Maximum flow 0.002 0.000 0.002 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.002 0.002 0.000 0.010"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.002 0.002 0.002 0.010"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.011 c.m/sec"
"      Hydrograph volume 24.341 c.m"
"      0.002 0.002 0.002 0.011"
" 40 HYDROGRAPH Confluence 700"
"      7 Confluence "

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"      700 Node #"
"      CULVERT"
"      Maximum flow 0.011 c.m/sec"
"      Hydrograph volume 24.341 c.m"
"      0.002 0.011 0.002 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.002 0.011 0.011 0.000"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow 0.015 c.m/sec"
"      Hydrograph volume 187.998 c.m"
"      0.002 0.011 0.011 0.015"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.002 0.000 0.011 0.015"
" 33 CATCHMENT 2051"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2051 Catchment 205-1"
"      25.000 % Impervious"
"      0.098 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.073 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.024 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.098 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.797 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.004 0.000 0.011 0.015 c.m/sec"
"      Catchment 2051 Pervious Impervious Total Area "
"      Surface Area 0.073 0.024 0.098 hectare"
"      Time of concentration 28.984 1.747 9.102 minutes"
"      Time to Centroid 175.889 119.783 134.933 minutes"
"      Rainfall depth 25.028 25.028 25.028 mm"
"      Rainfall volume 18.40 6.13 24.53 c.m"
"      Rainfall losses 22.568 5.082 18.197 mm"
"      Runoff depth 2.460 19.946 6.831 mm"
"      Runoff volume 1.81 4.89 6.69 c.m"
"      Runoff coefficient 0.098 0.797 0.273 "
"      Maximum flow 0.000 0.004 0.004 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.004 0.004 0.011 0.015"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.004 0.004 0.004 0.015"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"

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"	Maximum flow	0.004	c.m/sec"
"	Hydrograph volume	6.694	c.m"
"	0.004	0.004	0.004
"	0.004"		
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.004	0.000	0.004
"	0.004"		
" 33	CATCHMENT 2052"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2052 Catchment 205-2"		
"	40.000 % Impervious"		
"	0.108 Total Area"		
"	25.000 Flow length"		
"	5.000 Overland Slope"		
"	0.065 Pervious Area"		
"	25.000 Pervious length"		
"	5.000 Pervious slope"		
"	0.043 Impervious Area"		
"	25.000 Impervious length"		
"	5.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	74.000 Pervious SCS Curve No."		
"	0.098 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.798 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.006	0.000	0.004
"	0.004 c.m/sec"		
"	Catchment 2052	Pervious	Impervious
"	Surface Area	0.065	0.043
"	Time of concentration	29.915	1.803
"	Time to Centroid	176.981	119.890
"	Rainfall depth	25.028	25.028
"	Rainfall volume	16.22	10.81
"	Rainfall losses	22.567	5.063
"	Runoff depth	2.461	19.965
"	Runoff volume	1.59	8.62
"	Runoff coefficient	0.098	0.798
"	Maximum flow	0.000	0.006
"	0.006		
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.006	0.006	0.004
"	0.004"		
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.006	0.006	0.006
"	0.004"		
" 40	HYDROGRAPH Combine 600"		
"	6 Combine "		
"	600 Node #"		
"	Arkell Rd"		
"	Maximum flow	0.010	c.m/sec"
"	Hydrograph volume	16.914	c.m"
"	0.006	0.006	0.006
"	0.010"		
" 40	HYDROGRAPH Confluence 600"		
"	7 Confluence "		
"	600 Node #"		
"	Arkell Rd"		
"	Maximum flow	0.010	c.m/sec"
"	Hydrograph volume	16.914	c.m"
"	0.006	0.010	0.006
"	0.000"		
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		

"	0.006	0.010	0.010	0.000"
" 40	HYDROGRAPH Combine 800"			
"	6 Combine "			
"	800 Node #"			
"	TORRANCE CREEK"			
"	Maximum flow	0.023	c.m/sec"	
"	Hydrograph volume	204.912	c.m"	
"	0.006	0.010	0.010	
"	0.023"			
" 40	HYDROGRAPH Start - New Tributary"			
"	2 Start - New Tributary"			
"	0.006	0.000	0.010	
"	0.023"			
" 33	CATCHMENT 206"			
"	1 Triangular SCS"			
"	1 Equal length"			
"	1 SCS method"			
"	206 Catchment 206"			
"	70.000 % Impervious"			
"	0.035 Total Area"			
"	20.000 Flow length"			
"	1.250 Overland Slope"			
"	0.011 Pervious Area"			
"	20.000 Pervious length"			
"	1.250 Pervious slope"			
"	0.024 Impervious Area"			
"	20.000 Impervious length"			
"	1.250 Impervious slope"			
"	0.250 Pervious Manning 'n'"			
"	74.000 Pervious SCS Curve No."			
"	0.098 Pervious Runoff coefficient"			
"	0.100 Pervious Ia/S coefficient"			
"	8.924 Pervious Initial abstraction"			
"	0.015 Impervious Manning 'n'"			
"	98.000 Impervious SCS Curve No."			
"	0.795 Impervious Runoff coefficient"			
"	0.100 Impervious Ia/S coefficient"			
"	0.518 Impervious Initial abstraction"			
"	0.004	0.000	0.010	
"	0.023 c.m/sec"			
"	Catchment 206	Pervious	Impervious	
"	Surface Area	0.011	0.024	
"	Time of concentration	39.661	2.390	
"	Time to Centroid	188.432	120.907	
"	Rainfall depth	25.028	25.028	
"	Rainfall volume	2.63	6.13	
"	Rainfall losses	22.567	5.124	
"	Runoff depth	2.461	19.904	
"	Runoff volume	0.26	4.88	
"	Runoff coefficient	0.098	0.795	
"	Maximum flow	0.000	0.004	
"	0.004			
" 40	HYDROGRAPH Add Runoff "			
"	4 Add Runoff "			
"	0.004	0.004	0.010	
"	0.023"			
" 40	HYDROGRAPH Copy to Outflow"			
"	8 Copy to Outflow"			
"	0.004	0.004	0.004	
"	0.023"			
" 40	HYDROGRAPH Combine 800"			
"	6 Combine "			
"	800 Node #"			
"	TORRANCE CREEK"			
"	Maximum flow	0.027	c.m/sec"	
"	Hydrograph volume	210.047	c.m"	
"	0.004	0.004	0.004	
"	0.027"			
" 40	HYDROGRAPH Confluence 800"			
"	7 Confluence "			
"	800 Node #"			
"	TORRANCE CREEK"			

```

"      Maximum flow          0.027   c.m/sec"
"      Hydrograph volume    210.047 c.m"
"      0.004      0.027    0.004   0.000"
" 38  START/RE-START TOTALS 800"
"      3  Runoff Totals on EXIT"
"      Total Catchment area      3.108  hectare"
"      Total Impervious area     1.071  hectare"
"      Total % impervious      34.445"
" 19  EXIT"

```

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:             Q:\42063\104\SWM\September 2021\MIDUSS\
"                                POST"
"      Output filename:        2yrPOST.in"
"      Licensee name:          A"
"      Company                  Microsoft"
"      Date & Time last used:   10/5/2021 at 1:38:32 PM"
" 31  TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32  STORM Chicago storm"
"      1 Chicago storm"
"      743.000 Coefficient A"
"      6.000 Constant B"
"      0.799 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity      109.374 mm/hr"
"      Total depth            34.259 mm"
"      6 002hyd Hydrograph extension used in this file"
" 33  CATCHMENT 2011"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2011 Catchment 201-1"
"      60.000 % Impervious"
"      0.519 Total Area"
"      60.000 Flow length"
"      0.750 Overland Slope"
"      0.208 Pervious Area"
"      60.000 Pervious length"
"      0.750 Pervious slope"
"      0.311 Impervious Area"
"      60.000 Impervious length"
"      0.750 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.163 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.845 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.065      0.000      0.000      0.000 c.m/sec"
"      Catchment 2011 Pervious Impervious Total Area "
"      Surface Area      0.208      0.311      0.519 hectare"
"      Time of concentration 56.084      4.495      10.387 minutes"
"      Time to Centroid      170.993      94.804      103.505 minutes"
"      Rainfall depth      34.259      34.259      34.259 mm"
"      Rainfall volume      71.12      106.68      177.80 c.m"
"      Rainfall losses      28.658      5.300      14.643 mm"
"      Runoff depth      5.600      28.959      19.615 mm"
"      Runoff volume      11.63      90.18      101.80 c.m"
"      Runoff coefficient      0.163      0.845      0.573 "
"      Maximum flow      0.002      0.065      0.065 c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.065      0.065      0.000      0.000"

```

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.065 0.065 0.065 0.000"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.065 c.m/sec"

" Hydrograph volume 101.803 c.m"

" 0.065 0.065 0.065 0.065"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.065 0.000 0.065 0.065"

" 33 CATCHMENT 2012"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2012 Catchment 201-2"

" 70.000 % Impervious"

" 0.066 Total Area"

" 15.000 Flow length"

" 0.500 Overland Slope"

" 0.020 Pervious Area"

" 15.000 Pervious length"

" 0.500 Pervious slope"

" 0.046 Impervious Area"

" 15.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.163 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.841 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.010 0.000 0.065 0.065 c.m/sec"

" Catchment 2012 Pervious Impervious Total Area "

" Surface Area 0.020 0.046 0.066 hectare"

" Time of concentration 27.570 2.210 4.159 minutes"

" Time to Centroid 136.872 91.264 94.770 minutes"

" Rainfall depth 34.259 34.259 34.259 mm"

" Rainfall volume 6.78 15.83 22.61 c.m"

" Rainfall losses 28.660 5.447 12.411 mm"

" Runoff depth 5.599 28.811 21.848 mm"

" Runoff volume 1.11 13.31 14.42 c.m"

" Runoff coefficient 0.163 0.841 0.638 "

" Maximum flow 0.000 0.010 0.010 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.010 0.010 0.065 0.065"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.010 0.010 0.010 0.065"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.073 c.m/sec"

" Hydrograph volume 116.223 c.m"

" 0.010 0.010 0.010 0.073"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.010 0.000 0.010 0.073"

" 33 CATCHMENT 2013"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2013 Catchment 201-3"

" 75.000 % Impervious"

" 0.234 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.058 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.176 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.163 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.840 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.041 0.000 0.010 0.073 c.m/sec"

" Catchment 2013 Pervious Impervious Total Area "

" Surface Area 0.058 0.176 0.234 hectare"

" Time of concentration 18.189 1.458 2.476 minutes"

" Time to Centroid 125.657 90.077 92.242 minutes"

" Rainfall depth 34.259 34.259 34.259 mm"

" Rainfall volume 20.04 60.12 80.17 c.m"

" Rainfall losses 28.664 5.483 11.278 mm"

" Runoff depth 5.594 28.775 22.980 mm"

" Runoff volume 3.27 50.50 53.77 c.m"

" Runoff coefficient 0.163 0.840 0.671 "

" Maximum flow 0.001 0.041 0.041 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.041 0.041 0.010 0.073"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.041 0.041 0.041 0.073"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.102 c.m/sec"

" Hydrograph volume 169.997 c.m"

" 0.041 0.041 0.041 0.102"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.041 0.000 0.041 0.102"

" 33 CATCHMENT 2014"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2014 Catchment 201-4"

" 100.000 % Impervious"

" 0.065 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.000 Pervious Area"

" 10.000 Pervious length"

```

"      2.000 Pervious slope"
"      0.065 Impervious Area"
"     10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     74.000 Pervious SCS Curve No."
"      0.000 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.831 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.015      0.000      0.041      0.102 c.m/sec"
"      Catchment 2014      Pervious      Impervious      Total Area
"      Surface Area      0.000      0.065      0.065      hectare"
"      Time of concentration      14.261      1.143      1.143      minutes"
"      Time to Centroid      120.965      89.575      89.575      minutes"
"      Rainfall depth      34.259      34.259      34.259      mm"
"      Rainfall volume      0.00      22.27      22.27      c.m"
"      Rainfall losses      28.664      5.776      5.776      mm"
"      Runoff depth      5.594      28.482      28.482      mm"
"      Runoff volume      0.00      18.51      18.51      c.m"
"      Runoff coefficient      0.000      0.831      0.831
"      Maximum flow      0.000      0.015      0.015      c.m/sec"
" 40      HYDROGRAPH Add Runoff
"      4      Add Runoff
"      0.015      0.015      0.041      0.102"
" 57      TRENCH Design d/s of 2014"
"      0.015 Peak inflow"
"     18.513 Hydrograph volume"
"     337.000 Ground elevation"
"     334.200 Downstream trench invert"
"      1.000 Trench height"
"     333.200 Water table elevation"
"      5.000 Trench top width"
"      5.000 Trench bottom width"
"     40.000 Voids ratio (%)"
"     30.000 Hydraulic conductivity"
"      0.000 Trench gradient (%)"
"     10.000 Trench length"
"      1.000 Include base width"
"      29.      Number of stages"
"      Level Discharge      Volume"
"     334.200      0.000      0.0"
"     334.300      0.000      2.0"
"     334.400      0.000      4.0"
"     334.500      0.000      6.0"
"     334.600      0.000      8.0"
"     334.700      0.000     10.0"
"     334.800      0.000     12.0"
"     334.900      0.000     14.0"
"     335.000      0.000     16.0"
"     335.100      0.000     18.0"
"     335.200      0.000     20.0"
"     335.300      0.004     20.1"
"     335.400      0.006     20.2"
"     335.500      0.008     20.3"
"     335.600      0.010     20.5"
"     335.700      0.011     20.6"
"     335.800      0.013     20.7"
"     335.900      0.014     20.8"
"     336.000      0.015     20.9"
"     336.100      0.016     21.0"

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"      336.200      0.017      21.1"
"      336.300      0.018      21.2"
"      336.400      0.019      21.4"
"      336.500      0.020      21.5"
"      336.600      0.020      21.6"
"      336.700      0.021      21.7"
"      336.800      0.022      21.8"
"      336.900      0.023      21.9"
"      337.000      0.023      22.0"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe      Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      20.000      0.000      1.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      20.000      0.100      0.013      0.500"
"      Peak outflow      0.000      c.m/sec"
"      Outflow volume      0.003      c.m"
"      Peak exfiltration      0.001      c.m/sec"
"      Exfiltration volume      18.511      c.m"
"      Maximum level      334.890      metre"
"      Maximum storage      13.804      c.m"
"      Centroidal lag      5.524      hours"
"      Infiltration area 2 sides      13.804      sq.metre"
"      Infiltration Base area      50.000      sq.metre"
"      0.015      0.015      0.000      0.001 c.m/sec"
" 40      HYDROGRAPH Combine      900"
"      6      Combine
"      900      Node #
"      SWMF"
"      Maximum flow      0.102      c.m/sec"
"      Hydrograph volume      170.000      c.m"
"      0.015      0.015      0.000      0.102"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.015      0.000      0.000      0.102"
" 33      CATCHMENT 2015"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      2015      Catchment 201-5"
"     85.000      % Impervious"
"      0.074      Total Area"
"     15.000      Flow length"
"      2.000      Overland Slope"
"      0.011      Pervious Area"
"     15.000      Pervious length"
"      2.000      Pervious slope"
"      0.063      Impervious Area"
"     15.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"     74.000      Pervious SCS Curve No."
"      0.163      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"     98.000      Impervious SCS Curve No."
"      0.840      Impervious Runoff coefficient"

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```

"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.015      0.000      0.000      0.102 c.m/sec"
"      Catchment 2015      Pervious      Impervious Total Area      "
"      Surface Area      0.011      0.063      0.074      hectare"
"      Time of concentration      18.189      1.458      2.013      minutes"
"      Time to Centroid      125.657      90.077      91.257      minutes"
"      Rainfall depth      34.259      34.259      34.259      mm"
"      Rainfall volume      3.80      21.55      25.35      c.m"
"      Rainfall losses      28.664      5.483      8.960      mm"
"      Runoff depth      5.594      28.775      25.298      mm"
"      Runoff volume      0.62      18.10      18.72      c.m"
"      Runoff coefficient      0.163      0.840      0.738      "
"      Maximum flow      0.000      0.015      0.015      c.m/sec"
" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.015      0.015      0.000      0.102"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.015      0.015      0.015      0.102"
" 40      HYDROGRAPH Combine      900"
"      6      Combine      "
"      900      Node #      "
"      SWMF"
"      Maximum flow      0.117      c.m/sec"
"      Hydrograph volume      188.721      c.m"
"      0.015      0.015      0.015      0.117"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.015      0.000      0.015      0.117"
" 33      CATCHMENT 2016"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      2016      Catchment 201-6"
"      100.000      % Impervious"
"      0.032      Total Area"
"      10.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      10.000      Pervious length"
"      2.000      Pervious slope"
"      0.032      Impervious Area"
"      10.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.000      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.831      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.008      0.000      0.015      0.117 c.m/sec"
"      Catchment 2016      Pervious      Impervious Total Area      "
"      Surface Area      0.000      0.032      0.032      hectare"
"      Time of concentration      14.261      1.143      1.143      minutes"
"      Time to Centroid      120.965      89.575      89.575      minutes"
"      Rainfall depth      34.259      34.259      34.259      mm"
"      Rainfall volume      0.00      10.96      10.96      c.m"
"      Rainfall losses      28.664      5.776      5.776      mm"
"      Runoff depth      5.594      28.482      28.482      mm"
"      Runoff volume      0.00      9.11      9.11      c.m"

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```

"      Runoff coefficient      0.000      0.831      0.831      "
"      Maximum flow      0.000      0.008      0.008      c.m/sec"
" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.008      0.008      0.015      0.117"
" 57      TRENCH Design d/s of 2016"
"      0.008      Peak inflow"
"      9.114      Hydrograph volume"
"      337.000      Ground elevation"
"      334.200      Downstream trench invert"
"      1.000      Trench height"
"      333.200      Water table elevation"
"      5.000      Trench top width"
"      5.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      5.000      Trench length"
"      1.000      Include base width"
"      29.      Number of stages"
"      Level Discharge      Volume"
"      334.200      0.000      0.0"
"      334.300      0.000      1.0"
"      334.400      0.000      2.0"
"      334.500      0.000      3.0"
"      334.600      0.000      4.0"
"      334.700      0.000      5.0"
"      334.800      0.000      6.0"
"      334.900      0.000      7.0"
"      335.000      0.000      8.0"
"      335.100      0.000      9.0"
"      335.200      0.000      10.0"
"      335.300      0.002      10.1"
"      335.400      0.006      10.2"
"      335.500      0.008      10.3"
"      335.600      0.010      10.5"
"      335.700      0.011      10.6"
"      335.800      0.013      10.7"
"      335.900      0.014      10.8"
"      336.000      0.015      10.9"
"      336.100      0.016      11.0"
"      336.200      0.017      11.1"
"      336.300      0.018      11.2"
"      336.400      0.019      11.4"
"      336.500      0.020      11.5"
"      336.600      0.020      11.6"
"      336.700      0.021      11.7"
"      336.800      0.022      11.8"
"      336.900      0.023      11.9"
"      337.000      0.023      12.0"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      10.000      0.000      2.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      10.000      0.100      0.013      0.500"
"      Peak outflow      0.000      c.m/sec"
"      Outflow volume      0.003      c.m"

```

"	Peak exfiltration	0.000	c.m/sec"
"	Exfiltration volume	9.112	c.m"
"	Maximum level	334.878	metre"
"	Maximum storage	6.784	c.m"
"	Centroidal lag	5.477	hours"
"	Infiltration area 2 sides	6.783	sq.metre"
"	Infiltration Base area	25.000	sq.metre"
"	0.008 0.008 0.000 0.000	c.m/sec"	
" 40	HYDROGRAPH Combine	900"	
"	6 Combine "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.117	c.m/sec"
"	Hydrograph volume	188.724	c.m"
"	0.008 0.008 0.000 0.117"		
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.008 0.000 0.000 0.117"		
" 33	CATCHMENT 2017"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2017 Catchment 201-7 - SWMF"		
"	40.000 % Impervious"		
"	0.211 Total Area"		
"	10.000 Flow length"		
"	10.000 Overland Slope"		
"	0.127 Pervious Area"		
"	10.000 Pervious length"		
"	10.000 Pervious slope"		
"	0.084 Impervious Area"		
"	10.000 Impervious length"		
"	10.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	74.000 Pervious SCS Curve No."		
"	0.163 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.804 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.021 0.000 0.000 0.117	c.m/sec"	
"	Catchment 2017 Pervious Impervious Total Area "		
"	Surface Area 0.127 0.084 0.211	hectare"	
"	Time of concentration 8.800 0.705 2.591	minutes"	
"	Time to Centroid 114.422 89.351 95.193	minutes"	
"	Rainfall depth 34.259 34.259 34.259	mm"	
"	Rainfall volume 43.37 28.91 72.29	c.m"	
"	Rainfall losses 28.680 6.715 19.894	mm"	
"	Runoff depth 5.579 27.543 14.364	mm"	
"	Runoff volume 7.06 23.25 30.31	c.m"	
"	Runoff coefficient 0.163 0.804 0.419	"	
"	Maximum flow 0.003 0.020 0.021	c.m/sec"	
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.021 0.021 0.000 0.117"		
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.021 0.021 0.021 0.117"		
" 40	HYDROGRAPH Combine	900"	
"	6 Combine "		
"	900 Node #"		
"	SWMF"		

"	Maximum flow	0.138	c.m/sec"
"	Hydrograph volume	219.033	c.m"
"	0.021 0.021 0.021 0.138"		
" 40	HYDROGRAPH Confluence	900"	
"	7 Confluence "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.138	c.m/sec"
"	Hydrograph volume	219.033	c.m"
"	0.021 0.138 0.021 0.000"		
" 54	POND DESIGN"		
"	0.138 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	219.0 Hydrograph volume	c.m"	
"	13. Number of stages"		
"	334.400 Minimum water level	metre"	
"	335.600 Maximum water level	metre"	
"	334.400 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge Volume"		
"	334.400 0.000 0.000"		
"	334.500 0.00150 40.000"		
"	334.600 0.00230 85.000"		
"	334.700 0.00290 135.000"		
"	334.800 0.03110 189.000"		
"	334.900 0.04300 249.000"		
"	335.000 0.05220 314.000"		
"	335.100 0.05990 384.000"		
"	335.200 0.06680 457.000"		
"	335.300 0.07300 534.000"		
"	335.400 0.1628 614.000"		
"	335.500 0.4275 698.000"		
"	335.600 0.9140 786.000"		
"	Peak outflow	0.013	c.m/sec"
"	Maximum level	334.737	metre"
"	Maximum storage	154.946	c.m"
"	Centroidal lag	9.226	hours"
"	0.021 0.138 0.013 0.000	c.m/sec"	
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.021 0.013 0.013 0.000"		
" 54	POND DESIGN"		
"	0.013 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	207.1 Hydrograph volume	c.m"	
"	10. Number of stages"		
"	334.200 Minimum water level	metre"	
"	335.100 Maximum water level	metre"	
"	334.200 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge Volume"		
"	334.200 0.000 0.000"		
"	334.300 0.00191 18.000"		
"	334.400 0.00207 38.000"		
"	334.500 0.00225 60.000"		
"	334.600 0.00243 83.000"		
"	334.700 0.00261 109.000"		
"	334.800 0.00281 136.000"		
"	334.900 0.1543 165.000"		
"	335.000 0.4628 196.000"		
"	335.100 0.9060 229.000"		
"	Peak outflow	0.002	c.m/sec"
"	Maximum level	334.547	metre"
"	Maximum storage	70.889	c.m"
"	Centroidal lag	14.554	hours"

" 0.021 0.013 0.002 0.000 c.m/sec"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.002 c.m/sec"

" Hydrograph volume 186.196 c.m"

" 0.021 0.013 0.002 0.002"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.021 0.000 0.002 0.002"

" 33 CATCHMENT 2021"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2021 Catchment 202-1"

" 0.000 % Impervious"

" 0.863 Total Area"

" 50.000 Flow length"

" 0.500 Overland Slope"

" 0.863 Pervious Area"

" 50.000 Pervious length"

" 0.500 Pervious slope"

" 0.000 Impervious Area"

" 50.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.163 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.000 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.007 0.000 0.002 0.002 c.m/sec"

" Catchment 2021 Pervious Impervious Total Area "

" Surface Area 0.863 0.000 0.863 hectare"

" Time of concentration 56.775 4.550 56.775 minutes"

" Time to Centroid 171.819 94.883 171.819 minutes"

" Rainfall depth 34.259 34.259 34.259 mm"

" Rainfall volume 295.65 0.00 295.65 c.m"

" Rainfall losses 28.658 5.281 28.658 mm"

" Runoff depth 5.600 28.978 5.600 mm"

" Runoff volume 48.33 0.00 48.33 c.m"

" Runoff coefficient 0.163 0.000 0.163 "

" Maximum flow 0.007 0.000 0.007 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.007 0.007 0.002 0.002"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.007 0.007 0.007 0.002"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.009 c.m/sec"

" Hydrograph volume 234.525 c.m"

" 0.007 0.007 0.007 0.009"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.007 0.000 0.007 0.009"

" 33 CATCHMENT 2022"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2022 Catchment 202-2"

" 24.000 % Impervious"

" 0.184 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.140 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.044 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.163 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.831 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.011 0.000 0.007 0.009 c.m/sec"

" Catchment 2022 Pervious Impervious Total Area "

" Surface Area 0.140 0.044 0.184 hectare"

" Time of concentration 14.261 1.143 6.174 minutes"

" Time to Centroid 120.965 89.575 101.612 minutes"

" Rainfall depth 34.259 34.259 34.259 mm"

" Rainfall volume 47.91 15.13 63.04 c.m"

" Rainfall losses 28.664 5.776 23.171 mm"

" Runoff depth 5.594 28.482 11.088 mm"

" Runoff volume 7.82 12.58 20.40 c.m"

" Runoff coefficient 0.163 0.831 0.324 "

" Maximum flow 0.003 0.010 0.011 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.011 0.011 0.007 0.009"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.011 0.011 0.011 0.009"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.011 c.m/sec"

" Hydrograph volume 254.926 c.m"

" 0.011 0.011 0.011 0.011"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.011 0.000 0.011 0.011"

" 33 CATCHMENT 2031"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2031 Catchment 203-1"

" 30.000 % Impervious"

" 0.216 Total Area"

" 130.000 Flow length"

" 2.000 Overland Slope"

" 0.151 Pervious Area"

" 130.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"



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"      130.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.163 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.851 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.014 0.000 0.011 0.011 c.m/sec"
"      Catchment 2031 Pervious Impervious Total Area "
"      Surface Area 0.151 0.065 0.216 hectare"
"      Time of concentration 66.455 5.326 24.246 minutes"
"      Time to Centroid 183.401 96.017 123.063 minutes"
"      Rainfall depth 34.259 34.259 34.259 mm"
"      Rainfall volume 51.80 22.20 74.00 c.m"
"      Rainfall losses 28.658 5.104 21.592 mm"
"      Runoff depth 5.601 29.155 12.667 mm"
"      Runoff volume 8.47 18.89 27.36 c.m"
"      Runoff coefficient 0.163 0.851 0.370 "
"      Maximum flow 0.001 0.014 0.014 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
"      0.014 0.014 0.011 0.011"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
"      0.014 0.014 0.014 0.011"
" 40 HYDROGRAPH Combine 700"
" 6 Combine "
" 700 Node #"
" CULVERT"
" Maximum flow 0.014 c.m/sec"
" Hydrograph volume 27.361 c.m"
"      0.014 0.014 0.014 0.014"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
"      0.014 0.000 0.014 0.014"
" 33 CATCHMENT 2032"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 2032 Catchment 203-2"
" 0.000 % Impervious"
" 0.113 Total Area"
" 10.000 Flow length"
" 33.000 Overland Slope"
" 0.113 Pervious Area"
" 10.000 Pervious length"
" 33.000 Pervious slope"
" 0.000 Impervious Area"
" 10.000 Impervious length"
" 33.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.162 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"

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"      0.003 0.000 0.014 0.014 c.m/sec"
"      Catchment 2032 Pervious Impervious Total Area "
"      Surface Area 0.113 0.000 0.113 hectare"
"      Time of concentration 6.151 0.493 6.151 minutes"
"      Time to Centroid 111.394 89.327 111.394 minutes"
"      Rainfall depth 34.259 34.259 34.259 mm"
"      Rainfall volume 38.71 0.00 38.71 c.m"
"      Rainfall losses 28.713 7.785 28.713 mm"
"      Runoff depth 5.545 26.473 5.545 mm"
"      Runoff volume 6.27 0.00 6.27 c.m"
"      Runoff coefficient 0.162 0.000 0.162 "
"      Maximum flow 0.003 0.000 0.003 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
"      0.003 0.003 0.014 0.014"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
"      0.003 0.003 0.003 0.014"
" 40 HYDROGRAPH Combine 700"
" 6 Combine "
" 700 Node #"
" CULVERT"
" Maximum flow 0.016 c.m/sec"
" Hydrograph volume 33.627 c.m"
"      0.003 0.003 0.003 0.016"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
"      0.003 0.000 0.003 0.016"
" 33 CATCHMENT 2033"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 2033 Catchment 203-3"
" 100.000 % Impervious"
" 0.092 Total Area"
" 10.000 Flow length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 10.000 Pervious length"
" 2.000 Pervious slope"
" 0.092 Impervious Area"
" 10.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.000 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.831 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
"      0.022 0.000 0.003 0.016 c.m/sec"
"      Catchment 2033 Pervious Impervious Total Area "
"      Surface Area 0.000 0.092 0.092 hectare"
"      Time of concentration 14.261 1.143 1.143 minutes"
"      Time to Centroid 120.965 89.575 89.575 minutes"
"      Rainfall depth 34.259 34.259 34.259 mm"
"      Rainfall volume 0.00 31.52 31.52 c.m"
"      Rainfall losses 28.664 5.776 5.776 mm"
"      Runoff depth 5.594 28.482 28.482 mm"
"      Runoff volume 0.00 26.20 26.20 c.m"
"      Runoff coefficient 0.000 0.831 0.831 "
"      Maximum flow 0.000 0.022 0.022 c.m/sec"

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" 40      HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.022      0.022      0.003      0.016"
" 57      TRENCH Design d/s of 2033"
"      0.022      Peak inflow"
"      26.204      Hydrograph volume"
"      337.000      Ground elevation"
"      334.500      Downstream trench invert"
"      1.000      Trench height"
"      333.500      Water table elevation"
"      1.000      Trench top width"
"      1.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      60.000      Trench length"
"      1.000      Include base width"
"      26.      Number of stages"
"      Level Discharge      Volume"
"      334.500      0.000      0.0"
"      334.600      0.000      2.4"
"      334.700      0.000      4.8"
"      334.800      0.000      7.2"
"      334.900      0.000      9.6"
"      335.000      0.000      12.0"
"      335.100      0.000      14.4"
"      335.200      0.000      16.8"
"      335.300      0.000      19.2"
"      335.400      0.000      21.6"
"      335.500      0.000      24.0"
"      335.600      0.004      24.1"
"      335.700      0.006      24.2"
"      335.800      0.008      24.3"
"      335.900      0.010      24.5"
"      336.000      0.011      24.6"
"      336.100      0.013      24.7"
"      336.200      0.014      24.8"
"      336.300      0.015      24.9"
"      336.400      0.016      25.0"
"      336.500      0.017      25.1"
"      336.600      0.018      25.2"
"      336.700      0.019      25.4"
"      336.800      0.020      25.5"
"      336.900      0.020      25.6"
"      337.000      0.021      25.7"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.500      20.000      0.000      1.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.500      335.300      20.000      0.100      0.013      0.500"
"      Peak outflow      0.000      c.m/sec"
"      Outflow volume      0.002      c.m"
"      Peak exfiltration      0.002      c.m/sec"
"      Exfiltration volume      26.197      c.m"
"      Maximum level      335.237      metre"
"      Maximum storage      17.682      c.m"
"      Centroidal lag      4.429      hours"

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"      Infiltration area 2 sides      88.407      sq.metre"
"      Infiltration Base area      60.000      sq.metre"
"      0.022      0.022      0.000      0.002 c.m/sec"
" 40      HYDROGRAPH Combine      700"
"      6      Combine "
"      700      Node #"
"      CULVERT"
"      Maximum flow      0.016      c.m/sec"
"      Hydrograph volume      33.629      c.m"
"      0.022      0.022      0.000      0.016"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.022      0.000      0.000      0.016"
" 33      CATCHMENT 204"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      204      Catchment 204"
"      0.000      % Impervious"
"      0.198      Total Area"
"      8.000      Flow length"
"      20.000      Overland Slope"
"      0.198      Pervious Area"
"      8.000      Pervious length"
"      20.000      Pervious slope"
"      0.000      Impervious Area"
"      8.000      Impervious length"
"      20.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.162      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.000      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.005      0.000      0.000      0.016 c.m/sec"
"      Catchment 204      Pervious      Impervious      Total Area "
"      Surface Area      0.198      0.000      0.198      hectare"
"      Time of concentration      6.252      0.501      6.252      minutes"
"      Time to Centroid      111.541      89.345      111.541      minutes"
"      Rainfall depth      34.259      34.259      34.259      mm"
"      Rainfall volume      67.83      0.00      67.83      c.m"
"      Rainfall losses      28.719      7.755      28.719      mm"
"      Runoff depth      5.540      26.504      5.540      mm"
"      Runoff volume      10.97      0.00      10.97      c.m"
"      Runoff coefficient      0.162      0.000      0.162      "
"      Maximum flow      0.005      0.000      0.005      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.005      0.005      0.000      0.016"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.005      0.005      0.005      0.016"
" 40      HYDROGRAPH Combine      700"
"      6      Combine "
"      700      Node #"
"      CULVERT"
"      Maximum flow      0.021      c.m/sec"
"      Hydrograph volume      44.598      c.m"
"      0.005      0.005      0.005      0.021"
" 40      HYDROGRAPH Confluence      700"
"      7      Confluence "

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"      700 Node #"
"      CULVERT"
"      Maximum flow      0.021 c.m/sec"
"      Hydrograph volume 44.598 c.m"
"      0.005      0.021      0.005      0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.005      0.021      0.021      0.000"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.030 c.m/sec"
"      Hydrograph volume 299.525 c.m"
"      0.005      0.021      0.021      0.030"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.005      0.000      0.021      0.030"
" 33 CATCHMENT 2051"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2051 Catchment 205-1"
"      25.000 % Impervious"
"      0.098 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.073 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.024 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.163 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.840 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.006      0.000      0.021      0.030 c.m/sec"
"      Catchment 2051 Pervious Impervious Total Area "
"      Surface Area 0.073 0.024 0.098 hectare"
"      Time of concentration 18.189 1.458 7.621 minutes"
"      Time to Centroid 125.657 90.077 103.184 minutes"
"      Rainfall depth 34.259 34.259 34.259 mm"
"      Rainfall volume 25.18 8.39 33.57 c.m"
"      Rainfall losses 28.664 5.483 22.869 mm"
"      Runoff depth 5.594 28.775 11.390 mm"
"      Runoff volume 4.11 7.05 11.16 c.m"
"      Runoff coefficient 0.163 0.840 0.332 "
"      Maximum flow 0.001 0.006 0.006 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.006      0.006      0.021      0.030"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.006      0.006      0.006      0.030"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.016 c.m/sec"
"      Hydrograph volume 27.227 c.m"
"      0.010      0.010      0.010      0.016"
" 40 HYDROGRAPH Confluence 600"
"      7 Confluence "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.016 c.m/sec"
"      Hydrograph volume 27.227 c.m"
"      0.010      0.016      0.010      0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"

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"      Maximum flow      0.006 c.m/sec"
"      Hydrograph volume 11.162 c.m"
"      0.006      0.006      0.006      0.006"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.006      0.000      0.006      0.006"
" 33 CATCHMENT 2052"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2052 Catchment 205-2"
"      40.000 % Impervious"
"      0.108 Total Area"
"      25.000 Flow length"
"      5.000 Overland Slope"
"      0.065 Pervious Area"
"      25.000 Pervious length"
"      5.000 Pervious slope"
"      0.043 Impervious Area"
"      25.000 Impervious length"
"      5.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.163 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.840 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.010      0.000      0.006      0.006 c.m/sec"
"      Catchment 2052 Pervious Impervious Total Area "
"      Surface Area 0.065 0.043 0.108 hectare"
"      Time of concentration 18.773 1.505 5.402 minutes"
"      Time to Centroid 126.363 90.128 98.306 minutes"
"      Rainfall depth 34.259 34.259 34.259 mm"
"      Rainfall volume 22.20 14.80 37.00 c.m"
"      Rainfall losses 28.663 5.465 19.384 mm"
"      Runoff depth 5.596 28.794 14.875 mm"
"      Runoff volume 3.63 12.44 16.06 c.m"
"      Runoff coefficient 0.163 0.840 0.434 "
"      Maximum flow 0.001 0.010 0.010 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.010      0.010      0.006      0.006"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.010      0.010      0.010      0.006"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.016 c.m/sec"
"      Hydrograph volume 27.227 c.m"
"      0.010      0.010      0.010      0.016"
" 40 HYDROGRAPH Confluence 600"
"      7 Confluence "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.016 c.m/sec"
"      Hydrograph volume 27.227 c.m"
"      0.010      0.016      0.010      0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"

```

"		0.010	0.016	0.016	0.000"
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	TORRANCE CREEK"				
"	Maximum flow		0.041	c.m/sec"	
"	Hydrograph volume		326.751	c.m"	
"		0.010	0.016	0.016	0.041"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		0.010	0.000	0.016	0.041"
" 33	CATCHMENT 206"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	206 Catchment 206"				
"	70.000 % Impervious"				
"	0.035 Total Area"				
"	20.000 Flow length"				
"	1.250 Overland Slope"				
"	0.011 Pervious Area"				
"	20.000 Pervious length"				
"	1.250 Pervious slope"				
"	0.024 Impervious Area"				
"	20.000 Impervious length"				
"	1.250 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.163 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.842 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.005	0.000	0.016	0.041 c.m/sec"
"	Catchment 206	Pervious	Impervious	Total Area	"
"	Surface Area	0.011	0.024	0.035	hectare"
"	Time of concentration	24.889	1.995	3.753	minutes"
"	Time to Centroid	133.685	90.906	94.190	minutes"
"	Rainfall depth	34.259	34.259	34.259	mm"
"	Rainfall volume	3.60	8.39	11.99	c.m"
"	Rainfall losses	28.660	5.410	12.385	mm"
"	Runoff depth	5.598	28.848	21.873	mm"
"	Runoff volume	0.59	7.07	7.66	c.m"
"	Runoff coefficient	0.163	0.842	0.638	"
"	Maximum flow	0.000	0.005	0.005	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.005	0.005	0.016	0.041"
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"		0.005	0.005	0.005	0.041"
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	TORRANCE CREEK"				
"	Maximum flow		0.046	c.m/sec"	
"	Hydrograph volume		334.407	c.m"	
"		0.005	0.005	0.005	0.046"
" 40	HYDROGRAPH Confluence	800"			
"	7 Confluence "				
"	800 Node #"				
"	TORRANCE CREEK"				

"	Maximum flow	0.046	c.m/sec"
"	Hydrograph volume	334.407	c.m"
"		0.005	0.046
"		0.005	0.000"
" 38	START/RE-START TOTALS 800"		
"	3 Runoff Totals on EXIT"		
"	Total Catchment area	3.108	hectare"
"	Total Impervious area	1.071	hectare"
"	Total % impervious	34.445"	
" 19	EXIT"		

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:              Q:\42063\104\SWM\September 2021\MIDUSS\
"                                POST"
"      Output filename:         5yrPOST.in"
"      Licensee name:           A"
"      Company                  Microsoft"
"      Date & Time last used:    10/5/2021 at 1:41:56 PM"
" 31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      1593.000 Coefficient A"
"      11.000 Constant B"
"      0.879 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity        139.250 mm/hr"
"      Total depth              47.240 mm"
"      6 005hyd Hydrograph extension used in this file"
" 33 CATCHMENT 2011"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2011 Catchment 201-1"
"      60.000 % Impervious"
"      0.519 Total Area"
"      60.000 Flow length"
"      0.750 Overland Slope"
"      0.208 Pervious Area"
"      60.000 Pervious length"
"      0.750 Pervious slope"
"      0.311 Impervious Area"
"      60.000 Impervious length"
"      0.750 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.244 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.878 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.090 0.000 0.000 0.000 c.m/sec"
"      Catchment 2011 Pervious Impervious Total Area "
"      Surface Area 0.208 0.311 0.519 hectare"
"      Time of concentration 40.157 4.029 9.665 minutes"
"      Time to Centroid 147.725 91.876 100.588 minutes"
"      Rainfall depth 47.240 47.240 47.240 mm"
"      Rainfall volume 98.07 147.10 245.17 c.m"
"      Rainfall losses 35.735 5.740 17.738 mm"
"      Runoff depth 11.505 41.499 29.502 mm"
"      Runoff volume 23.89 129.23 153.11 c.m"
"      Runoff coefficient 0.244 0.878 0.625 "
"      Maximum flow 0.005 0.089 0.090 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.090 0.090 0.000 0.000"

```

```

" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.090 0.090 0.090 0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.090 c.m/sec"
"      Hydrograph volume 153.114 c.m"
"      0.090 0.090 0.090 0.090"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.090 0.000 0.090 0.090"
" 33 CATCHMENT 2012"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2012 Catchment 201-2"
"      70.000 % Impervious"
"      0.066 Total Area"
"      15.000 Flow length"
"      0.500 Overland Slope"
"      0.020 Pervious Area"
"      15.000 Pervious length"
"      0.500 Pervious slope"
"      0.046 Impervious Area"
"      15.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.243 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.878 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.014 0.000 0.090 0.090 c.m/sec"
"      Catchment 2012 Pervious Impervious Total Area "
"      Surface Area 0.020 0.046 0.066 hectare"
"      Time of concentration 19.740 1.981 3.865 minutes"
"      Time to Centroid 122.738 88.783 92.386 minutes"
"      Rainfall depth 47.240 47.240 47.240 mm"
"      Rainfall volume 9.35 21.82 31.18 c.m"
"      Rainfall losses 35.749 5.741 14.743 mm"
"      Runoff depth 11.491 41.499 32.497 mm"
"      Runoff volume 2.28 19.17 21.45 c.m"
"      Runoff coefficient 0.243 0.878 0.688 "
"      Maximum flow 0.001 0.014 0.014 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.014 0.014 0.090 0.090"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.014 0.014 0.014 0.090"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.101 c.m/sec"
"      Hydrograph volume 174.561 c.m"
"      0.014 0.014 0.014 0.101"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"

```

" 0.014 0.000 0.014 0.101"

" 33 CATCHMENT 2013"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2013 Catchment 201-3"

" 75.000 % Impervious"

" 0.234 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.058 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.176 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.243 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.874 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.056 0.000 0.014 0.101 c.m/sec"

" Catchment 2013 Pervious Impervious Total Area "

" Surface Area 0.058 0.176 0.234 hectare"

" Time of concentration 13.024 1.307 2.299 minutes"

" Time to Centroid 114.543 87.817 90.081 minutes"

" Rainfall depth 47.240 47.240 47.240 mm"

" Rainfall volume 27.64 82.91 110.54 c.m"

" Rainfall losses 35.778 5.949 13.406 mm"

" Runoff depth 11.461 41.291 33.834 mm"

" Runoff volume 6.70 72.47 79.17 c.m"

" Runoff coefficient 0.243 0.874 0.716 "

" Maximum flow 0.003 0.056 0.056 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.056 0.056 0.014 0.101"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.056 0.056 0.056 0.101"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.149 c.m/sec"

" Hydrograph volume 253.732 c.m"

" 0.056 0.056 0.056 0.149"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.056 0.000 0.056 0.149"

" 33 CATCHMENT 2014"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2014 Catchment 201-4"

" 100.000 % Impervious"

" 0.065 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.000 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.000 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.862 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.021 0.000 0.056 0.149 c.m/sec"

" Catchment 2014 Pervious Impervious Total Area "

" Surface Area 0.000 0.065 0.065 hectare"

" Time of concentration 10.211 1.025 1.025 minutes"

" Time to Centroid 111.096 87.416 87.416 minutes"

" Rainfall depth 47.240 47.240 47.240 mm"

" Rainfall volume 0.00 30.71 30.71 c.m"

" Rainfall losses 35.782 6.504 6.504 mm"

" Runoff depth 11.458 40.736 40.736 mm"

" Runoff volume 0.00 26.48 26.48 c.m"

" Runoff coefficient 0.000 0.862 0.862 "

" Maximum flow 0.000 0.021 0.021 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.021 0.021 0.056 0.149"

" 57 TRENCH Design d/s of 2014"

" 0.021 Peak inflow"

" 26.478 Hydrograph volume"

" 337.000 Ground elevation"

" 334.200 Downstream trench invert"

" 1.000 Trench height"

" 333.200 Water table elevation"

" 5.000 Trench top width"

" 5.000 Trench bottom width"

" 40.000 Voids ratio (%) "

" 30.000 Hydraulic conductivity"

" 0.000 Trench gradient (%) "

" 10.000 Trench length"

" 1.000 Include base width"

" 29. Number of stages"

" Level Discharge Volume"

" 334.200 0.000 0.0"

" 334.300 0.000 2.0"

" 334.400 0.000 4.0"

" 334.500 0.000 6.0"

" 334.600 0.000 8.0"

" 334.700 0.000 10.0"

" 334.800 0.000 12.0"

" 334.900 0.000 14.0"

" 335.000 0.000 16.0"

" 335.100 0.000 18.0"

" 335.200 0.000 20.0"

" 335.300 0.004 20.1"

" 335.400 0.006 20.2"

" 335.500 0.008 20.3"

" 335.600 0.010 20.5"

" 335.700 0.011 20.6"

" 335.800 0.013 20.7"

" 335.900 0.014 20.8"

" 336.000 0.015 20.9"

" 336.100 0.016 21.0"

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"          336.200      0.017      21.1"
"          336.300      0.018      21.2"
"          336.400      0.019      21.4"
"          336.500      0.020      21.5"
"          336.600      0.020      21.6"
"          336.700      0.021      21.7"
"          336.800      0.022      21.8"
"          336.900      0.023      21.9"
"          337.000      0.023      22.0"
"
"      1.  TRENCH PIPES"
"          Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"          Invert      length      diam.      grade%      0=Yes      distance"
"          335.200      20.000      0.000      1.000      1.000      0.000"
"
"      1.  MANHOLE"
"          Access"
"          diameter"
"          1.200"
"
"      1.  OUTFLOW PIPE"
"      0.  Inflow at upstream end of trench: 1=True; 0=False"
"          Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"          invert      invert      Length      Diameter      'n'      loss Ke"
"          335.200      335.000      20.000      0.100      0.013      0.500"
"
"          Peak outflow      0.001      c.m/sec"
"          Outflow volume      0.819      c.m"
"          Peak exfiltration      0.001      c.m/sec"
"          Exfiltration volume      25.594      c.m"
"          Maximum level      335.232      metre"
"          Maximum storage      20.036      c.m"
"          Centroidal lag      1.963      hours"
"          Infiltration area 2 sides      20.000      sq.metre"
"          Infiltration Base area      50.000      sq.metre"
"          0.021      0.021      0.001      0.001 c.m/sec"
"
" 40      HYDROGRAPH Combine      900"
"
"      6  Combine "
"
" 900  Node #"
"      SWMF"
"
"          Maximum flow      0.149      c.m/sec"
"          Hydrograph volume      254.552      c.m"
"          0.021      0.021      0.001      0.149"
"
" 40      HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.021      0.000      0.001      0.149"
"
" 33      CATCHMENT 2015"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"
"      2015  Catchment 201-5"
"
" 85.000  % Impervious"
"          0.074  Total Area"
"          15.000  Flow length"
"          2.000  Overland Slope"
"          0.011  Pervious Area"
"          15.000  Pervious length"
"          2.000  Pervious slope"
"          0.063  Impervious Area"
"          15.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.243  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.874  Impervious Runoff coefficient"

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"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.020      0.000      0.001      0.149 c.m/sec"
"
"          Catchment 2015      Pervious      Impervious Total Area "
"          Surface Area      0.011      0.063      0.074      hectare"
"          Time of concentration      13.024      1.307      1.854      minutes"
"          Time to Centroid      114.543      87.817      89.065      minutes"
"          Rainfall depth      47.240      47.240      47.240      mm"
"          Rainfall volume      5.24      29.71      34.96      c.m"
"          Rainfall losses      35.778      5.949      10.423      mm"
"          Runoff depth      11.461      41.291      36.817      mm"
"          Runoff volume      1.27      25.97      27.24      c.m"
"          Runoff coefficient      0.243      0.874      0.779      "
"          Maximum flow      0.001      0.020      0.020      c.m/sec"
"
" 40      HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.020      0.020      0.001      0.149"
"
" 40      HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.020      0.020      0.020      0.149"
"
" 40      HYDROGRAPH Combine      900"
"      6  Combine "
"
" 900  Node #"
"      SWMF"
"
"          Maximum flow      0.169      c.m/sec"
"          Hydrograph volume      281.796      c.m"
"          0.020      0.020      0.020      0.169"
"
" 40      HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.020      0.000      0.020      0.169"
"
" 33      CATCHMENT 2016"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"
"      2016  Catchment 201-6"
"
" 100.000  % Impervious"
"          0.032  Total Area"
"          10.000  Flow length"
"          2.000  Overland Slope"
"          0.000  Pervious Area"
"          10.000  Pervious length"
"          2.000  Pervious slope"
"          0.032  Impervious Area"
"          10.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.000  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.862  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.010      0.000      0.020      0.169 c.m/sec"
"
"          Catchment 2016      Pervious      Impervious Total Area "
"          Surface Area      0.000      0.032      0.032      hectare"
"          Time of concentration      10.211      1.025      1.025      minutes"
"          Time to Centroid      111.096      87.416      87.416      minutes"
"          Rainfall depth      47.240      47.240      47.240      mm"
"          Rainfall volume      0.00      15.12      15.12      c.m"
"          Rainfall losses      35.782      6.504      6.504      mm"
"          Runoff depth      11.458      40.736      40.736      mm"
"          Runoff volume      0.00      13.04      13.04      c.m"

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```

"      Runoff coefficient      0.000      0.862      0.862      "
"      Maximum flow           0.000      0.010      0.010      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.010      0.010      0.020      0.169"
" 57  TRENCH Design d/s of 2016"
"      0.010  Peak inflow"
"      13.036 Hydrograph volume"
"      337.000 Ground elevation"
"      334.200 Downstream trench invert"
"      1.000  Trench height"
"      333.200 Water table elevation"
"      5.000  Trench top width"
"      5.000  Trench bottom width"
"      40.000 Voids ratio (%)"
"      30.000 Hydraulic conductivity"
"      0.000  Trench gradient (%)"
"      5.000  Trench length"
"      1.000  Include base width"
"      29.  Number of stages"
"          Level Discharge      Volume"
"          334.200      0.000      0.0"
"          334.300      0.000      1.0"
"          334.400      0.000      2.0"
"          334.500      0.000      3.0"
"          334.600      0.000      4.0"
"          334.700      0.000      5.0"
"          334.800      0.000      6.0"
"          334.900      0.000      7.0"
"          335.000      0.000      8.0"
"          335.100      0.000      9.0"
"          335.200      0.000      10.0"
"          335.300      0.002      10.1"
"          335.400      0.006      10.2"
"          335.500      0.008      10.3"
"          335.600      0.010      10.5"
"          335.700      0.011      10.6"
"          335.800      0.013      10.7"
"          335.900      0.014      10.8"
"          336.000      0.015      10.9"
"          336.100      0.016      11.0"
"          336.200      0.017      11.1"
"          336.300      0.018      11.2"
"          336.400      0.019      11.4"
"          336.500      0.020      11.5"
"          336.600      0.020      11.6"
"          336.700      0.021      11.7"
"          336.800      0.022      11.8"
"          336.900      0.023      11.9"
"          337.000      0.023      12.0"
" 1.  TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      10.000      0.000      2.000      1.000      0.000"
" 1.  MANHOLE"
"      Access"
"      diameter"
"      1.200"
" 1.  OUTFLOW PIPE"
" 0.  Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      10.000      0.100      0.013      0.500"
"      Peak outflow      0.000      c.m/sec"
"      Outflow volume      0.253      c.m"

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```

"      Peak exfiltration      0.000      c.m/sec"
"      Exfiltration volume      12.770      c.m"
"      Maximum level      335.216      metre"
"      Maximum storage      10.018      c.m"
"      Centroidal lag      2.074      hours"
"      Infiltration area 2 sides      10.000      sq.metre"
"      Infiltration Base area      25.000      sq.metre"
"          0.010      0.010      0.000      0.000 c.m/sec"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"
"      Maximum flow      0.169      c.m/sec"
"      Hydrograph volume      282.050      c.m"
"          0.010      0.010      0.000      0.169"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.010      0.000      0.000      0.169"
" 33  CATCHMENT 2017"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      2017  Catchment 201-7 - SWMF"
"      40.000  % Impervious"
"      0.211  Total Area"
"      10.000  Flow length"
"      10.000  Overland Slope"
"      0.127  Pervious Area"
"      10.000  Pervious length"
"      10.000  Pervious slope"
"      0.084  Impervious Area"
"      10.000  Impervious length"
"      10.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.241  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.824  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.029      0.000      0.000      0.169 c.m/sec"
"      Catchment 2017      Pervious      Impervious      Total Area "
"      Surface Area      0.127      0.084      0.211      hectare"
"      Time of concentration      6.301      0.632      2.360      minutes"
"      Time to Centroid      106.453      87.264      93.114      minutes"
"      Rainfall depth      47.240      47.240      47.240      mm"
"      Rainfall volume      59.81      39.87      99.68      c.m"
"      Rainfall losses      35.855      8.303      24.834      mm"
"      Runoff depth      11.385      38.937      22.406      mm"
"      Runoff volume      14.41      32.86      47.28      c.m"
"      Runoff coefficient      0.241      0.824      0.474      "
"      Maximum flow      0.008      0.027      0.029      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.029      0.029      0.000      0.169"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.029      0.029      0.029      0.169"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"

```



"	Maximum flow	0.198	c.m/sec"
"	Hydrograph volume	329.325	c.m"
"	0.029 0.029 0.029	0.198"	
" 40	HYDROGRAPH Confluence	900"	
"	7 Confluence "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.198	c.m/sec"
"	Hydrograph volume	329.325	c.m"
"	0.029 0.198 0.029	0.000"	
" 54	POND DESIGN"		
"	0.198 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	329.3 Hydrograph volume	c.m"	
"	13. Number of stages"		
"	334.400 Minimum water level	metre"	
"	335.600 Maximum water level	metre"	
"	334.400 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.400 0.000	0.000"	
"	334.500 0.00150	40.000"	
"	334.600 0.00230	85.000"	
"	334.700 0.00290	135.000"	
"	334.800 0.03110	189.000"	
"	334.900 0.04300	249.000"	
"	335.000 0.05220	314.000"	
"	335.100 0.05990	384.000"	
"	335.200 0.06680	457.000"	
"	335.300 0.07300	534.000"	
"	335.400 0.1628	614.000"	
"	335.500 0.4275	698.000"	
"	335.600 0.9140	786.000"	
"	Peak outflow	0.034	c.m/sec"
"	Maximum level	334.823	metre"
"	Maximum storage	203.005	c.m"
"	Centroidal lag	6.984	hours"
"	0.029 0.198 0.034	0.000 c.m/sec"	
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.029 0.034 0.034	0.000"	
" 54	POND DESIGN"		
"	0.034 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	316.6 Hydrograph volume	c.m"	
"	10. Number of stages"		
"	334.200 Minimum water level	metre"	
"	335.100 Maximum water level	metre"	
"	334.200 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.200 0.000	0.000"	
"	334.300 0.00191	18.000"	
"	334.400 0.00207	38.000"	
"	334.500 0.00225	60.000"	
"	334.600 0.00243	83.000"	
"	334.700 0.00261	109.000"	
"	334.800 0.00281	136.000"	
"	334.900 0.1543	165.000"	
"	335.000 0.4628	196.000"	
"	335.100 0.9060	229.000"	
"	Peak outflow	0.016	c.m/sec"
"	Maximum level	334.809	metre"
"	Maximum storage	138.483	c.m"
"	Centroidal lag	15.315	hours"

"	0.029 0.034 0.016 0.000 c.m/sec"		
" 40	HYDROGRAPH Combine	800"	
"	6 Combine "		
"	800 Node #"		
"	TORRANCE CREEK"		
"	Maximum flow	0.016	c.m/sec"
"	Hydrograph volume	254.866	c.m"
"	0.029 0.034 0.016	0.016"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.029 0.000 0.016	0.016"	
" 33	CATCHMENT 2021"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2021 Catchment 202-1"		
"	0.000 % Impervious"		
"	0.863 Total Area"		
"	50.000 Flow length"		
"	0.500 Overland Slope"		
"	0.863 Pervious Area"		
"	50.000 Pervious length"		
"	0.500 Pervious slope"		
"	0.000 Impervious Area"		
"	50.000 Impervious length"		
"	0.500 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	74.000 Pervious SCS Curve No."		
"	0.243 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.000 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.020 0.000 0.016 0.016 c.m/sec"		
"	Catchment 2021 Pervious Impervious Total Area "		
"	Surface Area 0.863 0.000 0.863	hectare"	
"	Time of concentration 40.652 4.079	40.652	minutes"
"	Time to Centroid 148.337 91.940	148.336	minutes"
"	Rainfall depth 47.240 47.240	47.240	mm"
"	Rainfall volume 407.68 0.00	407.68	c.m"
"	Rainfall losses 35.737 5.719	35.737	mm"
"	Runoff depth 11.503 41.521	11.503	mm"
"	Runoff volume 99.27 0.00	99.27	c.m"
"	Runoff coefficient 0.243 0.000	0.243	"
"	Maximum flow 0.020 0.000	0.020	c.m/sec"
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.020 0.020 0.016 0.016"		
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.020 0.020 0.020 0.016"		
" 40	HYDROGRAPH Combine	800"	
"	6 Combine "		
"	800 Node #"		
"	TORRANCE CREEK"		
"	Maximum flow	0.022	c.m/sec"
"	Hydrograph volume	354.135	c.m"
"	0.020 0.020 0.020	0.022"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.020 0.000 0.020 0.022"		
" 33	CATCHMENT 2022"		

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"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2022 Catchment 202-2"
" 24.000 % Impervious"
"      0.184 Total Area"
" 10.000 Flow length"
"      2.000 Overland Slope"
"      0.140 Pervious Area"
" 10.000 Pervious length"
"      2.000 Pervious slope"
"      0.044 Impervious Area"
" 10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
"      0.243 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
"      0.862 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.016      0.000      0.020      0.022 c.m/sec"
"      Catchment 2022      Pervious      Impervious      Total Area
"      Surface Area      0.140      0.044      0.184      hectare"
"      Time of concentration      10.211      1.025      5.352      minutes"
"      Time to Centroid      111.096      87.416      98.571      minutes"
"      Rainfall depth      47.240      47.240      47.240      mm"
"      Rainfall volume      66.06      20.86      86.92      c.m"
"      Rainfall losses      35.782      6.504      28.755      mm"
"      Runoff depth      11.458      40.736      18.484      mm"
"      Runoff volume      16.02      17.99      34.01      c.m"
"      Runoff coefficient      0.243      0.862      0.391      "
"      Maximum flow      0.007      0.014      0.016      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.016      0.016      0.020      0.022"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.016      0.016      0.016      0.022"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.027      c.m/sec"
"      Hydrograph volume      388.147      c.m"
"      0.016      0.016      0.016      0.027"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.016      0.000      0.016      0.027"
" 33 CATCHMENT 2031"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2031 Catchment 203-1"
" 30.000 % Impervious"
"      0.216 Total Area"
" 130.000 Flow length"
"      2.000 Overland Slope"
"      0.151 Pervious Area"
" 130.000 Pervious length"
"      2.000 Pervious slope"
"      0.065 Impervious Area"

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" 130.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.244 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.882 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.019      0.000      0.016      0.027 c.m/sec"
"      Catchment 2031      Pervious      Impervious      Total Area
"      Surface Area      0.151      0.065      0.216      hectare"
"      Time of concentration      47.583      4.774      21.543      minutes"
"      Time to Centroid      156.815      92.944      117.964      minutes"
"      Rainfall depth      47.240      47.240      47.240      mm"
"      Rainfall volume      71.43      30.61      102.04      c.m"
"      Rainfall losses      35.734      5.552      26.679      mm"
"      Runoff depth      11.506      41.688      20.560      mm"
"      Runoff volume      17.40      27.01      44.41      c.m"
"      Runoff coefficient      0.244      0.882      0.435      "
"      Maximum flow      0.003      0.019      0.019      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.019      0.019      0.016      0.027"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.019      0.019      0.019      0.027"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow      0.019      c.m/sec"
"      Hydrograph volume      44.410      c.m"
"      0.019      0.019      0.019      0.019"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.019      0.000      0.019      0.019"
" 33 CATCHMENT 2032"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2032 Catchment 203-2"
"      0.000 % Impervious"
"      0.113 Total Area"
"      10.000 Flow length"
"      33.000 Overland Slope"
"      0.113 Pervious Area"
"      10.000 Pervious length"
"      33.000 Pervious slope"
"      0.000 Impervious Area"
"      10.000 Impervious length"
"      33.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.242 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"

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"      0.007      0.000      0.019      0.019 c.m/sec"
"      Catchment 2032      Pervious      Impervious      Total Area      "
"      Surface Area      0.113      0.000      0.113      hectare"
"      Time of concentration      4.404      0.442      4.404      minutes"
"      Time to Centroid      104.023      87.079      104.023      minutes"
"      Rainfall depth      47.240      47.240      47.240      mm"
"      Rainfall volume      53.38      0.00      53.38      c.m"
"      Rainfall losses      35.831      9.835      35.831      mm"
"      Runoff depth      11.408      37.405      11.408      mm"
"      Runoff volume      12.89      0.00      12.89      c.m"
"      Runoff coefficient      0.242      0.000      0.242      "
"      Maximum flow      0.007      0.000      0.007      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.007      0.007      0.019      0.019"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.007      0.007      0.007      0.019"
" 40      HYDROGRAPH Combine      700"
"      6      Combine "
"      700      Node #"
"      CULVERT"
"      Maximum flow      0.027      c.m/sec"
"      Hydrograph volume      57.302      c.m"
"      0.007      0.007      0.007      0.027"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.007      0.000      0.007      0.027"
" 33      CATCHMENT 2033"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      2033      Catchment 203-3"
"      100.000      % Impervious"
"      0.092      Total Area"
"      10.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      10.000      Pervious length"
"      2.000      Pervious slope"
"      0.092      Impervious Area"
"      10.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.000      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.862      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.030      0.000      0.007      0.027 c.m/sec"
"      Catchment 2033      Pervious      Impervious      Total Area      "
"      Surface Area      0.000      0.092      0.092      hectare"
"      Time of concentration      10.211      1.025      1.025      minutes"
"      Time to Centroid      111.096      87.416      87.416      minutes"
"      Rainfall depth      47.240      47.240      47.240      mm"
"      Rainfall volume      0.00      43.46      43.46      c.m"
"      Rainfall losses      35.782      6.504      6.504      mm"
"      Runoff depth      11.458      40.736      40.736      mm"
"      Runoff volume      0.00      37.48      37.48      c.m"
"      Runoff coefficient      0.000      0.862      0.862      "
"      Maximum flow      0.000      0.030      0.030      c.m/sec"

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" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.030      0.030      0.007      0.027"
" 57      TRENCH Design d/s of 2033"
"      0.030      Peak inflow"
"      37.477      Hydrograph volume"
"      337.000      Ground elevation"
"      334.500      Downstream trench invert"
"      1.000      Trench height"
"      333.500      Water table elevation"
"      1.000      Trench top width"
"      1.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      60.000      Trench length"
"      1.000      Include base width"
"      26.      Number of stages"
"      Level Discharge      Volume"
"      334.500      0.000      0.0"
"      334.600      0.000      2.4"
"      334.700      0.000      4.8"
"      334.800      0.000      7.2"
"      334.900      0.000      9.6"
"      335.000      0.000      12.0"
"      335.100      0.000      14.4"
"      335.200      0.000      16.8"
"      335.300      0.000      19.2"
"      335.400      0.000      21.6"
"      335.500      0.000      24.0"
"      335.600      0.004      24.1"
"      335.700      0.006      24.2"
"      335.800      0.008      24.3"
"      335.900      0.010      24.5"
"      336.000      0.011      24.6"
"      336.100      0.013      24.7"
"      336.200      0.014      24.8"
"      336.300      0.015      24.9"
"      336.400      0.016      25.0"
"      336.500      0.017      25.1"
"      336.600      0.018      25.2"
"      336.700      0.019      25.4"
"      336.800      0.020      25.5"
"      336.900      0.020      25.6"
"      337.000      0.021      25.7"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.500      20.000      0.000      1.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.500      335.300      20.000      0.100      0.013      0.500"
"      Peak outflow      0.003      c.m/sec"
"      Outflow volume      2.108      c.m"
"      Peak exfiltration      0.002      c.m/sec"
"      Exfiltration volume      34.919      c.m"
"      Maximum level      335.632      metre"
"      Maximum storage      24.149      c.m"
"      Centroidal lag      1.642      hours"

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"      Infiltration area 2 sides 120.000 sq.metre"
"      Infiltration Base area 60.000 sq.metre"
"      0.030 0.030 0.003 0.002 c.m/sec"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.027 c.m/sec"
"      Hydrograph volume 59.410 c.m"
"      0.030 0.030 0.003 0.027"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.030 0.000 0.003 0.027"
" 33 CATCHMENT 204"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      204 Catchment 204"
"      0.000 % Impervious"
"      0.198 Total Area"
"      8.000 Flow length"
"      20.000 Overland Slope"
"      0.198 Pervious Area"
"      8.000 Pervious length"
"      20.000 Pervious slope"
"      0.000 Impervious Area"
"      8.000 Impervious length"
"      20.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.242 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.013 0.000 0.003 0.027 c.m/sec"
"      Catchment 204 Pervious Impervious Total Area "
"      Surface Area 0.198 0.000 0.198 hectare"
"      Time of concentration 4.476 0.449 4.476 minutes"
"      Time to Centroid 104.105 87.097 104.104 minutes"
"      Rainfall depth 47.240 47.240 47.240 mm"
"      Rainfall volume 93.53 0.00 93.53 c.m"
"      Rainfall losses 35.825 9.778 35.825 mm"
"      Runoff depth 11.415 37.462 11.415 mm"
"      Runoff volume 22.60 0.00 22.60 c.m"
"      Runoff coefficient 0.242 0.000 0.242 "
"      Maximum flow 0.013 0.000 0.013 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.013 0.013 0.003 0.027"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.013 0.013 0.013 0.027"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.040 c.m/sec"
"      Hydrograph volume 82.010 c.m"
"      0.013 0.013 0.013 0.040"
" 40 HYDROGRAPH Confluence 700"
"      7 Confluence "

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"      700 Node #"
"      CULVERT"
"      Maximum flow 0.040 c.m/sec"
"      Hydrograph volume 82.010 c.m"
"      0.013 0.040 0.013 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.013 0.040 0.040 0.000"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow 0.058 c.m/sec"
"      Hydrograph volume 470.157 c.m"
"      0.013 0.040 0.040 0.058"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.013 0.000 0.040 0.058"
" 33 CATCHMENT 2051"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2051 Catchment 205-1"
"      25.000 % Impervious"
"      0.098 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.073 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.024 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.243 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.874 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.008 0.000 0.040 0.058 c.m/sec"
"      Catchment 2051 Pervious Impervious Total Area "
"      Surface Area 0.073 0.024 0.098 hectare"
"      Time of concentration 13.024 1.307 6.631 minutes"
"      Time to Centroid 114.543 87.817 99.960 minutes"
"      Rainfall depth 47.240 47.240 47.240 mm"
"      Rainfall volume 34.72 11.57 46.30 c.m"
"      Rainfall losses 35.778 5.949 28.321 mm"
"      Runoff depth 11.461 41.291 18.919 mm"
"      Runoff volume 8.42 10.12 18.54 c.m"
"      Runoff coefficient 0.243 0.874 0.400 "
"      Maximum flow 0.004 0.008 0.008 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.008 0.008 0.040 0.058"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.008 0.008 0.008 0.058"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"

```

" Maximum flow 0.008 c.m/sec"

" Hydrograph volume 18.541 c.m"

" 0.008 0.008 0.008 0.008"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.008 0.000 0.008 0.008"

" 33 CATCHMENT 2052"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2052 Catchment 205-2"

" 40.000 % Impervious"

" 0.108 Total Area"

" 25.000 Flow length"

" 5.000 Overland Slope"

" 0.065 Pervious Area"

" 25.000 Pervious length"

" 5.000 Pervious slope"

" 0.043 Impervious Area"

" 25.000 Impervious length"

" 5.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.243 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.875 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.014 0.000 0.008 0.008 c.m/sec"

" Catchment 2052 Pervious Impervious Total Area "

" Surface Area 0.065 0.043 0.108 hectare"

" Time of concentration 13.442 1.349 4.906 minutes"

" Time to Centroid 115.044 87.865 95.860 minutes"

" Rainfall depth 47.240 47.240 47.240 mm"

" Rainfall volume 30.61 20.41 51.02 c.m"

" Rainfall losses 35.756 5.905 23.816 mm"

" Runoff depth 11.484 41.334 23.424 mm"

" Runoff volume 7.44 17.86 25.30 c.m"

" Runoff coefficient 0.243 0.875 0.496 "

" Maximum flow 0.003 0.014 0.014 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.014 0.014 0.008 0.008"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.014 0.014 0.014 0.008"

" 40 HYDROGRAPH Combine 600"

" 6 Combine "

" 600 Node #"

" Arkell Rd"

" Maximum flow 0.022 c.m/sec"

" Hydrograph volume 43.839 c.m"

" 0.014 0.014 0.014 0.022"

" 40 HYDROGRAPH Confluence 600"

" 7 Confluence "

" 600 Node #"

" Arkell Rd"

" Maximum flow 0.022 c.m/sec"

" Hydrograph volume 43.839 c.m"

" 0.014 0.022 0.014 0.000"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.014 0.022 0.022 0.000"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.077 c.m/sec"

" Hydrograph volume 513.996 c.m"

" 0.014 0.022 0.022 0.077"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.014 0.000 0.022 0.077"

" 33 CATCHMENT 206"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 206 Catchment 206"

" 70.000 % Impervious"

" 0.035 Total Area"

" 20.000 Flow length"

" 1.250 Overland Slope"

" 0.011 Pervious Area"

" 20.000 Pervious length"

" 1.250 Pervious slope"

" 0.024 Impervious Area"

" 20.000 Impervious length"

" 1.250 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.243 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.878 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.008 0.000 0.022 0.077 c.m/sec"

" Catchment 206 Pervious Impervious Total Area "

" Surface Area 0.011 0.024 0.035 hectare"

" Time of concentration 17.821 1.788 3.489 minutes"

" Time to Centroid 120.385 88.491 91.875 minutes"

" Rainfall depth 47.240 47.240 47.240 mm"

" Rainfall volume 4.96 11.57 16.53 c.m"

" Rainfall losses 35.749 5.749 14.749 mm"

" Runoff depth 11.491 41.491 32.491 mm"

" Runoff volume 1.21 10.17 11.37 c.m"

" Runoff coefficient 0.243 0.878 0.688 "

" Maximum flow 0.000 0.007 0.008 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.008 0.008 0.022 0.077"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.008 0.008 0.008 0.077"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.083 c.m/sec"

" Hydrograph volume 525.368 c.m"

" 0.008 0.008 0.008 0.083"

" 40 HYDROGRAPH Confluence 800"

" 7 Confluence "

" 800 Node #"

" TORRANCE CREEK"

```

"      Maximum flow          0.083    c.m/sec"
"      Hydrograph volume    525.368    c.m"
"      0.008      0.083    0.008    0.000"
" 38  START/RE-START TOTALS 800"
"      3  Runoff Totals on EXIT"
"      Total Catchment area      3.108    hectare"
"      Total Impervious area      1.071    hectare"
"      Total % impervious      34.445"
" 19  EXIT"

```

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:             Q:\42063\104\SWM\September 2021\MIDUSS\
"                                POST"
"      Output filename:        10yrPOST.in"
"      Licensee name:          A"
"      Company                  Microsoft"
"      Date & Time last used:   10/5/2021 at 1:55:51 PM"
" 31  TIME PARAMETERS"
"      5.000    Time Step"
"      180.000  Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32  STORM Chicago storm"
"      1    Chicago storm"
"      2221.000 Coefficient A"
"      12.000    Constant B"
"      0.908    Exponent C"
"      0.400    Fraction R"
"      180.000  Duration"
"      1.000    Time step multiplier"
"      Maximum intensity      169.551    mm/hr"
"      Total depth            56.290    mm"
"      6  010hyd Hydrograph extension used in this file"
" 33  CATCHMENT 2011"
"      1    Triangular SCS"
"      1    Equal length"
"      1    SCS method"
"      2011  Catchment 201-1"
"      60.000 % Impervious"
"      0.519  Total Area"
"      60.000 Flow length"
"      0.750  Overland Slope"
"      0.208  Pervious Area"
"      60.000 Pervious length"
"      0.750  Pervious slope"
"      0.311  Impervious Area"
"      60.000 Impervious length"
"      0.750  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.292  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.889  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.110    0.000    0.000    0.000 c.m/sec"
"      Catchment 2011    Pervious    Impervious Total Area "
"      Surface Area      0.208    0.311    0.519    hectare"
"      Time of concentration 33.678    3.706    9.087    minutes"
"      Time to Centroid    138.194    90.447    99.019    minutes"
"      Rainfall depth      56.290    56.290    56.290    mm"
"      Rainfall volume     116.86    175.29    292.15    c.m"
"      Rainfall losses     39.872    6.269    19.710    mm"
"      Runoff depth        16.418    50.021    36.580    mm"
"      Runoff volume       34.08    155.77    189.85    c.m"
"      Runoff coefficient   0.292    0.889    0.650    "
"      Maximum flow        0.008    0.108    0.110    c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.110    0.110    0.000    0.000"

```

"	40	HYDROGRAPH Copy to Outflow"				
"		8	Copy to Outflow"			
"		0.110	0.110	0.110	0.000"	
"	40	HYDROGRAPH Combine 900"				
"		6	Combine "			
"		900	Node #"			
"		SWMF"				
"		Maximum flow	0.110	c.m/sec"		
"		Hydrograph volume	189.851	c.m"		
"		0.110	0.110	0.110	0.110"	
"	40	HYDROGRAPH Start - New Tributary"				
"		2	Start - New Tributary"			
"		0.110	0.000	0.110	0.110"	
"	33	CATCHMENT 2012"				
"		1	Triangular SCS"			
"		1	Equal length"			
"		1	SCS method"			
"		2012	Catchment 201-2"			
"		70.000	% Impervious"			
"		0.066	Total Area"			
"		15.000	Flow length"			
"		0.500	Overland Slope"			
"		0.020	Pervious Area"			
"		15.000	Pervious length"			
"		0.500	Pervious slope"			
"		0.046	Impervious Area"			
"		15.000	Impervious length"			
"		0.500	Impervious slope"			
"		0.250	Pervious Manning 'n'"			
"		74.000	Pervious SCS Curve No."			
"		0.291	Pervious Runoff coefficient"			
"		0.100	Pervious Ia/S coefficient"			
"		8.924	Pervious Initial abstraction"			
"		0.015	Impervious Manning 'n'"			
"		98.000	Impervious SCS Curve No."			
"		0.894	Impervious Runoff coefficient"			
"		0.100	Impervious Ia/S coefficient"			
"		0.518	Impervious Initial abstraction"			
"		0.018	0.000	0.110	0.110 c.m/sec"	
"		Catchment 2012	Pervious	Impervious	Total Area "	
"		Surface Area	0.020	0.046	0.066 hectare"	
"		Time of concentration	16.555	1.822	3.625 minutes"	
"		Time to Centroid	116.815	87.573	91.151 minutes"	
"		Rainfall depth	56.290	56.290	56.290 mm"	
"		Rainfall volume	11.15	26.01	37.15 c.m"	
"		Rainfall losses	39.909	5.945	16.134 mm"	
"		Runoff depth	16.381	50.345	40.156 mm"	
"		Runoff volume	3.24	23.26	26.50 c.m"	
"		Runoff coefficient	0.291	0.894	0.713 "	
"		Maximum flow	0.001	0.017	0.018 c.m/sec"	
"	40	HYDROGRAPH Add Runoff "				
"		4	Add Runoff "			
"		0.018	0.018	0.110	0.110"	
"	40	HYDROGRAPH Copy to Outflow"				
"		8	Copy to Outflow"			
"		0.018	0.018	0.018	0.110"	
"	40	HYDROGRAPH Combine 900"				
"		6	Combine "			
"		900	Node #"			
"		SWMF"				
"		Maximum flow	0.124	c.m/sec"		
"		Hydrograph volume	216.354	c.m"		
"		0.018	0.018	0.018	0.124"	
"	40	HYDROGRAPH Start - New Tributary"				
"		2	Start - New Tributary"			

"		0.018	0.000	0.018	0.124"	
"	33	CATCHMENT 2013"				
"		1	Triangular SCS"			
"		1	Equal length"			
"		1	SCS method"			
"		2013	Catchment 201-3"			
"		75.000	% Impervious"			
"		0.234	Total Area"			
"		15.000	Flow length"			
"		2.000	Overland Slope"			
"		0.058	Pervious Area"			
"		15.000	Pervious length"			
"		2.000	Pervious slope"			
"		0.176	Impervious Area"			
"		15.000	Impervious length"			
"		2.000	Impervious slope"			
"		0.250	Pervious Manning 'n'"			
"		74.000	Pervious SCS Curve No."			
"		0.291	Pervious Runoff coefficient"			
"		0.100	Pervious Ia/S coefficient"			
"		8.924	Pervious Initial abstraction"			
"		0.015	Impervious Manning 'n'"			
"		98.000	Impervious SCS Curve No."			
"		0.887	Impervious Runoff coefficient"			
"		0.100	Impervious Ia/S coefficient"			
"		0.518	Impervious Initial abstraction"			
"		0.070	0.000	0.018	0.124 c.m/sec"	
"		Catchment 2013	Pervious	Impervious	Total Area "	
"		Surface Area	0.058	0.176	0.234 hectare"	
"		Time of concentration	10.922	1.202	2.161 minutes"	
"		Time to Centroid	109.745	86.719	88.991 minutes"	
"		Rainfall depth	56.290	56.290	56.290 mm"	
"		Rainfall volume	32.93	98.79	131.72 c.m"	
"		Rainfall losses	39.899	6.369	14.752 mm"	
"		Runoff depth	16.391	49.921	41.539 mm"	
"		Runoff volume	9.59	87.61	97.20 c.m"	
"		Runoff coefficient	0.291	0.887	0.738 "	
"		Maximum flow	0.005	0.069	0.070 c.m/sec"	
"	40	HYDROGRAPH Add Runoff "				
"		4	Add Runoff "			
"		0.070	0.070	0.018	0.124"	
"	40	HYDROGRAPH Copy to Outflow"				
"		8	Copy to Outflow"			
"		0.070	0.070	0.070	0.124"	
"	40	HYDROGRAPH Combine 900"				
"		6	Combine "			
"		900	Node #"			
"		SWMF"				
"		Maximum flow	0.189	c.m/sec"		
"		Hydrograph volume	313.554	c.m"		
"		0.070	0.070	0.070	0.189"	
"	40	HYDROGRAPH Start - New Tributary"				
"		2	Start - New Tributary"			
"		0.070	0.000	0.070	0.189"	
"	33	CATCHMENT 2014"				
"		1	Triangular SCS"			
"		1	Equal length"			
"		1	SCS method"			
"		2014	Catchment 201-4"			
"		100.000	% Impervious"			
"		0.065	Total Area"			
"		10.000	Flow length"			
"		2.000	Overland Slope"			
"		0.000	Pervious Area"			
"		10.000	Pervious length"			

" 2.000 Pervious slope"

" 0.065 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.000 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.872 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.026 0.000 0.070 0.189 c.m/sec"

" Catchment 2014 Pervious Impervious Total Area "

" Surface Area 0.000 0.065 0.065 hectare"

" Time of concentration 8.564 0.942 0.942 minutes"

" Time to Centroid 106.793 86.363 86.363 minutes"

" Rainfall depth 56.290 56.290 56.290 mm"

" Rainfall volume 0.00 36.59 36.59 c.m"

" Rainfall losses 39.928 7.184 7.184 mm"

" Runoff depth 16.362 49.106 49.106 mm"

" Runoff volume 0.00 31.92 31.92 c.m"

" Runoff coefficient 0.000 0.872 0.872 "

" Maximum flow 0.000 0.026 0.026 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.026 0.026 0.070 0.189"

" 57 TRENCH Design d/s of 2014"

" 0.026 Peak inflow"

" 31.919 Hydrograph volume"

" 337.000 Ground elevation"

" 334.200 Downstream trench invert"

" 1.000 Trench height"

" 333.200 Water table elevation"

" 5.000 Trench top width"

" 5.000 Trench bottom width"

" 40.000 Voids ratio (%) "

" 30.000 Hydraulic conductivity"

" 0.000 Trench gradient (%) "

" 10.000 Trench length"

" 1.000 Include base width"

" 29. Number of stages"

" Level Discharge Volume"

" 334.200 0.000 0.0"

" 334.300 0.000 2.0"

" 334.400 0.000 4.0"

" 334.500 0.000 6.0"

" 334.600 0.000 8.0"

" 334.700 0.000 10.0"

" 334.800 0.000 12.0"

" 334.900 0.000 14.0"

" 335.000 0.000 16.0"

" 335.100 0.000 18.0"

" 335.200 0.000 20.0"

" 335.300 0.004 20.1"

" 335.400 0.006 20.2"

" 335.500 0.008 20.3"

" 335.600 0.010 20.5"

" 335.700 0.011 20.6"

" 335.800 0.013 20.7"

" 335.900 0.014 20.8"

" 336.000 0.015 20.9"

" 336.100 0.016 21.0"

" 336.200 0.017 21.1"

" 336.300 0.018 21.2"

" 336.400 0.019 21.4"

" 336.500 0.020 21.5"

" 336.600 0.020 21.6"

" 336.700 0.021 21.7"

" 336.800 0.022 21.8"

" 336.900 0.023 21.9"

" 337.000 0.023 22.0"

" 1. TRENCH PIPES"

" Downstream Pipe Pipe Pipe Perf'ted? Offset"

" Invert length diam. grade% 0=Yes distance"

" 335.200 20.000 0.000 1.000 1.000 0.000"

" 1. MANHOLE"

" Access"

" diameter"

" 1.200"

" 1. OUTFLOW PIPE"

" 0. Inflow at upstream end of trench: 1=True; 0=False"

" Upstream Downstr'm Pipe Pipe Manning Entry"

" invert invert Length Diameter 'n' loss Ke"

" 335.200 335.000 20.000 0.100 0.013 0.500"

" Peak outflow 0.006 c.m/sec"

" Outflow volume 5.409 c.m"

" Peak exfiltration 0.001 c.m/sec"

" Exfiltration volume 25.898 c.m"

" Maximum level 335.446 metre"

" Maximum storage 20.277 c.m"

" Centroidal lag 1.671 hours"

" Infiltration area 2 sides 20.000 sq.metre"

" Infiltration Base area 50.000 sq.metre"

" 0.026 0.026 0.006 0.001 c.m/sec"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.189 c.m/sec"

" Hydrograph volume 318.963 c.m"

" 0.026 0.026 0.006 0.189"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.026 0.000 0.006 0.189"

" 33 CATCHMENT 2015"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2015 Catchment 201-5"

" 85.000 % Impervious"

" 0.074 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.011 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.063 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.291 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.887 Impervious Runoff coefficient"



```

"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.025      0.000      0.006      0.189 c.m/sec"
"      Catchment 2015      Pervious      Impervious Total Area      "
"      Surface Area      0.011      0.063      0.074      hectare"
"      Time of concentration      10.922      1.202      1.734      minutes"
"      Time to Centroid      109.745      86.719      87.980      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      6.25      35.41      41.65      c.m"
"      Rainfall losses      39.899      6.369      11.398      mm"
"      Runoff depth      16.391      49.921      44.892      mm"
"      Runoff volume      1.82      31.40      33.22      c.m"
"      Runoff coefficient      0.291      0.887      0.798      "
"      Maximum flow      0.001      0.025      0.025      c.m/sec"
" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.025      0.025      0.006      0.189"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.025      0.025      0.025      0.189"
" 40      HYDROGRAPH Combine      900"
"      6      Combine      "
"      900      Node #      "
"      SWMF"
"      Maximum flow      0.214      c.m/sec"
"      Hydrograph volume      352.183      c.m"
"      0.025      0.025      0.025      0.214"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.025      0.000      0.025      0.214"
" 33      CATCHMENT 2016"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      2016      Catchment 201-6"
"      100.000      % Impervious"
"      0.032      Total Area"
"      10.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      10.000      Pervious length"
"      2.000      Pervious slope"
"      0.032      Impervious Area"
"      10.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.000      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.872      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.013      0.000      0.025      0.214 c.m/sec"
"      Catchment 2016      Pervious      Impervious Total Area      "
"      Surface Area      0.000      0.032      0.032      hectare"
"      Time of concentration      8.564      0.942      0.942      minutes"
"      Time to Centroid      106.793      86.363      86.363      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      0.00      18.01      18.01      c.m"
"      Rainfall losses      39.928      7.184      7.184      mm"
"      Runoff depth      16.362      49.106      49.106      mm"
"      Runoff volume      0.00      15.71      15.71      c.m"

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"      Runoff coefficient      0.000      0.872      0.872      "
"      Maximum flow      0.000      0.013      0.013      c.m/sec"
" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.013      0.013      0.025      0.214"
" 57      TRENCH Design d/s of 2016"
"      0.013      Peak inflow"
"      15.714      Hydrograph volume"
"      337.000      Ground elevation"
"      334.200      Downstream trench invert"
"      1.000      Trench height"
"      333.200      Water table elevation"
"      5.000      Trench top width"
"      5.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      5.000      Trench length"
"      1.000      Include base width"
"      29.      Number of stages"
"      Level Discharge      Volume"
"      334.200      0.000      0.0"
"      334.300      0.000      1.0"
"      334.400      0.000      2.0"
"      334.500      0.000      3.0"
"      334.600      0.000      4.0"
"      334.700      0.000      5.0"
"      334.800      0.000      6.0"
"      334.900      0.000      7.0"
"      335.000      0.000      8.0"
"      335.100      0.000      9.0"
"      335.200      0.000      10.0"
"      335.300      0.002      10.1"
"      335.400      0.006      10.2"
"      335.500      0.008      10.3"
"      335.600      0.010      10.5"
"      335.700      0.011      10.6"
"      335.800      0.013      10.7"
"      335.900      0.014      10.8"
"      336.000      0.015      10.9"
"      336.100      0.016      11.0"
"      336.200      0.017      11.1"
"      336.300      0.018      11.2"
"      336.400      0.019      11.4"
"      336.500      0.020      11.5"
"      336.600      0.020      11.6"
"      336.700      0.021      11.7"
"      336.800      0.022      11.8"
"      336.900      0.023      11.9"
"      337.000      0.023      12.0"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      10.000      0.000      2.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      10.000      0.100      0.013      0.500"
"      Peak outflow      0.003      c.m/sec"
"      Outflow volume      2.657      c.m"

```

"	Peak exfiltration	0.000	c.m/sec"
"	Exfiltration volume	12.921	c.m"
"	Maximum level	335.327	metre"
"	Maximum storage	10.143	c.m"
"	Centroidal lag	1.676	hours"
"	Infiltration area 2 sides	10.000	sq.metre"
"	Infiltration Base area	25.000	sq.metre"
"	0.013 0.013 0.003 0.000	c.m/sec"	
" 40	HYDROGRAPH Combine	900"	
"	6 Combine "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.214	c.m/sec"
"	Hydrograph volume	354.841	c.m"
"	0.013 0.013 0.003 0.214"		
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.013 0.000 0.003 0.214"		
" 33	CATCHMENT 2017"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2017 Catchment 201-7 - SWMF"		
"	40.000 % Impervious"		
"	0.211 Total Area"		
"	10.000 Flow length"		
"	10.000 Overland Slope"		
"	0.127 Pervious Area"		
"	10.000 Pervious length"		
"	10.000 Pervious slope"		
"	0.084 Impervious Area"		
"	10.000 Impervious length"		
"	10.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	74.000 Pervious SCS Curve No."		
"	0.290 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.829 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.038 0.000 0.003 0.214	c.m/sec"	
"	Catchment 2017 Pervious Impervious Total Area "		
"	Surface Area 0.127 0.084 0.211	hectare"	
"	Time of concentration 5.284 0.581 2.201	minutes"	
"	Time to Centroid 102.754 86.220 91.914	minutes"	
"	Rainfall depth 56.290 56.290 56.290	mm"	
"	Rainfall volume 71.26 47.51 118.77	c.m"	
"	Rainfall losses 39.958 9.650 27.835	mm"	
"	Runoff depth 16.332 46.640 28.455	mm"	
"	Runoff volume 20.68 39.36 60.04	c.m"	
"	Runoff coefficient 0.290 0.829 0.506	"	
"	Maximum flow 0.012 0.033 0.038	c.m/sec"	
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.038 0.038 0.003 0.214"		
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.038 0.038 0.038 0.214"		
" 40	HYDROGRAPH Combine	900"	
"	6 Combine "		
"	900 Node #"		
"	SWMF"		

"	Maximum flow	0.253	c.m/sec"
"	Hydrograph volume	414.881	c.m"
"	0.038 0.038 0.038 0.253"		
" 40	HYDROGRAPH Confluence	900"	
"	7 Confluence "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.253	c.m/sec"
"	Hydrograph volume	414.881	c.m"
"	0.038 0.253 0.038 0.000"		
" 54	POND DESIGN"		
"	0.253 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	414.9 Hydrograph volume	c.m"	
"	13. Number of stages"		
"	334.400 Minimum water level	metre"	
"	335.600 Maximum water level	metre"	
"	334.400 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.400 0.000 0.000"		
"	334.500 0.00150 40.000"		
"	334.600 0.00230 85.000"		
"	334.700 0.00290 135.000"		
"	334.800 0.03110 189.000"		
"	334.900 0.04300 249.000"		
"	335.000 0.05220 314.000"		
"	335.100 0.05990 384.000"		
"	335.200 0.06680 457.000"		
"	335.300 0.07300 534.000"		
"	335.400 0.1628 614.000"		
"	335.500 0.4275 698.000"		
"	335.600 0.9140 786.000"		
"	Peak outflow	0.044	c.m/sec"
"	Maximum level	334.910	metre"
"	Maximum storage	255.192	c.m"
"	Centroidal lag	6.044	hours"
"	0.038 0.253 0.044 0.000	c.m/sec"	
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.038 0.044 0.044 0.000"		
" 54	POND DESIGN"		
"	0.044 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	402.6 Hydrograph volume	c.m"	
"	10. Number of stages"		
"	334.200 Minimum water level	metre"	
"	335.100 Maximum water level	metre"	
"	334.200 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.200 0.000 0.000"		
"	334.300 0.00191 18.000"		
"	334.400 0.00207 38.000"		
"	334.500 0.00225 60.000"		
"	334.600 0.00243 83.000"		
"	334.700 0.00261 109.000"		
"	334.800 0.00281 136.000"		
"	334.900 0.1543 165.000"		
"	335.000 0.4628 196.000"		
"	335.100 0.9060 229.000"		
"	Peak outflow	0.036	c.m/sec"
"	Maximum level	334.822	metre"
"	Maximum storage	142.339	c.m"
"	Centroidal lag	12.714	hours"

" 0.038 0.044 0.036 0.000 c.m/sec"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.036 c.m/sec"

" Hydrograph volume 340.017 c.m"

" 0.038 0.044 0.036 0.036"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.038 0.000 0.036 0.036"

" 33 CATCHMENT 2021"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2021 Catchment 202-1"

" 0.000 % Impervious"

" 0.863 Total Area"

" 50.000 Flow length"

" 0.500 Overland Slope"

" 0.863 Pervious Area"

" 50.000 Pervious length"

" 0.500 Pervious slope"

" 0.000 Impervious Area"

" 50.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.292 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.000 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.034 0.000 0.036 0.036 c.m/sec"

" Catchment 2021 Pervious Impervious Total Area "

" Surface Area 0.863 0.000 0.863 hectare"

" Time of concentration 34.093 3.752 34.093 minutes"

" Time to Centroid 138.712 90.510 138.712 minutes"

" Rainfall depth 56.290 56.290 56.290 mm"

" Rainfall volume 485.78 0.00 485.78 c.m"

" Rainfall losses 39.876 6.201 39.876 mm"

" Runoff depth 16.415 50.089 16.415 mm"

" Runoff volume 141.66 0.00 141.66 c.m"

" Runoff coefficient 0.292 0.000 0.292 "

" Maximum flow 0.034 0.000 0.034 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.034 0.034 0.036 0.036"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.034 0.034 0.034 0.036"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.048 c.m/sec"

" Hydrograph volume 481.675 c.m"

" 0.034 0.034 0.034 0.048"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.034 0.000 0.034 0.048"

" 33 CATCHMENT 2022"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2022 Catchment 202-2"

" 24.000 % Impervious"

" 0.184 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.140 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.044 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.291 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.872 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.021 0.000 0.034 0.048 c.m/sec"

" Catchment 2022 Pervious Impervious Total Area "

" Surface Area 0.140 0.044 0.184 hectare"

" Time of concentration 8.564 0.942 4.855 minutes"

" Time to Centroid 106.793 86.363 96.852 minutes"

" Rainfall depth 56.290 56.290 56.290 mm"

" Rainfall volume 78.72 24.86 103.57 c.m"

" Rainfall losses 39.928 7.184 32.069 mm"

" Runoff depth 16.362 49.106 24.221 mm"

" Runoff volume 22.88 21.69 44.57 c.m"

" Runoff coefficient 0.291 0.872 0.430 "

" Maximum flow 0.012 0.018 0.021 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.021 0.021 0.034 0.048"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.021 0.021 0.021 0.048"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.050 c.m/sec"

" Hydrograph volume 526.241 c.m"

" 0.021 0.021 0.021 0.050"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.021 0.000 0.021 0.050"

" 33 CATCHMENT 2031"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2031 Catchment 203-1"

" 30.000 % Impervious"

" 0.216 Total Area"

" 130.000 Flow length"

" 2.000 Overland Slope"

" 0.151 Pervious Area"

" 130.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"

```

"      130.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.292 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.899 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.024 0.000 0.021 0.050 c.m/sec"
"      Catchment 2031 Pervious Impervious Total Area "
"      Surface Area 0.151 0.065 0.216 hectare"
"      Time of concentration 39.905 4.391 19.696 minutes"
"      Time to Centroid 145.975 91.370 114.901 minutes"
"      Rainfall depth 56.290 56.290 56.290 mm"
"      Rainfall volume 85.11 36.48 121.59 c.m"
"      Rainfall losses 39.875 5.711 29.626 mm"
"      Runoff depth 16.415 50.579 26.665 mm"
"      Runoff volume 24.82 32.78 57.60 c.m"
"      Runoff coefficient 0.292 0.899 0.474 "
"      Maximum flow 0.005 0.023 0.024 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
"      0.024 0.024 0.021 0.050"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
"      0.024 0.024 0.024 0.050"
" 40 HYDROGRAPH Combine 700"
" 6 Combine "
" 700 Node #"
" CULVERT"
" Maximum flow 0.024 c.m/sec"
" Hydrograph volume 57.595 c.m"
"      0.024 0.024 0.024 0.024"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
"      0.024 0.000 0.024 0.024"
" 33 CATCHMENT 2032"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 2032 Catchment 203-2"
" 0.000 % Impervious"
" 0.113 Total Area"
" 10.000 Flow length"
" 33.000 Overland Slope"
" 0.113 Pervious Area"
" 10.000 Pervious length"
" 33.000 Pervious slope"
" 0.000 Impervious Area"
" 10.000 Impervious length"
" 33.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.285 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"

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"      0.012 0.000 0.024 0.024 c.m/sec"
"      Catchment 2032 Pervious Impervious Total Area "
"      Surface Area 0.113 0.000 0.113 hectare"
"      Time of concentration 3.693 0.406 3.693 minutes"
"      Time to Centroid 100.959 85.958 100.959 minutes"
"      Rainfall depth 56.290 56.290 56.290 mm"
"      Rainfall volume 63.61 0.00 63.61 c.m"
"      Rainfall losses 40.243 11.359 40.243 mm"
"      Runoff depth 16.047 44.931 16.047 mm"
"      Runoff volume 18.13 0.00 18.13 c.m"
"      Runoff coefficient 0.285 0.000 0.285 "
"      Maximum flow 0.012 0.000 0.012 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
"      0.012 0.012 0.024 0.024"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
"      0.012 0.012 0.012 0.024"
" 40 HYDROGRAPH Combine 700"
" 6 Combine "
" 700 Node #"
" CULVERT"
" Maximum flow 0.036 c.m/sec"
" Hydrograph volume 75.729 c.m"
"      0.012 0.012 0.012 0.036"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
"      0.012 0.000 0.012 0.036"
" 33 CATCHMENT 2033"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 2033 Catchment 203-3"
" 100.000 % Impervious"
" 0.092 Total Area"
" 10.000 Flow length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 10.000 Pervious length"
" 2.000 Pervious slope"
" 0.092 Impervious Area"
" 10.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.000 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.872 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
"      0.036 0.000 0.012 0.036 c.m/sec"
"      Catchment 2033 Pervious Impervious Total Area "
"      Surface Area 0.000 0.092 0.092 hectare"
"      Time of concentration 8.564 0.942 0.942 minutes"
"      Time to Centroid 106.793 86.363 86.363 minutes"
"      Rainfall depth 56.290 56.290 56.290 mm"
"      Rainfall volume 0.00 51.79 51.79 c.m"
"      Rainfall losses 39.928 7.184 7.184 mm"
"      Runoff depth 16.362 49.106 49.106 mm"
"      Runoff volume 0.00 45.18 45.18 c.m"
"      Runoff coefficient 0.000 0.872 0.872 "
"      Maximum flow 0.000 0.036 0.036 c.m/sec"

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```

" 40      HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.036      0.036      0.012      0.036"
" 57      TRENCH Design d/s of 2033"
"      0.036      Peak inflow"
"      45.178      Hydrograph volume"
"      337.000      Ground elevation"
"      334.500      Downstream trench invert"
"      1.000      Trench height"
"      333.500      Water table elevation"
"      1.000      Trench top width"
"      1.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      60.000      Trench length"
"      1.000      Include base width"
"      26.      Number of stages"
"      Level Discharge      Volume"
"      334.500      0.000      0.0"
"      334.600      0.000      2.4"
"      334.700      0.000      4.8"
"      334.800      0.000      7.2"
"      334.900      0.000      9.6"
"      335.000      0.000      12.0"
"      335.100      0.000      14.4"
"      335.200      0.000      16.8"
"      335.300      0.000      19.2"
"      335.400      0.000      21.6"
"      335.500      0.000      24.0"
"      335.600      0.004      24.1"
"      335.700      0.006      24.2"
"      335.800      0.008      24.3"
"      335.900      0.010      24.5"
"      336.000      0.011      24.6"
"      336.100      0.013      24.7"
"      336.200      0.014      24.8"
"      336.300      0.015      24.9"
"      336.400      0.016      25.0"
"      336.500      0.017      25.1"
"      336.600      0.018      25.2"
"      336.700      0.019      25.4"
"      336.800      0.020      25.5"
"      336.900      0.020      25.6"
"      337.000      0.021      25.7"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.500      20.000      0.000      1.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.500      335.300      20.000      0.100      0.013      0.500"
"      Peak outflow      0.012      c.m/sec"
"      Outflow volume      8.771      c.m"
"      Peak exfiltration      0.003      c.m/sec"
"      Exfiltration volume      35.902      c.m"
"      Maximum level      336.118      metre"
"      Maximum storage      24.699      c.m"
"      Centroidal lag      1.522      hours"

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"      Infiltration area 2 sides      120.000      sq.metre"
"      Infiltration Base area      60.000      sq.metre"
"      0.036      0.036      0.012      0.003 c.m/sec"
" 40      HYDROGRAPH Combine      700"
"      6      Combine "
"      700      Node #"
"      CULVERT"
"      Maximum flow      0.036      c.m/sec"
"      Hydrograph volume      84.500      c.m"
"      0.036      0.036      0.012      0.036"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.036      0.000      0.012      0.036"
" 33      CATCHMENT 204"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      204      Catchment 204"
"      0.000      % Impervious"
"      0.198      Total Area"
"      8.000      Flow length"
"      20.000      Overland Slope"
"      0.198      Pervious Area"
"      8.000      Pervious length"
"      20.000      Pervious slope"
"      0.000      Impervious Area"
"      8.000      Impervious length"
"      20.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.286      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.000      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.021      0.000      0.012      0.036 c.m/sec"
"      Catchment 204      Pervious      Impervious      Total Area "
"      Surface Area      0.198      0.000      0.198      hectare"
"      Time of concentration      3.754      0.413      3.754      minutes"
"      Time to Centroid      101.003      85.977      101.003      minutes"
"      Rainfall depth      56.290      56.290      56.290      mm"
"      Rainfall volume      111.45      0.00      111.45      c.m"
"      Rainfall losses      40.210      11.286      40.210      mm"
"      Runoff depth      16.080      45.004      16.080      mm"
"      Runoff volume      31.84      0.00      31.84      c.m"
"      Runoff coefficient      0.286      0.000      0.286      "
"      Maximum flow      0.021      0.000      0.021      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.021      0.021      0.012      0.036"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.021      0.021      0.021      0.036"
" 40      HYDROGRAPH Combine      700"
"      6      Combine "
"      700      Node #"
"      CULVERT"
"      Maximum flow      0.057      c.m/sec"
"      Hydrograph volume      116.338      c.m"
"      0.021      0.021      0.057"
" 40      HYDROGRAPH Confluence      700"
"      7      Confluence "

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"      700 Node #"
"      CULVERT"
"      Maximum flow      0.057 c.m/sec"
"      Hydrograph volume 116.338 c.m"
"      0.021 0.057 0.021 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.021 0.057 0.057 0.000"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.086 c.m/sec"
"      Hydrograph volume 642.579 c.m"
"      0.021 0.057 0.057 0.086"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.021 0.000 0.057 0.086"
" 33 CATCHMENT 2051"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2051 Catchment 205-1"
"      25.000 % Impervious"
"      0.098 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.073 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.024 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.291 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.887 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.011 0.000 0.057 0.086 c.m/sec"
"      Catchment 2051 Pervious Impervious Total Area "
"      Surface Area 0.073 0.024 0.098 hectare"
"      Time of concentration 10.922 1.202 6.025 minutes"
"      Time to Centroid 109.745 86.719 98.145 minutes"
"      Rainfall depth 56.290 56.290 56.290 mm"
"      Rainfall volume 41.37 13.79 55.16 c.m"
"      Rainfall losses 39.899 6.369 31.517 mm"
"      Runoff depth 16.391 49.921 24.773 mm"
"      Runoff volume 12.05 12.23 24.28 c.m"
"      Runoff coefficient 0.291 0.887 0.440 "
"      Maximum flow 0.006 0.010 0.011 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.011 0.011 0.057 0.086"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.011 0.011 0.011 0.086"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.029 c.m/sec"
"      Hydrograph volume 56.496 c.m"
"      0.018 0.018 0.018 0.029"
" 40 HYDROGRAPH Confluence 600"
"      7 Confluence "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.029 c.m/sec"
"      Hydrograph volume 56.496 c.m"
"      0.018 0.029 0.018 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"

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"      Maximum flow      0.011 c.m/sec"
"      Hydrograph volume 24.278 c.m"
"      0.011 0.011 0.011 0.011"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.011 0.000 0.011 0.011"
" 33 CATCHMENT 2052"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2052 Catchment 205-2"
"      40.000 % Impervious"
"      0.108 Total Area"
"      25.000 Flow length"
"      5.000 Overland Slope"
"      0.065 Pervious Area"
"      25.000 Pervious length"
"      5.000 Pervious slope"
"      0.043 Impervious Area"
"      25.000 Impervious length"
"      5.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.291 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.888 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.018 0.000 0.011 0.011 c.m/sec"
"      Catchment 2052 Pervious Impervious Total Area "
"      Surface Area 0.065 0.043 0.108 hectare"
"      Time of concentration 11.273 1.241 4.548 minutes"
"      Time to Centroid 110.187 86.762 94.485 minutes"
"      Rainfall depth 56.290 56.290 56.290 mm"
"      Rainfall volume 36.48 24.32 60.79 c.m"
"      Rainfall losses 39.899 6.298 26.459 mm"
"      Runoff depth 16.391 49.992 29.832 mm"
"      Runoff volume 10.62 21.60 32.22 c.m"
"      Runoff coefficient 0.291 0.888 0.530 "
"      Maximum flow 0.005 0.017 0.018 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.018 0.018 0.011 0.011"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.018 0.018 0.018 0.011"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.029 c.m/sec"
"      Hydrograph volume 56.496 c.m"
"      0.018 0.018 0.018 0.029"
" 40 HYDROGRAPH Confluence 600"
"      7 Confluence "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.029 c.m/sec"
"      Hydrograph volume 56.496 c.m"
"      0.018 0.029 0.018 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"

```

"		0.018	0.029	0.029	0.000"
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	TORRANCE CREEK"				
"	Maximum flow		0.112	c.m/sec"	
"	Hydrograph volume		699.075	c.m"	
"		0.018	0.029	0.029	0.112"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		0.018	0.000	0.029	0.112"
" 33	CATCHMENT 206"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	206 Catchment 206"				
"	70.000 % Impervious"				
"	0.035 Total Area"				
"	20.000 Flow length"				
"	1.250 Overland Slope"				
"	0.011 Pervious Area"				
"	20.000 Pervious length"				
"	1.250 Pervious slope"				
"	0.024 Impervious Area"				
"	20.000 Impervious length"				
"	1.250 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.291 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.894 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.009	0.000	0.029	0.112 c.m/sec"
"	Catchment 206	Pervious	Impervious	Total Area	"
"	Surface Area	0.011	0.024	0.035	hectare"
"	Time of concentration	14.946	1.645	3.273	minutes"
"	Time to Centroid	114.808	87.294	90.663	minutes"
"	Rainfall depth	56.290	56.290	56.290	mm"
"	Rainfall volume	5.91	13.79	19.70	c.m"
"	Rainfall losses	39.897	5.940	16.127	mm"
"	Runoff depth	16.393	50.350	40.163	mm"
"	Runoff volume	1.72	12.34	14.06	c.m"
"	Runoff coefficient	0.291	0.894	0.714	"
"	Maximum flow	0.001	0.009	0.009	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.009	0.009	0.029	0.112"
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"		0.009	0.009	0.009	0.112"
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	TORRANCE CREEK"				
"	Maximum flow		0.119	c.m/sec"	
"	Hydrograph volume		713.132	c.m"	
"		0.009	0.009	0.009	0.119"
" 40	HYDROGRAPH Confluence	800"			
"	7 Confluence "				
"	800 Node #"				
"	TORRANCE CREEK"				

"	Maximum flow	0.119	c.m/sec"
"	Hydrograph volume	713.132	c.m"
"		0.009	0.119
"		0.009	0.000"
" 38	START/RE-START TOTALS 800"		
"	3 Runoff Totals on EXIT"		
"	Total Catchment area	3.108	hectare"
"	Total Impervious area	1.071	hectare"
"	Total % impervious	34.445"	
" 19	EXIT"		

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:             Q:\42063\104\SWM\September 2021\MIDUSS\
"                                POST"
"      Output filename:        25yrPOST.in"
"      Licensee name:          A"
"      Company                  Microsoft"
"      Date & Time last used:   10/5/2021 at 2:05:14 PM"
" 31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      3158.000 Coefficient A"
"      15.000 Constant B"
"      0.936 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity      191.271 mm/hr"
"      Total depth            68.087 mm"
"      6 025hyd Hydrograph extension used in this file"
" 33 CATCHMENT 2011"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2011 Catchment 201-1"
"      60.000 % Impervious"
"      0.519 Total Area"
"      60.000 Flow length"
"      0.750 Overland Slope"
"      0.208 Pervious Area"
"      60.000 Pervious length"
"      0.750 Pervious slope"
"      0.311 Impervious Area"
"      60.000 Impervious length"
"      0.750 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.346 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.903 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.128 0.000 0.000 0.000 c.m/sec"
"      Catchment 2011 Pervious Impervious Total Area "
"      Surface Area 0.208 0.311 0.519 hectare"
"      Time of concentration 29.417 3.518 8.791 minutes"
"      Time to Centroid 131.369 89.616 98.117 minutes"
"      Rainfall depth 68.087 68.087 68.087 mm"
"      Rainfall volume 141.35 212.02 353.37 c.m"
"      Rainfall losses 44.516 6.617 21.777 mm"
"      Runoff depth 23.571 61.469 46.310 mm"
"      Runoff volume 48.93 191.41 240.35 c.m"
"      Runoff coefficient 0.346 0.903 0.680 "
"      Maximum flow 0.014 0.124 0.128 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.128 0.128 0.000 0.000"

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" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.128 0.128 0.128 0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.128 c.m/sec"
"      Hydrograph volume 240.348 c.m"
"      0.128 0.128 0.128 0.128"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.128 0.000 0.128 0.128"
" 33 CATCHMENT 2012"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2012 Catchment 201-2"
"      70.000 % Impervious"
"      0.066 Total Area"
"      15.000 Flow length"
"      0.500 Overland Slope"
"      0.020 Pervious Area"
"      15.000 Pervious length"
"      0.500 Pervious slope"
"      0.046 Impervious Area"
"      15.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.346 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.910 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.021 0.000 0.128 0.128 c.m/sec"
"      Catchment 2012 Pervious Impervious Total Area "
"      Surface Area 0.020 0.046 0.066 hectare"
"      Time of concentration 14.461 1.729 3.511 minutes"
"      Time to Centroid 112.827 86.924 90.550 minutes"
"      Rainfall depth 68.087 68.087 68.087 mm"
"      Rainfall volume 13.48 31.46 44.94 c.m"
"      Rainfall losses 44.561 6.138 17.665 mm"
"      Runoff depth 23.526 61.949 50.422 mm"
"      Runoff volume 4.66 28.62 33.28 c.m"
"      Runoff coefficient 0.346 0.910 0.741 "
"      Maximum flow 0.002 0.020 0.021 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.021 0.021 0.128 0.128"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.021 0.021 0.021 0.128"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.145 c.m/sec"
"      Hydrograph volume 273.626 c.m"
"      0.021 0.021 0.021 0.145"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"

```



" 0.021 0.000 0.021 0.145"

" 33 CATCHMENT 2013"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2013 Catchment 201-3"

" 75.000 % Impervious"

" 0.234 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.058 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.176 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.344 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.899 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.082 0.000 0.021 0.145 c.m/sec"

" Catchment 2013 Pervious Impervious Total Area "

" Surface Area 0.058 0.176 0.234 hectare"

" Time of concentration 9.541 1.141 2.092 minutes"

" Time to Centroid 106.721 86.165 88.492 minutes"

" Rainfall depth 68.087 68.087 68.087 mm"

" Rainfall volume 39.83 119.49 159.32 c.m"

" Rainfall losses 44.632 6.843 16.290 mm"

" Runoff depth 23.455 61.244 51.796 mm"

" Runoff volume 13.72 107.48 121.20 c.m"

" Runoff coefficient 0.344 0.899 0.761 "

" Maximum flow 0.007 0.080 0.082 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.082 0.082 0.021 0.145"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.082 0.082 0.082 0.145"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.225 c.m/sec"

" Hydrograph volume 394.830 c.m"

" 0.082 0.082 0.082 0.225"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.082 0.000 0.082 0.225"

" 33 CATCHMENT 2014"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2014 Catchment 201-4"

" 100.000 % Impervious"

" 0.065 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.000 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.000 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.883 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.030 0.000 0.082 0.225 c.m/sec"

" Catchment 2014 Pervious Impervious Total Area "

" Surface Area 0.000 0.065 0.065 hectare"

" Time of concentration 7.480 0.894 0.894 minutes"

" Time to Centroid 104.266 85.842 85.842 minutes"

" Rainfall depth 68.087 68.087 68.087 mm"

" Rainfall volume 0.00 44.26 44.26 c.m"

" Rainfall losses 44.633 7.961 7.961 mm"

" Runoff depth 23.454 60.125 60.125 mm"

" Runoff volume 0.00 39.08 39.08 c.m"

" Runoff coefficient 0.000 0.883 0.883 "

" Maximum flow 0.000 0.030 0.030 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.030 0.030 0.082 0.225"

" 57 TRENCH Design d/s of 2014"

" 0.030 Peak inflow"

" 39.081 Hydrograph volume"

" 337.000 Ground elevation"

" 334.200 Downstream trench invert"

" 1.000 Trench height"

" 333.200 Water table elevation"

" 5.000 Trench top width"

" 5.000 Trench bottom width"

" 40.000 Voids ratio (%) "

" 30.000 Hydraulic conductivity"

" 0.000 Trench gradient (%) "

" 10.000 Trench length"

" 1.000 Include base width"

" 29. Number of stages"

" Level Discharge Volume"

" 334.200 0.000 0.0"

" 334.300 0.000 2.0"

" 334.400 0.000 4.0"

" 334.500 0.000 6.0"

" 334.600 0.000 8.0"

" 334.700 0.000 10.0"

" 334.800 0.000 12.0"

" 334.900 0.000 14.0"

" 335.000 0.000 16.0"

" 335.100 0.000 18.0"

" 335.200 0.000 20.0"

" 335.300 0.004 20.1"

" 335.400 0.006 20.2"

" 335.500 0.008 20.3"

" 335.600 0.010 20.5"

" 335.700 0.011 20.6"

" 335.800 0.013 20.7"

" 335.900 0.014 20.8"

" 336.000 0.015 20.9"

" 336.100 0.016 21.0"

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"          336.200      0.017      21.1"
"          336.300      0.018      21.2"
"          336.400      0.019      21.4"
"          336.500      0.020      21.5"
"          336.600      0.020      21.6"
"          336.700      0.021      21.7"
"          336.800      0.022      21.8"
"          336.900      0.023      21.9"
"          337.000      0.023      22.0"
"
"      1.  TRENCH PIPES"
"          Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"          Invert      length      diam.      grade%      0=Yes      distance"
"          335.200      20.000      0.000      1.000      1.000      0.000"
"
"      1.  MANHOLE"
"          Access"
"          diameter"
"          1.200"
"
"      1.  OUTFLOW PIPE"
"      0.  Inflow at upstream end of trench: 1=True; 0=False"
"          Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"          invert      invert      Length      Diameter      'n'      loss Ke"
"          335.200      335.000      20.000      0.100      0.013      0.500"
"
"          Peak outflow      0.012      c.m/sec"
"          Outflow volume      11.284      c.m"
"          Peak exfiltration      0.001      c.m/sec"
"          Exfiltration volume      26.325      c.m"
"          Maximum level      335.868      metre"
"          Maximum storage      20.756      c.m"
"          Centroidal lag      1.604      hours"
"          Infiltration area 2 sides      20.000      sq.metre"
"          Infiltration Base area      50.000      sq.metre"
"          0.030      0.030      0.012      0.001 c.m/sec"
"
" 40      HYDROGRAPH Combine      900"
"
"      6  Combine "
"
" 900  Node #"
"      SWMF"
"
"          Maximum flow      0.225      c.m/sec"
"          Hydrograph volume      406.114      c.m"
"          0.030      0.030      0.012      0.225"
"
" 40      HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.030      0.000      0.012      0.225"
"
" 33      CATCHMENT 2015"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"
"      2015  Catchment 201-5"
"      85.000  % Impervious"
"      0.074  Total Area"
"      15.000  Flow length"
"      2.000  Overland Slope"
"      0.011  Pervious Area"
"      15.000  Pervious length"
"      2.000  Pervious slope"
"      0.063  Impervious Area"
"      15.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.344  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.899  Impervious Runoff coefficient"

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"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.029      0.000      0.012      0.225 c.m/sec"
"
"          Catchment 2015      Pervious      Impervious Total Area "
"          Surface Area      0.011      0.063      0.074      hectare"
"          Time of concentration      9.541      1.141      1.673      minutes"
"          Time to Centroid      106.721      86.165      87.466      minutes"
"          Rainfall depth      68.087      68.087      68.087      mm"
"          Rainfall volume      7.56      42.83      50.38      c.m"
"          Rainfall losses      44.632      6.843      12.511      mm"
"          Runoff depth      23.455      61.244      55.575      mm"
"          Runoff volume      2.60      38.52      41.13      c.m"
"          Runoff coefficient      0.344      0.899      0.816      "
"          Maximum flow      0.001      0.029      0.029      c.m/sec"
"
" 40      HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.029      0.029      0.012      0.225"
"
" 40      HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.029      0.029      0.029      0.225"
"
" 40      HYDROGRAPH Combine      900"
"      6  Combine "
"
" 900  Node #"
"      SWMF"
"
"          Maximum flow      0.254      c.m/sec"
"          Hydrograph volume      447.240      c.m"
"          0.029      0.029      0.029      0.254"
"
" 40      HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.029      0.000      0.029      0.254"
"
" 33      CATCHMENT 2016"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"
"      2016  Catchment 201-6"
"      100.000  % Impervious"
"      0.032  Total Area"
"      10.000  Flow length"
"      2.000  Overland Slope"
"      0.000  Pervious Area"
"      10.000  Pervious length"
"      2.000  Pervious slope"
"      0.032  Impervious Area"
"      10.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.000  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.883  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.015      0.000      0.029      0.254 c.m/sec"
"
"          Catchment 2016      Pervious      Impervious Total Area "
"          Surface Area      0.000      0.032      0.032      hectare"
"          Time of concentration      7.480      0.894      0.894      minutes"
"          Time to Centroid      104.266      85.842      85.842      minutes"
"          Rainfall depth      68.087      68.087      68.087      mm"
"          Rainfall volume      0.00      21.79      21.79      c.m"
"          Rainfall losses      44.633      7.961      7.961      mm"
"          Runoff depth      23.454      60.125      60.125      mm"
"          Runoff volume      0.00      19.24      19.24      c.m"

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"      Runoff coefficient      0.000      0.883      0.883      "
"      Maximum flow            0.000      0.015      0.015      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.015      0.015      0.029      0.254"
" 57  TRENCH Design d/s of 2016"
"      0.015  Peak inflow"
"      19.240 Hydrograph volume"
"      337.000 Ground elevation"
"      334.200 Downstream trench invert"
"      1.000  Trench height"
"      333.200 Water table elevation"
"      5.000  Trench top width"
"      5.000  Trench bottom width"
"      40.000 Voids ratio (%)"
"      30.000 Hydraulic conductivity"
"      0.000  Trench gradient (%)"
"      5.000  Trench length"
"      1.000  Include base width"
"      29.  Number of stages"
"          Level Discharge      Volume"
"          334.200      0.000      0.0"
"          334.300      0.000      1.0"
"          334.400      0.000      2.0"
"          334.500      0.000      3.0"
"          334.600      0.000      4.0"
"          334.700      0.000      5.0"
"          334.800      0.000      6.0"
"          334.900      0.000      7.0"
"          335.000      0.000      8.0"
"          335.100      0.000      9.0"
"          335.200      0.000      10.0"
"          335.300      0.002      10.1"
"          335.400      0.006      10.2"
"          335.500      0.008      10.3"
"          335.600      0.010      10.5"
"          335.700      0.011      10.6"
"          335.800      0.013      10.7"
"          335.900      0.014      10.8"
"          336.000      0.015      10.9"
"          336.100      0.016      11.0"
"          336.200      0.017      11.1"
"          336.300      0.018      11.2"
"          336.400      0.019      11.4"
"          336.500      0.020      11.5"
"          336.600      0.020      11.6"
"          336.700      0.021      11.7"
"          336.800      0.022      11.8"
"          336.900      0.023      11.9"
"          337.000      0.023      12.0"
" 1.  TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      10.000      0.000      2.000      1.000      0.000"
" 1.  MANHOLE"
"      Access"
"      diameter"
"      1.200"
" 1.  OUTFLOW PIPE"
" 0.  Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      10.000      0.100      0.013      0.500"
"      Peak outflow      0.006      c.m/sec"
"      Outflow volume      5.450      c.m"

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"      Peak exfiltration      0.000      c.m/sec"
"      Exfiltration volume      13.116      c.m"
"      Maximum level      335.456      metre"
"      Maximum storage      10.289      c.m"
"      Centroidal lag      1.609      hours"
"      Infiltration area 2 sides      10.000      sq.metre"
"      Infiltration Base area      25.000      sq.metre"
"          0.015      0.015      0.006      0.000 c.m/sec"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"
"      Maximum flow      0.254      c.m/sec"
"      Hydrograph volume      452.690      c.m"
"          0.015      0.015      0.006      0.254"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.015      0.000      0.006      0.254"
" 33  CATCHMENT 2017"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      2017  Catchment 201-7 - SWMF"
"      40.000  % Impervious"
"      0.211  Total Area"
"      10.000  Flow length"
"      10.000  Overland Slope"
"      0.127  Pervious Area"
"      10.000  Pervious length"
"      10.000  Pervious slope"
"      0.084  Impervious Area"
"      10.000  Impervious length"
"      10.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.344  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.835  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.048      0.000      0.006      0.254 c.m/sec"
"      Catchment 2017      Pervious      Impervious      Total Area "
"      Surface Area      0.127      0.084      0.211      hectare"
"      Time of concentration      4.616      0.552      2.105      minutes"
"      Time to Centroid      100.646      85.713      91.419      minutes"
"      Rainfall depth      68.087      68.087      68.087      mm"
"      Rainfall volume      86.20      57.47      143.66      c.m"
"      Rainfall losses      44.649      11.241      31.286      mm"
"      Runoff depth      23.437      56.845      36.801      mm"
"      Runoff volume      29.67      47.98      77.65      c.m"
"      Runoff coefficient      0.344      0.835      0.540      "
"      Maximum flow      0.019      0.038      0.048      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.048      0.048      0.006      0.254"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.048      0.048      0.048      0.254"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"

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"		Maximum flow	0.301	c.m/sec"
"		Hydrograph volume	530.339	c.m"
"		0.048 0.048 0.048	0.301"	
" 40		HYDROGRAPH Confluence	900"	
"	7	Confluence "		
"	900	Node #"		
"		SWMF"		
"		Maximum flow	0.301	c.m/sec"
"		Hydrograph volume	530.339	c.m"
"		0.048 0.301 0.048	0.000"	
" 54		POND DESIGN"		
"	0.301	Current peak flow	c.m/sec"	
"	0.045	Target outflow	c.m/sec"	
"	530.3	Hydrograph volume	c.m"	
"	13.	Number of stages"		
"	334.400	Minimum water level	metre"	
"	335.600	Maximum water level	metre"	
"	334.400	Starting water level	metre"	
"	0	Keep Design Data: 1 = True; 0 = False"		
"		Level Discharge	Volume"	
"	334.400	0.000	0.000"	
"	334.500	0.00150	40.000"	
"	334.600	0.00230	85.000"	
"	334.700	0.00290	135.000"	
"	334.800	0.03110	189.000"	
"	334.900	0.04300	249.000"	
"	335.000	0.05220	314.000"	
"	335.100	0.05990	384.000"	
"	335.200	0.06680	457.000"	
"	335.300	0.07300	534.000"	
"	335.400	0.1628	614.000"	
"	335.500	0.4275	698.000"	
"	335.600	0.9140	786.000"	
"		Peak outflow	0.054	c.m/sec"
"		Maximum level	335.020	metre"
"		Maximum storage	328.056	c.m"
"		Centroidal lag	5.448	hours"
"	0.048 0.301	0.054 0.000	c.m/sec"	
" 40		HYDROGRAPH Next link "		
"	5	Next link "		
"	0.048	0.054 0.054	0.000"	
" 54		POND DESIGN"		
"	0.054	Current peak flow	c.m/sec"	
"	0.045	Target outflow	c.m/sec"	
"	516.7	Hydrograph volume	c.m"	
"	10.	Number of stages"		
"	334.200	Minimum water level	metre"	
"	335.100	Maximum water level	metre"	
"	334.200	Starting water level	metre"	
"	0	Keep Design Data: 1 = True; 0 = False"		
"		Level Discharge	Volume"	
"	334.200	0.000	0.000"	
"	334.300	0.00191	18.000"	
"	334.400	0.00207	38.000"	
"	334.500	0.00225	60.000"	
"	334.600	0.00243	83.000"	
"	334.700	0.00261	109.000"	
"	334.800	0.00281	136.000"	
"	334.900	0.1543	165.000"	
"	335.000	0.4628	196.000"	
"	335.100	0.9060	229.000"	
"		Peak outflow	0.050	c.m/sec"
"		Maximum level	334.831	metre"
"		Maximum storage	144.995	c.m"
"		Centroidal lag	10.744	hours"

"		0.048 0.054 0.050 0.000	c.m/sec"
" 40		HYDROGRAPH Combine	800"
"	6	Combine "	
"	800	Node #"	
"		TORRANCE CREEK"	
"		Maximum flow	0.050 c.m/sec"
"		Hydrograph volume	451.151 c.m"
"		0.048 0.054 0.050	0.050"
" 40		HYDROGRAPH Start - New Tributary"	
"	2	Start - New Tributary"	
"	0.048 0.000	0.050 0.050"	
" 33		CATCHMENT 2021"	
"	1	Triangular SCS"	
"	1	Equal length"	
"	1	SCS method"	
"	2021	Catchment 202-1"	
"	0.000	% Impervious"	
"	0.863	Total Area"	
"	50.000	Flow length"	
"	0.500	Overland Slope"	
"	0.863	Pervious Area"	
"	50.000	Pervious length"	
"	0.500	Pervious slope"	
"	0.000	Impervious Area"	
"	50.000	Impervious length"	
"	0.500	Impervious slope"	
"	0.250	Pervious Manning 'n'"	
"	74.000	Pervious SCS Curve No."	
"	0.346	Pervious Runoff coefficient"	
"	0.100	Pervious Ia/S coefficient"	
"	8.924	Pervious Initial abstraction"	
"	0.015	Impervious Manning 'n'"	
"	98.000	Impervious SCS Curve No."	
"	0.000	Impervious Runoff coefficient"	
"	0.100	Impervious Ia/S coefficient"	
"	0.518	Impervious Initial abstraction"	
"	0.057 0.000	0.050 0.050	c.m/sec"
"		Catchment 2021 Pervious Impervious Total Area "	
"		Surface Area 0.863 0.000 0.863	hectare"
"		Time of concentration 29.780 3.561 29.780	minutes"
"		Time to Centroid 131.824 89.674 131.824	minutes"
"		Rainfall depth 68.087 68.087 68.087	mm"
"		Rainfall volume 587.59 0.00 587.59	c.m"
"		Rainfall losses 44.508 6.651 44.508	mm"
"		Runoff depth 23.579 61.435 23.579	mm"
"		Runoff volume 203.48 0.00 203.48	c.m"
"		Runoff coefficient 0.346 0.000 0.346	"
"		Maximum flow 0.057 0.000 0.057	c.m/sec"
" 40		HYDROGRAPH Add Runoff "	
"	4	Add Runoff "	
"	0.057 0.057	0.050 0.050"	
" 40		HYDROGRAPH Copy to Outflow"	
"	8	Copy to Outflow"	
"	0.057 0.057	0.057 0.050"	
" 40		HYDROGRAPH Combine	800"
"	6	Combine "	
"	800	Node #"	
"		TORRANCE CREEK"	
"		Maximum flow	0.072 c.m/sec"
"		Hydrograph volume	654.635 c.m"
"		0.057 0.057 0.057	0.072"
" 40		HYDROGRAPH Start - New Tributary"	
"	2	Start - New Tributary"	
"	0.057 0.000	0.057 0.072"	
" 33		CATCHMENT 2022"	

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"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2022 Catchment 202-2"
" 24.000 % Impervious"
"      0.184 Total Area"
" 10.000 Flow length"
"      2.000 Overland Slope"
"      0.140 Pervious Area"
" 10.000 Pervious length"
"      2.000 Pervious slope"
"      0.044 Impervious Area"
" 10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
"      0.344 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
"      0.883 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.029      0.000      0.057      0.072 c.m/sec"
"      Catchment 2022      Pervious      Impervious      Total Area
"      Surface Area      0.140      0.044      0.184      hectare"
"      Time of concentration      7.480      0.894      4.534      minutes"
"      Time to Centroid      104.266      85.842      96.024      minutes"
"      Rainfall depth      68.087      68.087      68.087      mm"
"      Rainfall volume      95.21      30.07      125.28      c.m"
"      Rainfall losses      44.633      7.961      35.831      mm"
"      Runoff depth      23.454      60.125      32.255      mm"
"      Runoff volume      32.80      26.55      59.35      c.m"
"      Runoff coefficient      0.344      0.883      0.474      "
"      Maximum flow      0.019      0.020      0.029      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.029      0.029      0.057      0.072"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.029      0.029      0.029      0.072"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.075      c.m/sec"
"      Hydrograph volume      713.985      c.m"
"      0.029      0.029      0.029      0.075"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.029      0.000      0.029      0.075"
" 33 CATCHMENT 2031"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2031 Catchment 203-1"
" 30.000 % Impervious"
"      0.216 Total Area"
" 130.000 Flow length"
"      2.000 Overland Slope"
"      0.151 Pervious Area"
" 130.000 Pervious length"
"      2.000 Pervious slope"
"      0.065 Impervious Area"

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" 130.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.346 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.912 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.029      0.000      0.029      0.075 c.m/sec"
"      Catchment 2031      Pervious      Impervious      Total Area
"      Surface Area      0.151      0.065      0.216      hectare"
"      Time of concentration      34.857      4.168      18.582      minutes"
"      Time to Centroid      138.118      90.497      112.864      minutes"
"      Rainfall depth      68.087      68.087      68.087      mm"
"      Rainfall volume      102.95      44.12      147.07      c.m"
"      Rainfall losses      44.521      6.000      32.965      mm"
"      Runoff depth      23.565      62.087      35.122      mm"
"      Runoff volume      35.63      40.23      75.86      c.m"
"      Runoff coefficient      0.346      0.912      0.516      "
"      Maximum flow      0.009      0.027      0.029      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.029      0.029      0.029      0.075"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.029      0.029      0.029      0.075"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow      0.029      c.m/sec"
"      Hydrograph volume      75.863      c.m"
"      0.029      0.029      0.029      0.029"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.029      0.000      0.029      0.029"
" 33 CATCHMENT 2032"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2032 Catchment 203-2"
"      0.000 % Impervious"
"      0.113 Total Area"
"      10.000 Flow length"
"      33.000 Overland Slope"
"      0.113 Pervious Area"
"      10.000 Pervious length"
"      33.000 Pervious slope"
"      0.000 Impervious Area"
"      10.000 Impervious length"
"      33.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.338 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"

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"      0.017      0.000      0.029      0.029 c.m/sec"
"      Catchment 2032      Pervious      Impervious      Total Area      "
"      Surface Area      0.113      0.000      0.113      hectare"
"      Time of concentration      3.226      0.386      3.226      minutes"
"      Time to Centroid      99.023      85.385      99.023      minutes"
"      Rainfall depth      68.087      68.087      68.087      mm"
"      Rainfall volume      76.94      0.00      76.94      c.m"
"      Rainfall losses      45.081      13.216      45.081      mm"
"      Runoff depth      23.006      54.870      23.006      mm"
"      Runoff volume      26.00      0.00      26.00      c.m"
"      Runoff coefficient      0.338      0.000      0.338      "
"      Maximum flow      0.017      0.000      0.017      c.m/sec"
" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.017      0.017      0.029      0.029"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.017      0.017      0.017      0.029"
" 40      HYDROGRAPH Combine      700"
"      6      Combine      "
"      700      Node #      "
"      CULVERT"
"      Maximum flow      0.046      c.m/sec"
"      Hydrograph volume      101.859      c.m"
"      0.017      0.017      0.017      0.046"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.017      0.000      0.017      0.046"
" 33      CATCHMENT 2033"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      2033      Catchment 203-3"
"      100.000      % Impervious"
"      0.092      Total Area"
"      10.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      10.000      Pervious length"
"      2.000      Pervious slope"
"      0.092      Impervious Area"
"      10.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.000      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.883      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.042      0.000      0.017      0.046 c.m/sec"
"      Catchment 2033      Pervious      Impervious      Total Area      "
"      Surface Area      0.000      0.092      0.092      hectare"
"      Time of concentration      7.480      0.894      0.894      minutes"
"      Time to Centroid      104.266      85.842      85.842      minutes"
"      Rainfall depth      68.087      68.087      68.087      mm"
"      Rainfall volume      0.00      62.64      62.64      c.m"
"      Rainfall losses      44.633      7.961      7.961      mm"
"      Runoff depth      23.454      60.125      60.125      mm"
"      Runoff volume      0.00      55.32      55.32      c.m"
"      Runoff coefficient      0.000      0.883      0.883      "
"      Maximum flow      0.000      0.042      0.042      c.m/sec"

```

```

" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.042      0.042      0.017      0.046"
" 57      TRENCH Design d/s of 2033"
"      0.042      Peak inflow"
"      55.315      Hydrograph volume"
"      337.000      Ground elevation"
"      334.500      Downstream trench invert"
"      1.000      Trench height"
"      333.500      Water table elevation"
"      1.000      Trench top width"
"      1.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      60.000      Trench length"
"      1.000      Include base width"
"      26.      Number of stages"
"      Level Discharge      Volume"
"      334.500      0.000      0.0"
"      334.600      0.000      2.4"
"      334.700      0.000      4.8"
"      334.800      0.000      7.2"
"      334.900      0.000      9.6"
"      335.000      0.000      12.0"
"      335.100      0.000      14.4"
"      335.200      0.000      16.8"
"      335.300      0.000      19.2"
"      335.400      0.000      21.6"
"      335.500      0.000      24.0"
"      335.600      0.004      24.1"
"      335.700      0.006      24.2"
"      335.800      0.008      24.3"
"      335.900      0.010      24.5"
"      336.000      0.011      24.6"
"      336.100      0.013      24.7"
"      336.200      0.014      24.8"
"      336.300      0.015      24.9"
"      336.400      0.016      25.0"
"      336.500      0.017      25.1"
"      336.600      0.018      25.2"
"      336.700      0.019      25.4"
"      336.800      0.020      25.5"
"      336.900      0.020      25.6"
"      337.000      0.021      25.7"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.500      20.000      0.000      1.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.500      335.300      20.000      0.100      0.013      0.500"
"      Peak outflow      0.020      c.m/sec"
"      Outflow volume      18.859      c.m"
"      Peak exfiltration      0.003      c.m/sec"
"      Exfiltration volume      37.392      c.m"
"      Maximum level      336.946      metre"
"      Maximum storage      25.635      c.m"
"      Centroidal lag      1.475      hours"

```

```

"      Infiltration area 2 sides 120.000 sq.metre"
"      Infiltration Base area 60.000 sq.metre"
"      0.042 0.042 0.020 0.003 c.m/sec"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.066 c.m/sec"
"      Hydrograph volume 120.719 c.m"
"      0.042 0.042 0.020 0.066"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.042 0.000 0.020 0.066"
" 33 CATCHMENT 204"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      204 Catchment 204"
"      0.000 % Impervious"
"      0.198 Total Area"
"      8.000 Flow length"
"      20.000 Overland Slope"
"      0.198 Pervious Area"
"      8.000 Pervious length"
"      20.000 Pervious slope"
"      0.000 Impervious Area"
"      8.000 Impervious length"
"      20.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.337 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.031 0.000 0.020 0.066 c.m/sec"
"      Catchment 204 Pervious Impervious Total Area "
"      Surface Area 0.198 0.000 0.198 hectare"
"      Time of concentration 3.279 0.392 3.279 minutes"
"      Time to Centroid 99.107 85.405 99.107 minutes"
"      Rainfall depth 68.087 68.087 68.087 mm"
"      Rainfall volume 134.81 0.00 134.81 c.m"
"      Rainfall losses 45.109 13.152 45.109 mm"
"      Runoff depth 22.977 54.935 22.977 mm"
"      Runoff volume 45.50 0.00 45.50 c.m"
"      Runoff coefficient 0.337 0.000 0.337 "
"      Maximum flow 0.031 0.000 0.031 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.031 0.031 0.020 0.066"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.031 0.031 0.031 0.066"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.097 c.m/sec"
"      Hydrograph volume 166.214 c.m"
"      0.031 0.031 0.097"
" 40 HYDROGRAPH Confluence 700"
"      7 Confluence "

```

```

"      700 Node #"
"      CULVERT"
"      Maximum flow 0.097 c.m/sec"
"      Hydrograph volume 166.214 c.m"
"      0.031 0.097 0.031 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.031 0.097 0.097 0.000"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow 0.140 c.m/sec"
"      Hydrograph volume 880.199 c.m"
"      0.031 0.097 0.097 0.140"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.031 0.000 0.097 0.140"
" 33 CATCHMENT 2051"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2051 Catchment 205-1"
"      25.000 % Impervious"
"      0.098 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.073 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.024 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.344 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.899 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.015 0.000 0.097 0.140 c.m/sec"
"      Catchment 2051 Pervious Impervious Total Area "
"      Surface Area 0.073 0.024 0.098 hectare"
"      Time of concentration 9.541 1.141 5.632 minutes"
"      Time to Centroid 106.721 86.165 97.155 minutes"
"      Rainfall depth 68.087 68.087 68.087 mm"
"      Rainfall volume 50.04 16.68 66.72 c.m"
"      Rainfall losses 44.632 6.843 35.185 mm"
"      Runoff depth 23.455 61.244 32.902 mm"
"      Runoff volume 17.24 15.00 32.24 c.m"
"      Runoff coefficient 0.344 0.899 0.483 "
"      Maximum flow 0.009 0.011 0.015 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.015 0.015 0.097 0.140"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.015 0.015 0.015 0.140"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"

```

"	Maximum flow	0.015	c.m/sec"
"	Hydrograph volume	32.244	c.m"
"	0.015	0.015	0.015
"	0.015		0.015"
"	40		
"	HYDROGRAPH Start - New Tributary"		
"	2	Start - New Tributary"	
"	0.015	0.000	0.015
"	0.015		0.015"
"	33		
"	CATCHMENT 2052"		
"	1	Triangular SCS"	
"	1	Equal length"	
"	1	SCS method"	
"	2052	Catchment 205-2"	
"	40.000	% Impervious"	
"	0.108	Total Area"	
"	25.000	Flow length"	
"	5.000	Overland Slope"	
"	0.065	Pervious Area"	
"	25.000	Pervious length"	
"	5.000	Pervious slope"	
"	0.043	Impervious Area"	
"	25.000	Impervious length"	
"	5.000	Impervious slope"	
"	0.250	Pervious Manning 'n'"	
"	74.000	Pervious SCS Curve No."	
"	0.344	Pervious Runoff coefficient"	
"	0.100	Pervious Ia/S coefficient"	
"	8.924	Pervious Initial abstraction"	
"	0.015	Impervious Manning 'n'"	
"	98.000	Impervious SCS Curve No."	
"	0.901	Impervious Runoff coefficient"	
"	0.100	Impervious Ia/S coefficient"	
"	0.518	Impervious Initial abstraction"	
"	0.022	0.000	0.015
"	0.022		0.015 c.m/sec"
"	Catchment 2052	Pervious	Impervious
"	Surface Area	0.065	0.043
"	Time of concentration	9.847	1.177
"	Time to Centroid	107.117	86.210
"	Rainfall depth	68.087	68.087
"	Rainfall volume	44.12	29.41
"	Rainfall losses	44.655	6.729
"	Runoff depth	23.431	61.358
"	Runoff volume	15.18	26.51
"	Runoff coefficient	0.344	0.901
"	Maximum flow	0.008	0.020
"	0.022	0.020	0.022
"	0.022		0.022 c.m/sec"
"	40		
"	HYDROGRAPH Add Runoff "		
"	4	Add Runoff "	
"	0.022	0.022	0.015
"	0.022		0.015"
"	40		
"	HYDROGRAPH Copy to Outflow"		
"	8	Copy to Outflow"	
"	0.022	0.022	0.022
"	0.022		0.015"
"	40		
"	HYDROGRAPH Combine	600"	
"	6	Combine "	
"	600	Node #"	
"	Arkeell Rd"		
"	Maximum flow	0.035	c.m/sec"
"	Hydrograph volume	73.934	c.m"
"	0.022	0.022	0.022
"	0.022		0.035"
"	40		
"	HYDROGRAPH Confluence	600"	
"	7	Confluence "	
"	600	Node #"	
"	Arkeell Rd"		
"	Maximum flow	0.035	c.m/sec"
"	Hydrograph volume	73.934	c.m"
"	0.022	0.035	0.022
"	0.022		0.000"
"	40		
"	HYDROGRAPH Copy to Outflow"		
"	8	Copy to Outflow"	

"	0.022	0.035	0.035	0.000"
"	40			
"	HYDROGRAPH Combine	800"		
"	6	Combine "		
"	800	Node #"		
"	TORRANCE CREEK"			
"	Maximum flow	0.175	c.m/sec"	
"	Hydrograph volume	954.133	c.m"	
"	0.022	0.035	0.035	
"	0.022		0.175"	
"	40			
"	HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"		
"	0.022	0.000	0.035	
"	0.022		0.175"	
"	33			
"	CATCHMENT 206"			
"	1	Triangular SCS"		
"	1	Equal length"		
"	1	SCS method"		
"	206	Catchment 206"		
"	70.000	% Impervious"		
"	0.035	Total Area"		
"	20.000	Flow length"		
"	1.250	Overland Slope"		
"	0.011	Pervious Area"		
"	20.000	Pervious length"		
"	1.250	Pervious slope"		
"	0.024	Impervious Area"		
"	20.000	Impervious length"		
"	1.250	Impervious slope"		
"	0.250	Pervious Manning 'n'"		
"	74.000	Pervious SCS Curve No."		
"	0.345	Pervious Runoff coefficient"		
"	0.100	Pervious Ia/S coefficient"		
"	8.924	Pervious Initial abstraction"		
"	0.015	Impervious Manning 'n'"		
"	98.000	Impervious SCS Curve No."		
"	0.909	Impervious Runoff coefficient"		
"	0.100	Impervious Ia/S coefficient"		
"	0.518	Impervious Initial abstraction"		
"	0.011	0.000	0.035	
"	0.011		0.175 c.m/sec"	
"	Catchment 206	Pervious	Impervious	
"	Surface Area	0.011	0.024	
"	Time of concentration	13.055	1.561	
"	Time to Centroid	111.092	86.686	
"	Rainfall depth	68.087	68.087	
"	Rainfall volume	7.15	16.68	
"	Rainfall losses	44.589	6.207	
"	Runoff depth	23.497	61.880	
"	Runoff volume	2.47	15.16	
"	Runoff coefficient	0.345	0.909	
"	Maximum flow	0.001	0.011	
"	0.001	0.011	0.011	
"	0.001		0.175 c.m/sec"	
"	40			
"	HYDROGRAPH Add Runoff "			
"	4	Add Runoff "		
"	0.011	0.011	0.035	
"	0.011		0.175"	
"	40			
"	HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"		
"	0.011	0.011	0.011	
"	0.011		0.175"	
"	40			
"	HYDROGRAPH Combine	800"		
"	6	Combine "		
"	800	Node #"		
"	TORRANCE CREEK"			
"	Maximum flow	0.184	c.m/sec"	
"	Hydrograph volume	971.760	c.m"	
"	0.011	0.011	0.011	
"	0.011		0.184"	
"	40			
"	HYDROGRAPH Confluence	800"		
"	7	Confluence "		
"	800	Node #"		
"	TORRANCE CREEK"			



```

"      Maximum flow          0.184    c.m/sec"
"      Hydrograph volume    971.760    c.m"
"      0.011      0.184    0.011    0.000"
" 38      START/RE-START TOTALS 800"
" 3      Runoff Totals on EXIT"
"      Total Catchment area          3.108    hectare"
"      Total Impervious area        1.071    hectare"
"      Total % impervious          34.445"
" 19      EXIT"

```

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:             Q:\42063\104\SWM\September 2021\MIDUSS\
"                                POST"
"      Output filename:        50yrPOST.in"
"      Licensee name:          A"
"      Company                  Microsoft"
"      Date & Time last used:   10/5/2021 at 2:10:05 PM"
" 31      TIME PARAMETERS"
"      5.000    Time Step"
"      180.000    Max. Storm length"
"      1500.000    Max. Hydrograph"
" 32      STORM Chicago storm"
"      1    Chicago storm"
"      3886.000    Coefficient A"
"      16.000    Constant B"
"      0.950    Exponent C"
"      0.400    Fraction R"
"      180.000    Duration"
"      1.000    Time step multiplier"
"      Maximum intensity          215.474    mm/hr"
"      Total depth                77.443    mm"
"      6    050hyd    Hydrograph extension used in this file"
" 33      CATCHMENT 2011"
"      1    Triangular SCS"
"      1    Equal length"
"      1    SCS method"
"      2011    Catchment 201-1"
"      60.000    % Impervious"
"      0.519    Total Area"
"      60.000    Flow length"
"      0.750    Overland Slope"
"      0.208    Pervious Area"
"      60.000    Pervious length"
"      0.750    Pervious slope"
"      0.311    Impervious Area"
"      60.000    Impervious length"
"      0.750    Impervious slope"
"      0.250    Pervious Manning 'n'"
"      74.000    Pervious SCS Curve No."
"      0.384    Pervious Runoff coefficient"
"      0.100    Pervious Ia/S coefficient"
"      8.924    Pervious Initial abstraction"
"      0.015    Impervious Manning 'n'"
"      98.000    Impervious SCS Curve No."
"      0.913    Impervious Runoff coefficient"
"      0.100    Impervious Ia/S coefficient"
"      0.518    Impervious Initial abstraction"
"      0.145    0.000    0.000    0.000 c.m/sec"
"      Catchment 2011    Pervious    Impervious    Total Area "
"      Surface Area          0.208    0.311    0.519    hectare"
"      Time of concentration    26.629    3.347    8.448    minutes"
"      Time to Centroid        127.071    88.941    97.295    minutes"
"      Rainfall depth          77.443    77.443    77.443    mm"
"      Rainfall volume          160.77    241.16    401.93    c.m"
"      Rainfall losses          47.699    6.760    23.136    mm"
"      Runoff depth             29.744    70.683    54.307    mm"
"      Runoff volume             61.75    220.11    281.85    c.m"
"      Runoff coefficient        0.384    0.913    0.701    "
"      Maximum flow             0.019    0.141    0.145    c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4    Add Runoff "
"      0.145    0.145    0.000    0.000"

```

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.145 0.145 0.145 0.000"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.145 c.m/sec"

" Hydrograph volume 281.854 c.m"

" 0.145 0.145 0.145 0.145"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.145 0.000 0.145 0.145"

" 33 CATCHMENT 2012"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2012 Catchment 201-2"

" 70.000 % Impervious"

" 0.066 Total Area"

" 15.000 Flow length"

" 0.500 Overland Slope"

" 0.020 Pervious Area"

" 15.000 Pervious length"

" 0.500 Pervious slope"

" 0.046 Impervious Area"

" 15.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.383 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.919 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.024 0.000 0.145 0.145 c.m/sec"

" Catchment 2012 Pervious Impervious Total Area "

" Surface Area 0.020 0.046 0.066 hectare"

" Time of concentration 13.090 1.645 3.380 minutes"

" Time to Centroid 110.215 86.425 90.030 minutes"

" Rainfall depth 77.443 77.443 77.443 mm"

" Rainfall volume 15.33 35.78 51.11 c.m"

" Rainfall losses 47.794 6.300 18.749 mm"

" Runoff depth 29.648 71.142 58.694 mm"

" Runoff volume 5.87 32.87 38.74 c.m"

" Runoff coefficient 0.383 0.919 0.758 "

" Maximum flow 0.003 0.023 0.024 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.024 0.024 0.145 0.145"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.024 0.024 0.024 0.145"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.166 c.m/sec"

" Hydrograph volume 320.592 c.m"

" 0.024 0.024 0.024 0.166"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.024 0.000 0.024 0.166"

" 33 CATCHMENT 2013"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2013 Catchment 201-3"

" 75.000 % Impervious"

" 0.234 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.058 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.176 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.383 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.906 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.093 0.000 0.024 0.166 c.m/sec"

" Catchment 2013 Pervious Impervious Total Area "

" Surface Area 0.058 0.176 0.234 hectare"

" Time of concentration 8.636 1.085 2.018 minutes"

" Time to Centroid 104.625 85.713 88.049 minutes"

" Rainfall depth 77.443 77.443 77.443 mm"

" Rainfall volume 45.30 135.91 181.22 c.m"

" Rainfall losses 47.804 7.314 17.437 mm"

" Runoff depth 29.639 70.128 60.006 mm"

" Runoff volume 17.34 123.08 140.41 c.m"

" Runoff coefficient 0.383 0.906 0.775 "

" Maximum flow 0.009 0.091 0.093 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.093 0.093 0.024 0.166"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.093 0.093 0.093 0.166"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.260 c.m/sec"

" Hydrograph volume 461.006 c.m"

" 0.093 0.093 0.093 0.260"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.093 0.000 0.093 0.260"

" 33 CATCHMENT 2014"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2014 Catchment 201-4"

" 100.000 % Impervious"

" 0.065 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.000 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.000 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.887 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.033 0.000 0.093 0.260 c.m/sec"

" Catchment 2014 Pervious Impervious Total Area "

" Surface Area 0.000 0.065 0.065 hectare"

" Time of concentration 6.771 0.851 0.851 minutes"

" Time to Centroid 102.464 85.418 85.418 minutes"

" Rainfall depth 77.443 77.443 77.443 mm"

" Rainfall volume 0.00 50.34 50.34 c.m"

" Rainfall losses 48.008 8.731 8.731 mm"

" Runoff depth 29.434 68.712 68.712 mm"

" Runoff volume 0.00 44.66 44.66 c.m"

" Runoff coefficient 0.000 0.887 0.887 "

" Maximum flow 0.000 0.033 0.033 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.033 0.033 0.093 0.260"

" 57 TRENCH Design d/s of 2014"

" 0.033 Peak inflow"

" 44.663 Hydrograph volume"

" 337.000 Ground elevation"

" 334.200 Downstream trench invert"

" 1.000 Trench height"

" 333.200 Water table elevation"

" 5.000 Trench top width"

" 5.000 Trench bottom width"

" 40.000 Voids ratio (%) "

" 30.000 Hydraulic conductivity"

" 0.000 Trench gradient (%) "

" 10.000 Trench length"

" 1.000 Include base width"

" 29. Number of stages"

" Level Discharge Volume"

" 334.200 0.000 0.0"

" 334.300 0.000 2.0"

" 334.400 0.000 4.0"

" 334.500 0.000 6.0"

" 334.600 0.000 8.0"

" 334.700 0.000 10.0"

" 334.800 0.000 12.0"

" 334.900 0.000 14.0"

" 335.000 0.000 16.0"

" 335.100 0.000 18.0"

" 335.200 0.000 20.0"

" 335.300 0.004 20.1"

" 335.400 0.006 20.2"

" 335.500 0.008 20.3"

" 335.600 0.010 20.5"

" 335.700 0.011 20.6"

" 335.800 0.013 20.7"

" 335.900 0.014 20.8"

" 336.000 0.015 20.9"

" 336.100 0.016 21.0"

" 336.200 0.017 21.1"

" 336.300 0.018 21.2"

" 336.400 0.019 21.4"

" 336.500 0.020 21.5"

" 336.600 0.020 21.6"

" 336.700 0.021 21.7"

" 336.800 0.022 21.8"

" 336.900 0.023 21.9"

" 337.000 0.023 22.0"

" 1. TRENCH PIPES"

" Downstream Pipe Pipe Pipe Perf'ted? Offset"

" Invert length diam. grade% 0=Yes distance"

" 335.200 20.000 0.000 1.000 1.000 0.000"

" 1. MANHOLE"

" Access"

" diameter"

" 1.200"

" 1. OUTFLOW PIPE"

" 0. Inflow at upstream end of trench: 1=True; 0=False"

" Upstream Downstr'm Pipe Pipe Manning Entry"

" invert invert Length Diameter 'n' loss Ke"

" 335.200 335.000 20.000 0.100 0.013 0.500"

" Peak outflow 0.017 c.m/sec"

" Outflow volume 18.824 c.m"

" Peak exfiltration 0.001 c.m/sec"

" Exfiltration volume 26.629 c.m"

" Maximum level 336.319 metre"

" Maximum storage 21.265 c.m"

" Centroidal lag 1.535 hours"

" Infiltration area 2 sides 20.000 sq.metre"

" Infiltration Base area 50.000 sq.metre"

" 0.033 0.033 0.017 0.001 c.m/sec"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.260 c.m/sec"

" Hydrograph volume 479.830 c.m"

" 0.033 0.033 0.017 0.260"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.033 0.000 0.017 0.260"

" 33 CATCHMENT 2015"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2015 Catchment 201-5"

" 85.000 % Impervious"

" 0.074 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.011 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.063 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.383 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.906 Impervious Runoff coefficient"

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"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.033 0.000 0.017 0.260 c.m/sec"
"      Catchment 2015 Pervious Impervious Total Area "
"      Surface Area 0.011 0.063 0.074 hectare"
"      Time of concentration 8.636 1.085 1.610 minutes"
"      Time to Centroid 104.625 85.713 87.026 minutes"
"      Rainfall depth 77.443 77.443 77.443 mm"
"      Rainfall volume 8.60 48.71 57.31 c.m"
"      Rainfall losses 47.804 7.314 13.388 mm"
"      Runoff depth 29.639 70.128 64.055 mm"
"      Runoff volume 3.29 44.11 47.40 c.m"
"      Runoff coefficient 0.383 0.906 0.827 "
"      Maximum flow 0.002 0.032 0.033 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.033 0.033 0.017 0.260"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.033 0.033 0.033 0.260"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.293 c.m/sec"
"      Hydrograph volume 527.231 c.m"
"      0.033 0.033 0.033 0.293"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.033 0.000 0.033 0.293"
" 33 CATCHMENT 2016"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2016 Catchment 201-6"
"      100.000 % Impervious"
"      0.032 Total Area"
"      10.000 Flow length"
"      2.000 Overland Slope"
"      0.000 Pervious Area"
"      10.000 Pervious length"
"      2.000 Pervious slope"
"      0.032 Impervious Area"
"      10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.000 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.887 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.016 0.000 0.033 0.293 c.m/sec"
"      Catchment 2016 Pervious Impervious Total Area "
"      Surface Area 0.000 0.032 0.032 hectare"
"      Time of concentration 6.771 0.851 0.851 minutes"
"      Time to Centroid 102.464 85.418 85.418 minutes"
"      Rainfall depth 77.443 77.443 77.443 mm"
"      Rainfall volume 0.00 24.78 24.78 c.m"
"      Rainfall losses 48.008 8.731 8.731 mm"
"      Runoff depth 29.434 68.712 68.712 mm"
"      Runoff volume 0.00 21.99 21.99 c.m"

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"      Runoff coefficient 0.000 0.887 0.887 "
"      Maximum flow 0.000 0.016 0.016 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.016 0.016 0.033 0.293"
" 57 TRENCH Design d/s of 2016"
"      0.016 Peak inflow"
"      21.988 Hydrograph volume"
"      337.000 Ground elevation"
"      334.200 Downstream trench invert"
"      1.000 Trench height"
"      333.200 Water table elevation"
"      5.000 Trench top width"
"      5.000 Trench bottom width"
"      40.000 Voids ratio (%)"
"      30.000 Hydraulic conductivity"
"      0.000 Trench gradient (%)"
"      5.000 Trench length"
"      1.000 Include base width"
"      29. Number of stages"
"      Level Discharge Volume"
"      334.200 0.000 0.0"
"      334.300 0.000 1.0"
"      334.400 0.000 2.0"
"      334.500 0.000 3.0"
"      334.600 0.000 4.0"
"      334.700 0.000 5.0"
"      334.800 0.000 6.0"
"      334.900 0.000 7.0"
"      335.000 0.000 8.0"
"      335.100 0.000 9.0"
"      335.200 0.000 10.0"
"      335.300 0.002 10.1"
"      335.400 0.006 10.2"
"      335.500 0.008 10.3"
"      335.600 0.010 10.5"
"      335.700 0.011 10.6"
"      335.800 0.013 10.7"
"      335.900 0.014 10.8"
"      336.000 0.015 10.9"
"      336.100 0.016 11.0"
"      336.200 0.017 11.1"
"      336.300 0.018 11.2"
"      336.400 0.019 11.4"
"      336.500 0.020 11.5"
"      336.600 0.020 11.6"
"      336.700 0.021 11.7"
"      336.800 0.022 11.8"
"      336.900 0.023 11.9"
"      337.000 0.023 12.0"
" 1. TRENCH PIPES"
"      Downstream Pipe Pipe Pipe Perf'ted? Offset"
"      Invert length diam. grade% 0=Yes distance"
"      335.200 10.000 0.000 2.000 1.000 0.000"
" 1. MANHOLE"
"      Access"
"      diameter"
"      1.200"
" 1. OUTFLOW PIPE"
" 0. Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm Pipe Pipe Manning Entry"
"      invert invert Length Diameter 'n' loss Ke"
"      335.200 335.000 10.000 0.100 0.013 0.500"
"      Peak outflow 0.010 c.m/sec"
"      Outflow volume 9.325 c.m"

```

"	Peak exfiltration	0.000	c.m/sec"
"	Exfiltration volume	13.231	c.m"
"	Maximum level	335.589	metre"
"	Maximum storage	10.439	c.m"
"	Centroidal lag	1.532	hours"
"	Infiltration area 2 sides	10.000	sq.metre"
"	Infiltration Base area	25.000	sq.metre"
"	0.016 0.016 0.010 0.000	c.m/sec"	
" 40	HYDROGRAPH Combine	900"	
"	6 Combine "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.293	c.m/sec"
"	Hydrograph volume	536.556	c.m"
"	0.016 0.016 0.010	0.293"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.016 0.000 0.010	0.293"	
" 33	CATCHMENT 2017"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2017 Catchment 201-7 - SWMF"		
"	40.000 % Impervious"		
"	0.211 Total Area"		
"	10.000 Flow length"		
"	10.000 Overland Slope"		
"	0.127 Pervious Area"		
"	10.000 Pervious length"		
"	10.000 Pervious slope"		
"	0.084 Impervious Area"		
"	10.000 Impervious length"		
"	10.000 Impervious slope"		
"	0.250 Pervious Manning 'n' "		
"	74.000 Pervious SCS Curve No. "		
"	0.380 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n' "		
"	98.000 Impervious SCS Curve No. "		
"	0.836 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.057 0.000 0.010	0.293 c.m/sec"	
"	Catchment 2017 Pervious Impervious Total Area "		
"	Surface Area 0.127 0.084 0.211	hectare"	
"	Time of concentration 4.178 0.525 2.005	minutes"	
"	Time to Centroid 99.183 85.280 90.914	minutes"	
"	Rainfall depth 77.443 77.443 77.443	mm"	
"	Rainfall volume 98.04 65.36 163.40	c.m"	
"	Rainfall losses 48.031 12.691 33.895	mm"	
"	Runoff depth 29.412 64.751 43.548	mm"	
"	Runoff volume 37.24 54.65 91.89	c.m"	
"	Runoff coefficient 0.380 0.836 0.562	"	
"	Maximum flow 0.024 0.042 0.057	c.m/sec"	
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.057 0.057 0.010	0.293"	
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.057 0.057 0.057	0.293"	
" 40	HYDROGRAPH Combine	900"	
"	6 Combine "		
"	900 Node #"		
"	SWMF"		

"	Maximum flow	0.350	c.m/sec"
"	Hydrograph volume	628.441	c.m"
"	0.057 0.057 0.057	0.350"	
" 40	HYDROGRAPH Confluence	900"	
"	7 Confluence "		
"	900 Node #"		
"	SWMF"		
"	Maximum flow	0.350	c.m/sec"
"	Hydrograph volume	628.441	c.m"
"	0.057 0.350 0.057	0.000"	
" 54	POND DESIGN"		
"	0.350 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	628.4 Hydrograph volume	c.m"	
"	13. Number of stages"		
"	334.400 Minimum water level	metre"	
"	335.600 Maximum water level	metre"	
"	334.400 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.400 0.000	0.000"	
"	334.500 0.00150	40.000"	
"	334.600 0.00230	85.000"	
"	334.700 0.00290	135.000"	
"	334.800 0.03110	189.000"	
"	334.900 0.04300	249.000"	
"	335.000 0.05220	314.000"	
"	335.100 0.05990	384.000"	
"	335.200 0.06680	457.000"	
"	335.300 0.07300	534.000"	
"	335.400 0.1628	614.000"	
"	335.500 0.4275	698.000"	
"	335.600 0.9140	786.000"	
"	Peak outflow	0.061	c.m/sec"
"	Maximum level	335.116	metre"
"	Maximum storage	395.533	c.m"
"	Centroidal lag	5.123	hours"
"	0.057 0.350 0.061	0.000 c.m/sec"	
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.057 0.061 0.061	0.000"	
" 54	POND DESIGN"		
"	0.061 Current peak flow	c.m/sec"	
"	0.045 Target outflow	c.m/sec"	
"	614.1 Hydrograph volume	c.m"	
"	10. Number of stages"		
"	334.200 Minimum water level	metre"	
"	335.100 Maximum water level	metre"	
"	334.200 Starting water level	metre"	
"	0 Keep Design Data: 1 = True; 0 = False"		
"	Level Discharge	Volume"	
"	334.200 0.000	0.000"	
"	334.300 0.00191	18.000"	
"	334.400 0.00207	38.000"	
"	334.500 0.00225	60.000"	
"	334.600 0.00243	83.000"	
"	334.700 0.00261	109.000"	
"	334.800 0.00281	136.000"	
"	334.900 0.1543	165.000"	
"	335.000 0.4628	196.000"	
"	335.100 0.9060	229.000"	
"	Peak outflow	0.058	c.m/sec"
"	Maximum level	334.837	metre"
"	Maximum storage	146.656	c.m"
"	Centroidal lag	9.528	hours"

" 0.057 0.061 0.058 0.000 c.m/sec"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.058 c.m/sec"

" Hydrograph volume 548.572 c.m"

" 0.057 0.061 0.058 0.058"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.057 0.000 0.058 0.058"

" 33 CATCHMENT 2021"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2021 Catchment 202-1"

" 0.000 % Impervious"

" 0.863 Total Area"

" 50.000 Flow length"

" 0.500 Overland Slope"

" 0.863 Pervious Area"

" 50.000 Pervious length"

" 0.500 Pervious slope"

" 0.000 Impervious Area"

" 50.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.384 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.000 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.077 0.000 0.058 0.058 c.m/sec"

" Catchment 2021 Pervious Impervious Total Area "

" Surface Area 0.863 0.000 0.863 hectare"

" Time of concentration 26.957 3.388 26.957 minutes"

" Time to Centroid 127.483 89.006 127.483 minutes"

" Rainfall depth 77.443 77.443 77.443 mm"

" Rainfall volume 668.33 0.00 668.33 c.m"

" Rainfall losses 47.692 6.765 47.692 mm"

" Runoff depth 29.750 70.677 29.750 mm"

" Runoff volume 256.74 0.00 256.75 c.m"

" Runoff coefficient 0.384 0.000 0.384 "

" Maximum flow 0.077 0.000 0.077 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.077 0.077 0.058 0.058"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.077 0.077 0.077 0.058"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.091 c.m/sec"

" Hydrograph volume 805.317 c.m"

" 0.077 0.077 0.077 0.091"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.077 0.000 0.077 0.091"

" 33 CATCHMENT 2022"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2022 Catchment 202-2"

" 24.000 % Impervious"

" 0.184 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.140 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.044 Impervious Area"

" 10.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.380 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.887 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.037 0.000 0.077 0.091 c.m/sec"

" Catchment 2022 Pervious Impervious Total Area "

" Surface Area 0.140 0.044 0.184 hectare"

" Time of concentration 6.771 0.851 4.259 minutes"

" Time to Centroid 102.464 85.419 95.231 minutes"

" Rainfall depth 77.443 77.443 77.443 mm"

" Rainfall volume 108.30 34.20 142.49 c.m"

" Rainfall losses 48.008 8.731 38.582 mm"

" Runoff depth 29.434 68.712 38.861 mm"

" Runoff volume 41.16 30.34 71.50 c.m"

" Runoff coefficient 0.380 0.887 0.502 "

" Maximum flow 0.025 0.023 0.037 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.037 0.037 0.077 0.091"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.037 0.037 0.037 0.091"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.096 c.m/sec"

" Hydrograph volume 876.821 c.m"

" 0.037 0.037 0.037 0.096"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.037 0.000 0.037 0.096"

" 33 CATCHMENT 2031"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2031 Catchment 203-1"

" 30.000 % Impervious"

" 0.216 Total Area"

" 130.000 Flow length"

" 2.000 Overland Slope"

" 0.151 Pervious Area"

" 130.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"

"	130.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.384	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.918	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.033	0.000	0.037	0.096 c.m/sec"	
"	Catchment 2031	Pervious	Impervious	Total Area	"
"	Surface Area	0.151	0.065	0.216	hectare"
"	Time of concentration	31.553	3.966	17.588	minutes"
"	Time to Centroid	133.217	89.825	111.250	minutes"
"	Rainfall depth	77.443	77.443	77.443	mm"
"	Rainfall volume	117.09	50.18	167.28	c.m"
"	Rainfall losses	47.716	6.330	35.300	mm"
"	Runoff depth	29.727	71.113	42.143	mm"
"	Runoff volume	44.95	46.08	91.03	c.m"
"	Runoff coefficient	0.384	0.918	0.544	"
"	Maximum flow	0.012	0.030	0.033	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.033	0.033	0.037	0.096"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.033	0.033	0.033	0.096"	
" 40	HYDROGRAPH Combine 700"				
"	6 Combine "				
"	700 Node #"				
"	CULVERT"				
"	Maximum flow	0.033		c.m/sec"	
"	Hydrograph volume	91.028		c.m"	
"	0.033	0.033	0.033	0.033"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.033	0.000	0.033	0.033"	
" 33	CATCHMENT 2032"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2032 Catchment 203-2"				
"	0.000 % Impervious"				
"	0.113 Total Area"				
"	10.000 Flow length"				
"	33.000 Overland Slope"				
"	0.113 Pervious Area"				
"	10.000 Pervious length"				
"	33.000 Pervious slope"				
"	0.000 Impervious Area"				
"	10.000 Impervious length"				
"	33.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.376 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				

"	0.022	0.000	0.033	0.033 c.m/sec"	
"	Catchment 2032	Pervious	Impervious	Total Area	"
"	Surface Area	0.113	0.000	0.113	hectare"
"	Time of concentration	2.920	0.367	2.920	minutes"
"	Time to Centroid	97.553	84.895	97.553	minutes"
"	Rainfall depth	77.443	77.443	77.443	mm"
"	Rainfall volume	87.51	0.00	87.51	c.m"
"	Rainfall losses	48.296	14.614	48.295	mm"
"	Runoff depth	29.147	62.828	29.147	mm"
"	Runoff volume	32.94	0.00	32.94	c.m"
"	Runoff coefficient	0.376	0.000	0.376	"
"	Maximum flow	0.022	0.000	0.022	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.022	0.022	0.033	0.033"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.022	0.022	0.022	0.033"	
" 40	HYDROGRAPH Combine 700"				
"	6 Combine "				
"	700 Node #"				
"	CULVERT"				
"	Maximum flow	0.055		c.m/sec"	
"	Hydrograph volume	123.965		c.m"	
"	0.022	0.022	0.022	0.055"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.022	0.000	0.022	0.055"	
" 33	CATCHMENT 2033"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2033 Catchment 203-3"				
"	100.000 % Impervious"				
"	0.092 Total Area"				
"	10.000 Flow length"				
"	2.000 Overland Slope"				
"	0.000 Pervious Area"				
"	10.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.092 Impervious Area"				
"	10.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.000 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.887 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.047	0.000	0.022	0.055 c.m/sec"	
"	Catchment 2033	Pervious	Impervious	Total Area	"
"	Surface Area	0.000	0.092	0.092	hectare"
"	Time of concentration	6.771	0.851	0.851	minutes"
"	Time to Centroid	102.464	85.418	85.418	minutes"
"	Rainfall depth	77.443	77.443	77.443	mm"
"	Rainfall volume	0.00	71.25	71.25	c.m"
"	Rainfall losses	48.008	8.731	8.731	mm"
"	Runoff depth	29.434	68.712	68.712	mm"
"	Runoff volume	0.00	63.21	63.21	c.m"
"	Runoff coefficient	0.000	0.887	0.887	"
"	Maximum flow	0.000	0.047	0.047	c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.047      0.047      0.022      0.055"
" 57      TRENCH Design d/s of 2033"
"      0.047      Peak inflow"
"      63.215      Hydrograph volume"
"      337.000      Ground elevation"
"      334.500      Downstream trench invert"
"      1.000      Trench height"
"      333.500      Water table elevation"
"      1.000      Trench top width"
"      1.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      60.000      Trench length"
"      1.000      Include base width"
"      26.      Number of stages"
"      Level Discharge      Volume"
"      334.500      0.000      0.0"
"      334.600      0.000      2.4"
"      334.700      0.000      4.8"
"      334.800      0.000      7.2"
"      334.900      0.000      9.6"
"      335.000      0.000      12.0"
"      335.100      0.000      14.4"
"      335.200      0.000      16.8"
"      335.300      0.000      19.2"
"      335.400      0.000      21.6"
"      335.500      0.000      24.0"
"      335.600      0.005      24.1"
"      335.700      0.010      24.2"
"      335.800      0.016      24.3"
"      335.900      0.020      24.5"
"      336.000      0.024      24.6"
"      336.100      0.027      24.7"
"      336.200      0.030      24.8"
"      336.300      0.032      24.9"
"      336.400      0.035      25.0"
"      336.500      0.037      25.1"
"      336.600      0.039      25.2"
"      336.700      0.041      25.4"
"      336.800      0.043      25.5"
"      336.900      0.045      25.6"
"      337.000      0.047      25.7"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.500      20.000      0.000      1.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.500      335.300      20.000      0.150      0.013      0.500"
"      Peak outflow      0.029      c.m/sec"
"      Outflow volume      23.211      c.m"
"      Peak exfiltration      0.003      c.m/sec"
"      Exfiltration volume      37.739      c.m"
"      Maximum level      336.367      metre"
"      Maximum storage      24.981      c.m"
"      Centroidal lag      1.473      hours"

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"      Infiltration area 2 sides      120.000      sq.metre"
"      Infiltration Base area      60.000      sq.metre"
"      0.047      0.047      0.029      0.003 c.m/sec"
" 40      HYDROGRAPH Combine      700"
"      6      Combine "
"      700      Node #"
"      CULVERT"
"      Maximum flow      0.085      c.m/sec"
"      Hydrograph volume      147.175      c.m"
"      0.047      0.047      0.029      0.085"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.047      0.000      0.029      0.085"
" 33      CATCHMENT 204"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      204      Catchment 204"
"      0.000      % Impervious"
"      0.198      Total Area"
"      8.000      Flow length"
"      20.000      Overland Slope"
"      0.198      Pervious Area"
"      8.000      Pervious length"
"      20.000      Pervious slope"
"      0.000      Impervious Area"
"      8.000      Impervious length"
"      20.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.376      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.000      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.039      0.000      0.029      0.085 c.m/sec"
"      Catchment 204      Pervious      Impervious      Total Area "
"      Surface Area      0.198      0.000      0.198      hectare"
"      Time of concentration      2.968      0.373      2.968      minutes"
"      Time to Centroid      97.626      84.920      97.626      minutes"
"      Rainfall depth      77.443      77.443      77.443      mm"
"      Rainfall volume      153.34      0.00      153.34      c.m"
"      Rainfall losses      48.309      14.568      48.309      mm"
"      Runoff depth      29.134      62.875      29.134      mm"
"      Runoff volume      57.68      0.00      57.68      c.m"
"      Runoff coefficient      0.376      0.000      0.376      "
"      Maximum flow      0.039      0.000      0.039      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.039      0.039      0.029      0.085"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.039      0.039      0.039      0.085"
" 40      HYDROGRAPH Combine      700"
"      6      Combine "
"      700      Node #"
"      CULVERT"
"      Maximum flow      0.124      c.m/sec"
"      Hydrograph volume      204.860      c.m"
"      0.039      0.039      0.039      0.124"
" 40      HYDROGRAPH Confluence      700"
"      7      Confluence "

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"      700 Node #"
"      CULVERT"
"      Maximum flow      0.124 c.m/sec"
"      Hydrograph volume 204.860 c.m"
"      0.039 0.124 0.039 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.039 0.124 0.124 0.000"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.184 c.m/sec"
"      Hydrograph volume 1081.681 c.m"
"      0.039 0.124 0.124 0.184"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.039 0.000 0.124 0.184"
" 33 CATCHMENT 2051"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2051 Catchment 205-1"
"      25.000 % Impervious"
"      0.098 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.073 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.024 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.383 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.906 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.019 0.000 0.124 0.184 c.m/sec"
"      Catchment 2051 Pervious Impervious Total Area "
"      Surface Area 0.073 0.024 0.098 hectare"
"      Time of concentration 8.636 1.085 5.307 minutes"
"      Time to Centroid 104.625 85.713 96.286 minutes"
"      Rainfall depth 77.443 77.443 77.443 mm"
"      Rainfall volume 56.92 18.97 75.89 c.m"
"      Rainfall losses 47.804 7.314 37.681 mm"
"      Runoff depth 29.639 70.128 39.761 mm"
"      Runoff volume 21.78 17.18 38.97 c.m"
"      Runoff coefficient 0.383 0.906 0.513 "
"      Maximum flow 0.012 0.013 0.019 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.019 0.019 0.124 0.184"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.019 0.019 0.019 0.184"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"

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"      Maximum flow      0.019 c.m/sec"
"      Hydrograph volume 38.966 c.m"
"      0.019 0.019 0.019 0.019"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.019 0.000 0.019 0.019"
" 33 CATCHMENT 2052"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2052 Catchment 205-2"
"      40.000 % Impervious"
"      0.108 Total Area"
"      25.000 Flow length"
"      5.000 Overland Slope"
"      0.065 Pervious Area"
"      25.000 Pervious length"
"      5.000 Pervious slope"
"      0.043 Impervious Area"
"      25.000 Impervious length"
"      5.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.382 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.907 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.025 0.000 0.019 0.019 c.m/sec"
"      Catchment 2052 Pervious Impervious Total Area "
"      Surface Area 0.065 0.043 0.108 hectare"
"      Time of concentration 8.914 1.120 4.138 minutes"
"      Time to Centroid 104.988 85.758 93.205 minutes"
"      Rainfall depth 77.443 77.443 77.443 mm"
"      Rainfall volume 50.18 33.46 83.64 c.m"
"      Rainfall losses 47.837 7.179 31.574 mm"
"      Runoff depth 29.605 70.264 45.869 mm"
"      Runoff volume 19.18 30.35 49.54 c.m"
"      Runoff coefficient 0.382 0.907 0.592 "
"      Maximum flow 0.010 0.022 0.025 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.025 0.025 0.019 0.019"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.025 0.025 0.025 0.019"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.043 c.m/sec"
"      Hydrograph volume 88.504 c.m"
"      0.025 0.025 0.025 0.043"
" 40 HYDROGRAPH Confluence 600"
"      7 Confluence "
"      600 Node #"
"      Arkell Rd"
"      Maximum flow      0.043 c.m/sec"
"      Hydrograph volume 88.504 c.m"
"      0.025 0.043 0.025 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"

```

"		0.025	0.043	0.043	0.000"
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	TORRANCE CREEK"				
"	Maximum flow		0.227	c.m/sec"	
"	Hydrograph volume		1170.185	c.m"	
"		0.025	0.043	0.043	0.227"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		0.025	0.000	0.043	0.227"
" 33	CATCHMENT 206"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	206 Catchment 206"				
"	70.000 % Impervious"				
"	0.035 Total Area"				
"	20.000 Flow length"				
"	1.250 Overland Slope"				
"	0.011 Pervious Area"				
"	20.000 Pervious length"				
"	1.250 Pervious slope"				
"	0.024 Impervious Area"				
"	20.000 Impervious length"				
"	1.250 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.383 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.917 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.013	0.000	0.043	0.227 c.m/sec"
"	Catchment 206	Pervious	Impervious	Total Area	"
"	Surface Area	0.011	0.024	0.035	hectare"
"	Time of concentration	11.818	1.485	3.054	minutes"
"	Time to Centroid	108.607	86.190	89.592	minutes"
"	Rainfall depth	77.443	77.443	77.443	mm"
"	Rainfall volume	8.13	18.97	27.10	c.m"
"	Rainfall losses	47.777	6.393	18.808	mm"
"	Runoff depth	29.666	71.050	58.635	mm"
"	Runoff volume	3.11	17.41	20.52	c.m"
"	Runoff coefficient	0.383	0.917	0.757	"
"	Maximum flow	0.001	0.012	0.013	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.013	0.013	0.043	0.227"
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"		0.013	0.013	0.013	0.227"
" 40	HYDROGRAPH Combine	800"			
"	6 Combine "				
"	800 Node #"				
"	TORRANCE CREEK"				
"	Maximum flow		0.237	c.m/sec"	
"	Hydrograph volume		1190.707	c.m"	
"		0.013	0.013	0.013	0.237"
" 40	HYDROGRAPH Confluence	800"			
"	7 Confluence "				
"	800 Node #"				
"	TORRANCE CREEK"				

"	Maximum flow	0.237	c.m/sec"
"	Hydrograph volume	1190.707	c.m"
"		0.013	0.237
"		0.013	0.000"
" 38	START/RE-START TOTALS 800"		
"	3 Runoff Totals on EXIT"		
"	Total Catchment area	3.108	hectare"
"	Total Impervious area	1.071	hectare"
"	Total % impervious	34.445"	
" 19	EXIT"		

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:              Q:\42063\104\SWM\September 2021\MIDUSS\
"                                POST"
"      Output filename:         100yrPOST.in"
"      Licensee name:           A"
"      Company                  Microsoft"
"      Date & Time last used:    10/5/2021 at 2:19:34 PM"
" 31 TIME PARAMETERS"
"      5.000 Time Step"
"      180.000 Max. Storm length"
"      1500.000 Max. Hydrograph"
" 32 STORM Chicago storm"
"      1 Chicago storm"
"      4688.000 Coefficient A"
"      17.000 Constant B"
"      0.962 Exponent C"
"      0.400 Fraction R"
"      180.000 Duration"
"      1.000 Time step multiplier"
"      Maximum intensity        239.650 mm/hr"
"      Total depth               87.263 mm"
"      6 100hyd Hydrograph extension used in this file"
" 33 CATCHMENT 2011"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2011 Catchment 201-1"
"      60.000 % Impervious"
"      0.519 Total Area"
"      60.000 Flow length"
"      0.750 Overland Slope"
"      0.208 Pervious Area"
"      60.000 Pervious length"
"      0.750 Pervious slope"
"      0.311 Impervious Area"
"      60.000 Impervious length"
"      0.750 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.419 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.919 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.163 0.000 0.000 0.000 c.m/sec"
"      Catchment 2011 Pervious Impervious Total Area "
"      Surface Area 0.208 0.311 0.519 hectare"
"      Time of concentration 24.435 3.203 8.155 minutes"
"      Time to Centroid 123.623 88.411 96.625 minutes"
"      Rainfall depth 87.263 87.263 87.263 mm"
"      Rainfall volume 181.16 271.74 452.90 c.m"
"      Rainfall losses 50.658 7.047 24.491 mm"
"      Runoff depth 36.605 80.217 62.772 mm"
"      Runoff volume 75.99 249.79 325.79 c.m"
"      Runoff coefficient 0.419 0.919 0.719 "
"      Maximum flow 0.024 0.160 0.163 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.163 0.163 0.000 0.000"

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" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.163 0.163 0.163 0.000"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.163 c.m/sec"
"      Hydrograph volume 325.787 c.m"
"      0.163 0.163 0.163 0.163"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.163 0.000 0.163 0.163"
" 33 CATCHMENT 2012"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2012 Catchment 201-2"
"      70.000 % Impervious"
"      0.066 Total Area"
"      15.000 Flow length"
"      0.500 Overland Slope"
"      0.020 Pervious Area"
"      15.000 Pervious length"
"      0.500 Pervious slope"
"      0.046 Impervious Area"
"      15.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.418 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.925 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.027 0.000 0.163 0.163 c.m/sec"
"      Catchment 2012 Pervious Impervious Total Area "
"      Surface Area 0.020 0.046 0.066 hectare"
"      Time of concentration 12.012 1.574 3.268 minutes"
"      Time to Centroid 108.087 86.019 89.600 minutes"
"      Rainfall depth 87.263 87.263 87.263 mm"
"      Rainfall volume 17.28 40.32 57.59 c.m"
"      Rainfall losses 50.758 6.508 19.783 mm"
"      Runoff depth 36.505 80.755 67.480 mm"
"      Runoff volume 7.23 37.31 44.54 c.m"
"      Runoff coefficient 0.418 0.925 0.773 "
"      Maximum flow 0.003 0.026 0.027 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.027 0.027 0.163 0.163"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.027 0.027 0.027 0.163"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.189 c.m/sec"
"      Hydrograph volume 370.324 c.m"
"      0.027 0.027 0.027 0.189"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"

```

" 0.027 0.000 0.027 0.189"  
" 33 CATCHMENT 2013"  
" 1 Triangular SCS"  
" 1 Equal length"  
" 1 SCS method"  
" 2013 Catchment 201-3"  
" 75.000 % Impervious"  
" 0.234 Total Area"  
" 15.000 Flow length"  
" 2.000 Overland Slope"  
" 0.058 Pervious Area"  
" 15.000 Pervious length"  
" 2.000 Pervious slope"  
" 0.176 Impervious Area"  
" 15.000 Impervious length"  
" 2.000 Impervious slope"  
" 0.250 Pervious Manning 'n'"  
" 74.000 Pervious SCS Curve No."  
" 0.418 Pervious Runoff coefficient"  
" 0.100 Pervious Ia/S coefficient"  
" 8.924 Pervious Initial abstraction"  
" 0.015 Impervious Manning 'n'"  
" 98.000 Impervious SCS Curve No."  
" 0.910 Impervious Runoff coefficient"  
" 0.100 Impervious Ia/S coefficient"  
" 0.518 Impervious Initial abstraction"  
" 0.105 0.000 0.027 0.189 c.m/sec"  
" Catchment 2013 Pervious Impervious Total Area "  
" Surface Area 0.058 0.176 0.234 hectare"  
" Time of concentration 7.925 1.039 1.954 minutes"  
" Time to Centroid 103.037 85.361 87.709 minutes"  
" Rainfall depth 87.263 87.263 87.263 mm"  
" Rainfall volume 51.05 153.15 204.20 c.m"  
" Rainfall losses 50.764 7.853 18.581 mm"  
" Runoff depth 36.499 79.410 68.683 mm"  
" Runoff volume 21.35 139.37 160.72 c.m"  
" Runoff coefficient 0.418 0.910 0.787 "  
" Maximum flow 0.012 0.101 0.105 c.m/sec"  
" 40 HYDROGRAPH Add Runoff "  
" 4 Add Runoff "  
" 0.105 0.105 0.027 0.189"  
" 40 HYDROGRAPH Copy to Outflow"  
" 8 Copy to Outflow"  
" 0.105 0.105 0.105 0.189"  
" 40 HYDROGRAPH Combine 900"  
" 6 Combine "  
" 900 Node #"  
" SWMF"  
" Maximum flow 0.294 c.m/sec"  
" Hydrograph volume 531.041 c.m"  
" 0.105 0.105 0.105 0.294"  
" 40 HYDROGRAPH Start - New Tributary"  
" 2 Start - New Tributary"  
" 0.105 0.000 0.105 0.294"  
" 33 CATCHMENT 2014"  
" 1 Triangular SCS"  
" 1 Equal length"  
" 1 SCS method"  
" 2014 Catchment 201-4"  
" 100.000 % Impervious"  
" 0.065 Total Area"  
" 10.000 Flow length"  
" 2.000 Overland Slope"  
" 0.000 Pervious Area"  
" 10.000 Pervious length"

" 2.000 Pervious slope"  
" 0.065 Impervious Area"  
" 10.000 Impervious length"  
" 2.000 Impervious slope"  
" 0.250 Pervious Manning 'n'"  
" 74.000 Pervious SCS Curve No."  
" 0.000 Pervious Runoff coefficient"  
" 0.100 Pervious Ia/S coefficient"  
" 8.924 Pervious Initial abstraction"  
" 0.015 Impervious Manning 'n'"  
" 98.000 Impervious SCS Curve No."  
" 0.890 Impervious Runoff coefficient"  
" 0.100 Impervious Ia/S coefficient"  
" 0.518 Impervious Initial abstraction"  
" 0.037 0.000 0.105 0.294 c.m/sec"  
" Catchment 2014 Pervious Impervious Total Area "  
" Surface Area 0.000 0.065 0.065 hectare"  
" Time of concentration 6.214 0.814 0.814 minutes"  
" Time to Centroid 100.973 85.085 85.085 minutes"  
" Rainfall depth 87.263 87.263 87.263 mm"  
" Rainfall volume 0.00 56.72 56.72 c.m"  
" Rainfall losses 51.029 9.575 9.575 mm"  
" Runoff depth 36.234 77.688 77.688 mm"  
" Runoff volume 0.00 50.50 50.50 c.m"  
" Runoff coefficient 0.000 0.890 0.890 "  
" Maximum flow 0.000 0.037 0.037 c.m/sec"  
" 40 HYDROGRAPH Add Runoff "  
" 4 Add Runoff "  
" 0.037 0.037 0.105 0.294"  
" 57 TRENCH Design d/s of 2014"  
" 0.037 Peak inflow"  
" 50.497 Hydrograph volume"  
" 337.000 Ground elevation"  
" 334.200 Downstream trench invert"  
" 1.000 Trench height"  
" 333.200 Water table elevation"  
" 5.000 Trench top width"  
" 5.000 Trench bottom width"  
" 40.000 Voids ratio (%) "  
" 30.000 Hydraulic conductivity"  
" 0.000 Trench gradient (%) "  
" 10.000 Trench length"  
" 1.000 Include base width"  
" 29. Number of stages"  
" Level Discharge Volume"  
" 334.200 0.000 0.0"  
" 334.300 0.000 2.0"  
" 334.400 0.000 4.0"  
" 334.500 0.000 6.0"  
" 334.600 0.000 8.0"  
" 334.700 0.000 10.0"  
" 334.800 0.000 12.0"  
" 334.900 0.000 14.0"  
" 335.000 0.000 16.0"  
" 335.100 0.000 18.0"  
" 335.200 0.000 20.0"  
" 335.300 0.004 20.1"  
" 335.400 0.006 20.2"  
" 335.500 0.008 20.3"  
" 335.600 0.010 20.5"  
" 335.700 0.011 20.6"  
" 335.800 0.013 20.7"  
" 335.900 0.014 20.8"  
" 336.000 0.015 20.9"  
" 336.100 0.016 21.0"

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"          336.200      0.017      21.1"
"          336.300      0.018      21.2"
"          336.400      0.019      21.4"
"          336.500      0.020      21.5"
"          336.600      0.020      21.6"
"          336.700      0.021      21.7"
"          336.800      0.022      21.8"
"          336.900      0.023      21.9"
"          337.000      0.023      22.0"
"
" 1. TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      20.000      0.000      1.000      1.000      0.000"
"
" 1. MANHOLE"
"      Access"
"      diameter"
"      1.200"
"
" 1. OUTFLOW PIPE"
" 0. Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      20.000      0.100      0.013      0.500"
"
"      Peak outflow      0.023      c.m/sec"
"      Outflow volume      22.935      c.m"
"      Peak exfiltration      0.001      c.m/sec"
"      Exfiltration volume      26.845      c.m"
"      Maximum level      336.908      metre"
"      Maximum storage      21.932      c.m"
"      Centroidal lag      1.536      hours"
"      Infiltration area 2 sides      20.000      sq.metre"
"      Infiltration Base area      50.000      sq.metre"
"      0.037      0.037      0.023      0.001 c.m/sec"
"
" 40 HYDROGRAPH Combine 900"
"
" 6 Combine "
"
" 900 Node #"
"      SWMF"
"      Maximum flow      0.294      c.m/sec"
"      Hydrograph volume      553.977      c.m"
"      0.037      0.037      0.023      0.294"
"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
"      0.037      0.000      0.023      0.294"
"
" 33 CATCHMENT 2015"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
"
" 2015 Catchment 201-5"
" 85.000 % Impervious"
" 0.074 Total Area"
" 15.000 Flow length"
" 2.000 Overland Slope"
" 0.011 Pervious Area"
" 15.000 Pervious length"
" 2.000 Pervious slope"
" 0.063 Impervious Area"
" 15.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.418 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.910 Impervious Runoff coefficient"

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" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
"      0.037      0.000      0.023      0.294 c.m/sec"
"
"      Catchment 2015      Pervious      Impervious Total Area "
"      Surface Area      0.011      0.063      0.074      hectare"
"      Time of concentration      7.925      1.039      1.555      minutes"
"      Time to Centroid      103.037      85.361      86.687      minutes"
"      Rainfall depth      87.263      87.263      87.263      mm"
"      Rainfall volume      9.69      54.89      64.57      c.m"
"      Rainfall losses      50.764      7.853      14.290      mm"
"      Runoff depth      36.499      79.410      72.974      mm"
"      Runoff volume      4.05      49.95      54.00      c.m"
"      Runoff coefficient      0.418      0.910      0.836      "
"      Maximum flow      0.002      0.036      0.037      c.m/sec"
"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
"      0.037      0.037      0.023      0.294"
"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
"      0.037      0.037      0.037      0.294"
"
" 40 HYDROGRAPH Combine 900"
" 6 Combine "
"
" 900 Node #"
"      SWMF"
"      Maximum flow      0.331      c.m/sec"
"      Hydrograph volume      607.975      c.m"
"      0.037      0.037      0.037      0.331"
"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
"      0.037      0.000      0.037      0.331"
"
" 33 CATCHMENT 2016"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
"
" 2016 Catchment 201-6"
" 100.000 % Impervious"
" 0.032 Total Area"
" 10.000 Flow length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 10.000 Pervious length"
" 2.000 Pervious slope"
" 0.032 Impervious Area"
" 10.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.000 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.890 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
"      0.018      0.000      0.037      0.331 c.m/sec"
"
"      Catchment 2016      Pervious      Impervious Total Area "
"      Surface Area      0.000      0.032      0.032      hectare"
"      Time of concentration      6.214      0.814      0.814      minutes"
"      Time to Centroid      100.973      85.085      85.085      minutes"
"      Rainfall depth      87.263      87.263      87.263      mm"
"      Rainfall volume      0.00      27.92      27.92      c.m"
"      Rainfall losses      51.029      9.575      9.575      mm"
"      Runoff depth      36.234      77.688      77.688      mm"
"      Runoff volume      0.00      24.86      24.86      c.m"

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"      Runoff coefficient      0.000      0.890      0.890      "
"      Maximum flow            0.000      0.018      0.018      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.018      0.018      0.037      0.331"
" 57  TRENCH Design d/s of 2016"
"      0.018  Peak inflow"
"      24.860 Hydrograph volume"
"      337.000 Ground elevation"
"      334.200 Downstream trench invert"
"      1.000  Trench height"
"      333.200 Water table elevation"
"      5.000  Trench top width"
"      5.000  Trench bottom width"
"      40.000 Voids ratio (%)"
"      30.000 Hydraulic conductivity"
"      0.000  Trench gradient (%)"
"      5.000  Trench length"
"      1.000  Include base width"
"      29.  Number of stages"
"          Level Discharge      Volume"
"          334.200      0.000      0.0"
"          334.300      0.000      1.0"
"          334.400      0.000      2.0"
"          334.500      0.000      3.0"
"          334.600      0.000      4.0"
"          334.700      0.000      5.0"
"          334.800      0.000      6.0"
"          334.900      0.000      7.0"
"          335.000      0.000      8.0"
"          335.100      0.000      9.0"
"          335.200      0.000      10.0"
"          335.300      0.002      10.1"
"          335.400      0.006      10.2"
"          335.500      0.008      10.3"
"          335.600      0.010      10.5"
"          335.700      0.011      10.6"
"          335.800      0.013      10.7"
"          335.900      0.014      10.8"
"          336.000      0.015      10.9"
"          336.100      0.016      11.0"
"          336.200      0.017      11.1"
"          336.300      0.018      11.2"
"          336.400      0.019      11.4"
"          336.500      0.020      11.5"
"          336.600      0.020      11.6"
"          336.700      0.021      11.7"
"          336.800      0.022      11.8"
"          336.900      0.023      11.9"
"          337.000      0.023      12.0"
" 1.  TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.200      10.000      0.000      2.000      1.000      0.000"
" 1.  MANHOLE"
"      Access"
"      diameter"
"      1.200"
" 1.  OUTFLOW PIPE"
" 0.  Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.200      335.000      10.000      0.100      0.013      0.500"
"      Peak outflow      0.012      c.m/sec"
"      Outflow volume      11.156      c.m"

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"      Peak exfiltration      0.001      c.m/sec"
"      Exfiltration volume      13.333      c.m"
"      Maximum level      335.775      metre"
"      Maximum storage      10.650      c.m"
"      Centroidal lag      1.535      hours"
"      Infiltration area 2 sides      10.000      sq.metre"
"      Infiltration Base area      25.000      sq.metre"
"          0.018      0.018      0.012      0.001 c.m/sec"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"
"      Maximum flow      0.331      c.m/sec"
"      Hydrograph volume      619.134      c.m"
"          0.018      0.018      0.012      0.331"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.018      0.000      0.012      0.331"
" 33  CATCHMENT 2017"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      2017  Catchment 201-7 - SWMF"
"      40.000 % Impervious"
"      0.211  Total Area"
"      10.000 Flow length"
"      10.000 Overland Slope"
"      0.127  Pervious Area"
"      10.000 Pervious length"
"      10.000 Pervious slope"
"      0.084  Impervious Area"
"      10.000 Impervious length"
"      10.000 Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.412  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.838  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.068      0.000      0.012      0.331 c.m/sec"
"      Catchment 2017      Pervious      Impervious      Total Area "
"      Surface Area      0.127      0.084      0.211      hectare"
"      Time of concentration      3.834      0.502      1.915      minutes"
"      Time to Centroid      98.024      84.924      90.480      minutes"
"      Rainfall depth      87.263      87.263      87.263      mm"
"      Rainfall volume      110.48      73.65      184.13      c.m"
"      Rainfall losses      51.338      14.098      36.442      mm"
"      Runoff depth      35.925      73.165      50.821      mm"
"      Runoff volume      45.48      61.75      107.23      c.m"
"      Runoff coefficient      0.412      0.838      0.582      "
"      Maximum flow      0.031      0.047      0.068      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.068      0.068      0.012      0.331"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.068      0.068      0.068      0.331"
" 40  HYDROGRAPH Combine      900"
"      6  Combine "
"      900  Node #"
"      SWMF"

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"      Maximum flow      0.399  c.m/sec"
"      Hydrograph volume 726.367 c.m"
"      0.068      0.068  0.068  0.399"
" 40      HYDROGRAPH Confluence 900"
"      7 Confluence "
"      900 Node #"
"      SWMF"
"      Maximum flow      0.399  c.m/sec"
"      Hydrograph volume 726.367 c.m"
"      0.068      0.399  0.068  0.000"
" 54      POND DESIGN"
"      0.399 Current peak flow c.m/sec"
"      0.045 Target outflow c.m/sec"
"      726.4 Hydrograph volume c.m"
"      13. Number of stages"
"      334.400 Minimum water level metre"
"      335.600 Maximum water level metre"
"      334.400 Starting water level metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"      Level Discharge Volume"
"      334.400 0.000 0.000"
"      334.500 0.00150 40.000"
"      334.600 0.00230 85.000"
"      334.700 0.00290 135.000"
"      334.800 0.03110 189.000"
"      334.900 0.04300 249.000"
"      335.000 0.05220 314.000"
"      335.100 0.05990 384.000"
"      335.200 0.06680 457.000"
"      335.300 0.07300 534.000"
"      335.400 0.1628 614.000"
"      335.500 0.4275 698.000"
"      335.600 0.9140 786.000"
"      Peak outflow      0.067  c.m/sec"
"      Maximum level      335.210 metre"
"      Maximum storage      464.298 c.m"
"      Centroidal lag      4.845 hours"
"      0.068 0.399 0.067 0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5 Next link "
"      0.068 0.067 0.067 0.000"
" 54      POND DESIGN"
"      0.067 Current peak flow c.m/sec"
"      0.045 Target outflow c.m/sec"
"      712.8 Hydrograph volume c.m"
"      10. Number of stages"
"      334.200 Minimum water level metre"
"      335.100 Maximum water level metre"
"      334.200 Starting water level metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"      Level Discharge Volume"
"      334.200 0.000 0.000"
"      334.300 0.00191 18.000"
"      334.400 0.00207 38.000"
"      334.500 0.00225 60.000"
"      334.600 0.00243 83.000"
"      334.700 0.00261 109.000"
"      334.800 0.00281 136.000"
"      334.900 0.1543 165.000"
"      335.000 0.4628 196.000"
"      335.100 0.9060 229.000"
"      Peak outflow      0.066  c.m/sec"
"      Maximum level      334.842 metre"
"      Maximum storage      148.057 c.m"
"      Centroidal lag      8.722 hours"

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"      0.068 0.067 0.066 0.000 c.m/sec"
" 40      HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.066  c.m/sec"
"      Hydrograph volume 645.849 c.m"
"      0.068 0.067 0.066 0.066"
" 40      HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.068 0.000 0.066 0.066"
" 33      CATCHMENT 2021"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2021 Catchment 202-1"
"      0.000 % Impervious"
"      0.863 Total Area"
"      50.000 Flow length"
"      0.500 Overland Slope"
"      0.863 Pervious Area"
"      50.000 Pervious length"
"      0.500 Pervious slope"
"      0.000 Impervious Area"
"      50.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.419 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.101 0.000 0.066 0.066 c.m/sec"
"      Catchment 2021 Pervious Impervious Total Area "
"      Surface Area 0.863 0.000 0.863 hectare"
"      Time of concentration 24.737 3.242 24.736 minutes"
"      Time to Centroid 124.000 88.466 124.000 minutes"
"      Rainfall depth 87.263 87.263 87.263 mm"
"      Rainfall volume 753.08 0.00 753.08 c.m"
"      Rainfall losses 50.668 7.034 50.668 mm"
"      Runoff depth 36.595 80.229 36.595 mm"
"      Runoff volume 315.82 0.00 315.82 c.m"
"      Runoff coefficient 0.419 0.000 0.419 "
"      Maximum flow 0.101 0.000 0.101 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.101 0.101 0.066 0.066"
" 40      HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.101 0.101 0.101 0.066"
" 40      HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.111  c.m/sec"
"      Hydrograph volume 961.668 c.m"
"      0.101 0.101 0.101 0.111"
" 40      HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.101 0.000 0.101 0.111"
" 33      CATCHMENT 2022"

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"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2022 Catchment 202-2"
" 24.000 % Impervious"
"      0.184 Total Area"
" 10.000 Flow length"
"      2.000 Overland Slope"
"      0.140 Pervious Area"
" 10.000 Pervious length"
"      2.000 Pervious slope"
"      0.044 Impervious Area"
" 10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
"      0.415 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
"      0.890 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.046      0.000      0.101      0.111 c.m/sec"
"      Catchment 2022      Pervious      Impervious Total Area "
"      Surface Area      0.140      0.044      0.184      hectare"
"      Time of concentration      6.214      0.814      4.034      minutes"
"      Time to Centroid      100.973      85.085      94.559      minutes"
"      Rainfall depth      87.263      87.263      87.263      mm"
"      Rainfall volume      122.03      38.54      160.56      c.m"
"      Rainfall losses      51.029      9.575      41.080      mm"
"      Runoff depth      36.234      77.688      46.183      mm"
"      Runoff volume      50.67      34.31      84.98      c.m"
"      Runoff coefficient      0.415      0.890      0.529      "
"      Maximum flow      0.030      0.025      0.046      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.046      0.046      0.101      0.111"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.046      0.046      0.046      0.111"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
" 800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.120      c.m/sec"
"      Hydrograph volume      1046.645      c.m"
"      0.046      0.046      0.046      0.120"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.046      0.000      0.046      0.120"
" 33 CATCHMENT 2031"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2031 Catchment 203-1"
" 30.000 % Impervious"
"      0.216 Total Area"
" 130.000 Flow length"
"      2.000 Overland Slope"
"      0.151 Pervious Area"
" 130.000 Pervious length"
"      2.000 Pervious slope"
"      0.065 Impervious Area"

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" 130.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.419 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.921 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.038      0.000      0.046      0.120 c.m/sec"
"      Catchment 2031      Pervious      Impervious Total Area "
"      Surface Area      0.151      0.065      0.216      hectare"
"      Time of concentration      28.954      3.795      16.752      minutes"
"      Time to Centroid      129.260      89.264      109.862      minutes"
"      Rainfall depth      87.263      87.263      87.263      mm"
"      Rainfall volume      131.94      56.55      188.49      c.m"
"      Rainfall losses      50.682      6.880      37.542      mm"
"      Runoff depth      36.581      80.383      49.722      mm"
"      Runoff volume      55.31      52.09      107.40      c.m"
"      Runoff coefficient      0.419      0.921      0.570      "
"      Maximum flow      0.016      0.033      0.038      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.038      0.038      0.046      0.120"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.038      0.038      0.038      0.120"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
" 700 Node #"
"      CULVERT"
"      Maximum flow      0.038      c.m/sec"
"      Hydrograph volume      107.399      c.m"
"      0.038      0.038      0.038      0.038"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.038      0.000      0.038      0.038"
" 33 CATCHMENT 2032"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2032 Catchment 203-2"
"      0.000 % Impervious"
"      0.113 Total Area"
"      10.000 Flow length"
"      33.000 Overland Slope"
"      0.113 Pervious Area"
"      10.000 Pervious length"
"      33.000 Pervious slope"
"      0.000 Impervious Area"
"      10.000 Impervious length"
"      33.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.412 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"

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"      0.027      0.000      0.038      0.038 c.m/sec"
"      Catchment 2032      Pervious      Impervious      Total Area      "
"      Surface Area      0.113      0.000      0.113      hectare"
"      Time of concentration      2.680      0.351      2.680      minutes"
"      Time to Centroid      96.408      84.507      96.408      minutes"
"      Rainfall depth      87.263      87.263      87.263      mm"
"      Rainfall volume      98.61      0.00      98.61      c.m"
"      Rainfall losses      51.352      16.110      51.352      mm"
"      Runoff depth      35.911      71.153      35.911      mm"
"      Runoff volume      40.58      0.00      40.58      c.m"
"      Runoff coefficient      0.412      0.000      0.412      "
"      Maximum flow      0.027      0.000      0.027      c.m/sec"
" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.027      0.027      0.038      0.038"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.027      0.027      0.027      0.038"
" 40      HYDROGRAPH Combine      700"
"      6      Combine      "
"      700      Node #      "
"      CULVERT"
"      Maximum flow      0.065      c.m/sec"
"      Hydrograph volume      147.979      c.m"
"      0.027      0.027      0.027      0.065"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.027      0.000      0.027      0.065"
" 33      CATCHMENT 2033"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      2033      Catchment 203-3"
"      100.000      % Impervious"
"      0.092      Total Area"
"      10.000      Flow length"
"      2.000      Overland Slope"
"      0.000      Pervious Area"
"      10.000      Pervious length"
"      2.000      Pervious slope"
"      0.092      Impervious Area"
"      10.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      74.000      Pervious SCS Curve No."
"      0.000      Pervious Runoff coefficient"
"      0.100      Pervious Ia/S coefficient"
"      8.924      Pervious Initial abstraction"
"      0.015      Impervious Manning 'n'"
"      98.000      Impervious SCS Curve No."
"      0.890      Impervious Runoff coefficient"
"      0.100      Impervious Ia/S coefficient"
"      0.518      Impervious Initial abstraction"
"      0.053      0.000      0.027      0.065 c.m/sec"
"      Catchment 2033      Pervious      Impervious      Total Area      "
"      Surface Area      0.000      0.092      0.092      hectare"
"      Time of concentration      6.214      0.814      0.814      minutes"
"      Time to Centroid      100.973      85.085      85.085      minutes"
"      Rainfall depth      87.263      87.263      87.263      mm"
"      Rainfall volume      0.00      80.28      80.28      c.m"
"      Rainfall losses      51.029      9.575      9.575      mm"
"      Runoff depth      36.234      77.688      77.688      mm"
"      Runoff volume      0.00      71.47      71.47      c.m"
"      Runoff coefficient      0.000      0.890      0.890      "
"      Maximum flow      0.000      0.053      0.053      c.m/sec"

```

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" 40      HYDROGRAPH Add Runoff      "
"      4      Add Runoff      "
"      0.053      0.053      0.027      0.065"
" 57      TRENCH Design d/s of 2033"
"      0.053      Peak inflow"
"      71.473      Hydrograph volume"
"      337.000      Ground elevation"
"      334.500      Downstream trench invert"
"      1.000      Trench height"
"      333.500      Water table elevation"
"      1.000      Trench top width"
"      1.000      Trench bottom width"
"      40.000      Voids ratio (%)"
"      30.000      Hydraulic conductivity"
"      0.000      Trench gradient (%)"
"      60.000      Trench length"
"      1.000      Include base width"
"      26.      Number of stages"
"      Level Discharge      Volume"
"      334.500      0.000      0.0"
"      334.600      0.000      2.4"
"      334.700      0.000      4.8"
"      334.800      0.000      7.2"
"      334.900      0.000      9.6"
"      335.000      0.000      12.0"
"      335.100      0.000      14.4"
"      335.200      0.000      16.8"
"      335.300      0.000      19.2"
"      335.400      0.000      21.6"
"      335.500      0.000      24.0"
"      335.600      0.005      24.1"
"      335.700      0.010      24.2"
"      335.800      0.016      24.3"
"      335.900      0.020      24.5"
"      336.000      0.024      24.6"
"      336.100      0.027      24.7"
"      336.200      0.030      24.8"
"      336.300      0.032      24.9"
"      336.400      0.035      25.0"
"      336.500      0.037      25.1"
"      336.600      0.039      25.2"
"      336.700      0.041      25.4"
"      336.800      0.043      25.5"
"      336.900      0.045      25.6"
"      337.000      0.047      25.7"
"      1.      TRENCH PIPES"
"      Downstream      Pipe      Pipe      Pipe Perf'ted?      Offset"
"      Invert      length      diam.      grade%      0=Yes      distance"
"      335.500      20.000      0.000      1.000      1.000      0.000"
"      1.      MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1.      OUTFLOW PIPE"
"      0.      Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm      Pipe      Pipe      Manning      Entry"
"      invert      invert      Length      Diameter      'n'      loss Ke"
"      335.500      335.300      20.000      0.150      0.013      0.500"
"      Peak outflow      0.033      c.m/sec"
"      Outflow volume      27.188      c.m"
"      Peak exfiltration      0.003      c.m/sec"
"      Exfiltration volume      38.435      c.m"
"      Maximum level      336.730      metre"
"      Maximum storage      25.391      c.m"
"      Centroidal lag      1.483      hours"

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"      Infiltration area 2 sides 120.000 sq.metre"
"      Infiltration Base area 60.000 sq.metre"
"      0.053 0.053 0.033 0.003 c.m/sec"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.098 c.m/sec"
"      Hydrograph volume 175.167 c.m"
"      0.053 0.053 0.033 0.098"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.053 0.000 0.033 0.098"
" 33 CATCHMENT 204"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      204 Catchment 204"
"      0.000 % Impervious"
"      0.198 Total Area"
"      8.000 Flow length"
"      20.000 Overland Slope"
"      0.198 Pervious Area"
"      8.000 Pervious length"
"      20.000 Pervious slope"
"      0.000 Impervious Area"
"      8.000 Impervious length"
"      20.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.411 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.048 0.000 0.033 0.098 c.m/sec"
"      Catchment 204 Pervious Impervious Total Area "
"      Surface Area 0.198 0.000 0.198 hectare"
"      Time of concentration 2.724 0.357 2.724 minutes"
"      Time to Centroid 96.470 84.532 96.470 minutes"
"      Rainfall depth 87.263 87.263 87.263 mm"
"      Rainfall volume 172.78 0.00 172.78 c.m"
"      Rainfall losses 51.380 16.052 51.380 mm"
"      Runoff depth 35.883 71.212 35.883 mm"
"      Runoff volume 71.05 0.00 71.05 c.m"
"      Runoff coefficient 0.411 0.000 0.411 "
"      Maximum flow 0.048 0.000 0.048 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.048 0.048 0.033 0.098"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.048 0.048 0.048 0.098"
" 40 HYDROGRAPH Combine 700"
"      6 Combine "
"      700 Node #"
"      CULVERT"
"      Maximum flow 0.146 c.m/sec"
"      Hydrograph volume 246.216 c.m"
"      0.048 0.048 0.048 0.146"
" 40 HYDROGRAPH Confluence 700"
"      7 Confluence "

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"      700 Node #"
"      CULVERT"
"      Maximum flow 0.146 c.m/sec"
"      Hydrograph volume 246.216 c.m"
"      0.048 0.146 0.048 0.000"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.048 0.146 0.146 0.000"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow 0.225 c.m/sec"
"      Hydrograph volume 1292.862 c.m"
"      0.048 0.146 0.146 0.225"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.048 0.000 0.146 0.225"
" 33 CATCHMENT 2051"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2051 Catchment 205-1"
"      25.000 % Impervious"
"      0.098 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.073 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.024 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.418 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.910 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.024 0.000 0.146 0.225 c.m/sec"
"      Catchment 2051 Pervious Impervious Total Area "
"      Surface Area 0.073 0.024 0.098 hectare"
"      Time of concentration 7.925 1.039 5.030 minutes"
"      Time to Centroid 103.037 85.361 95.607 minutes"
"      Rainfall depth 87.263 87.263 87.263 mm"
"      Rainfall volume 64.14 21.38 85.52 c.m"
"      Rainfall losses 50.764 7.853 40.036 mm"
"      Runoff depth 36.499 79.410 47.227 mm"
"      Runoff volume 26.83 19.46 46.28 c.m"
"      Runoff coefficient 0.418 0.910 0.541 "
"      Maximum flow 0.015 0.014 0.024 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.024 0.024 0.146 0.225"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.024 0.024 0.024 0.225"
" 40 HYDROGRAPH Combine 600"
"      6 Combine "
"      600 Node #"
"      Arkell Rd"

```

" Maximum flow 0.024 c.m/sec"

" Hydrograph volume 46.282 c.m"

" 0.024 0.024 0.024 0.024"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.024 0.000 0.024 0.024"

" 33 CATCHMENT 2052"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2052 Catchment 205-2"

" 40.000 % Impervious"

" 0.108 Total Area"

" 25.000 Flow length"

" 5.000 Overland Slope"

" 0.065 Pervious Area"

" 25.000 Pervious length"

" 5.000 Pervious slope"

" 0.043 Impervious Area"

" 25.000 Impervious length"

" 5.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.418 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.912 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.030 0.000 0.024 0.024 c.m/sec"

" Catchment 2052 Pervious Impervious Total Area "

" Surface Area 0.065 0.043 0.108 hectare"

" Time of concentration 8.179 1.072 3.969 minutes"

" Time to Centroid 103.355 85.398 92.718 minutes"

" Rainfall depth 87.263 87.263 87.263 mm"

" Rainfall volume 56.55 37.70 94.24 c.m"

" Rainfall losses 50.750 7.675 33.520 mm"

" Runoff depth 36.513 79.588 53.743 mm"

" Runoff volume 23.66 34.38 58.04 c.m"

" Runoff coefficient 0.418 0.912 0.616 "

" Maximum flow 0.013 0.025 0.030 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.030 0.030 0.024 0.024"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.030 0.030 0.030 0.024"

" 40 HYDROGRAPH Combine 600"

" 6 Combine "

" 600 Node #"

" Arkell Rd"

" Maximum flow 0.055 c.m/sec"

" Hydrograph volume 104.325 c.m"

" 0.030 0.030 0.030 0.055"

" 40 HYDROGRAPH Confluence 600"

" 7 Confluence "

" 600 Node #"

" Arkell Rd"

" Maximum flow 0.055 c.m/sec"

" Hydrograph volume 104.325 c.m"

" 0.030 0.055 0.030 0.000"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.030 0.055 0.055 0.000"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.280 c.m/sec"

" Hydrograph volume 1397.186 c.m"

" 0.030 0.055 0.055 0.280"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.030 0.000 0.055 0.280"

" 33 CATCHMENT 206"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 206 Catchment 206"

" 70.000 % Impervious"

" 0.035 Total Area"

" 20.000 Flow length"

" 1.250 Overland Slope"

" 0.011 Pervious Area"

" 20.000 Pervious length"

" 1.250 Pervious slope"

" 0.024 Impervious Area"

" 20.000 Impervious length"

" 1.250 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 74.000 Pervious SCS Curve No."

" 0.419 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n'"

" 98.000 Impervious SCS Curve No."

" 0.924 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.015 0.000 0.055 0.280 c.m/sec"

" Catchment 206 Pervious Impervious Total Area "

" Surface Area 0.011 0.024 0.035 hectare"

" Time of concentration 10.844 1.421 2.954 minutes"

" Time to Centroid 106.618 85.803 89.189 minutes"

" Rainfall depth 87.263 87.263 87.263 mm"

" Rainfall volume 9.16 21.38 30.54 c.m"

" Rainfall losses 50.713 6.652 19.870 mm"

" Runoff depth 36.551 80.611 67.393 mm"

" Runoff volume 3.84 19.75 23.59 c.m"

" Runoff coefficient 0.419 0.924 0.772 "

" Maximum flow 0.002 0.014 0.015 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.015 0.015 0.055 0.280"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.015 0.015 0.015 0.280"

" 40 HYDROGRAPH Combine 800"

" 6 Combine "

" 800 Node #"

" TORRANCE CREEK"

" Maximum flow 0.292 c.m/sec"

" Hydrograph volume 1420.774 c.m"

" 0.015 0.015 0.015 0.292"

" 40 HYDROGRAPH Confluence 800"

" 7 Confluence "

" 800 Node #"

" TORRANCE CREEK"

```

"      Maximum flow          0.292    c.m/sec"
"      Hydrograph volume    1420.774  c.m"
"      0.015      0.292    0.015    0.000"
" 38  START/RE-START TOTALS 800"
"      3  Runoff Totals on EXIT"
"      Total Catchment area      3.108  hectare"
"      Total Impervious area      1.071  hectare"
"      Total % impervious      34.445"
" 19  EXIT"

```

```

"      MIDUSS Output ----->"
"      MIDUSS version          Version 2.25 rev. 473"
"      MIDUSS created          Sunday, February 7, 2010"
"      10 Units used:          ie METRIC"
"      Job folder:              Q:\42063\104\SWM\September 2021\MIDUSS\
"                                  POST"
"      Output filename:          RegPOST.in"
"      Licensee name:            A"
"      Company                  Microsoft"
"      Date & Time last used:    10/5/2021 at 2:23:30 PM"
" 31  TIME PARAMETERS"
"      5.000 Time Step"
"      2880.000 Max. Storm length"
"      9000.000 Max. Hydrograph"
" 32  STORM Mass Curve"
"      3 Mass Curve"
"      285.000 Rainfall depth"
"      2880.000 Duration"
"      69 Q:\42063\104\SWM\September 2021\MIDUSS\POST\Hazel entire 48 hours.mrd Hurricane
Hazel (entire 48 h)"
"      Maximum intensity          53.012  mm/hr"
"      Total depth                285.000  mm"
"      8 99999hyd Hydrograph extension used in this file"
" 33  CATCHMENT 2011"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2011 Catchment 201-1"
"      60.000 % Impervious"
"      0.519 Total Area"
"      60.000 Flow length"
"      0.750 Overland Slope"
"      0.208 Pervious Area"
"      60.000 Pervious length"
"      0.750 Pervious slope"
"      0.311 Impervious Area"
"      60.000 Impervious length"
"      0.750 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.732 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.972 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.075 0.000 0.000 0.000 c.m/sec"
"      Catchment 2011 Pervious Impervious Total Area "
"      Surface Area 0.208 0.311 0.519 hectare"
"      Time of concentration 32.556 5.818 14.755 minutes"
"      Time to Centroid 2531.753 2271.613 2358.566 minutes"
"      Rainfall depth 285.000 285.000 285.000 mm"
"      Rainfall volume 591.66 887.49 1479.15 c.m"
"      Rainfall losses 76.442 8.071 35.419 mm"
"      Runoff depth 208.558 276.929 249.581 mm"
"      Runoff volume 432.97 862.36 1295.32 c.m"
"      Runoff coefficient 0.732 0.972 0.876 "
"      Maximum flow 0.030 0.048 0.075 c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.075 0.075 0.000 0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"

```

" 0.075 0.075 0.075 0.000"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.075 c.m/sec"

" Hydrograph volume 1295.324 c.m"

" 0.075 0.075 0.075 0.075"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.075 0.000 0.075 0.075"

" 33 CATCHMENT 2012"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2012 Catchment 201-2"

" 70.000 % Impervious"

" 0.066 Total Area"

" 15.000 Flow length"

" 0.500 Overland Slope"

" 0.020 Pervious Area"

" 15.000 Pervious length"

" 0.500 Pervious slope"

" 0.046 Impervious Area"

" 15.000 Impervious length"

" 0.500 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.730 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.965 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.009 0.000 0.075 0.075 c.m/sec"

" Catchment 2012 Pervious Impervious Total Area "

" Surface Area 0.020 0.046 0.066 hectare"

" Time of concentration 16.004 2.860 6.077 minutes"

" Time to Centroid 2507.937 2265.585 2324.907 minutes"

" Rainfall depth 285.000 285.000 285.000 mm"

" Rainfall volume 56.43 131.67 188.10 c.m"

" Rainfall losses 76.941 9.886 30.002 mm"

" Runoff depth 208.059 275.114 254.998 mm"

" Runoff volume 41.20 127.10 168.30 c.m"

" Runoff coefficient 0.730 0.965 0.895 "

" Maximum flow 0.003 0.007 0.009 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.009 0.009 0.075 0.075"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.009 0.009 0.009 0.075"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.084 c.m/sec"

" Hydrograph volume 1463.622 c.m"

" 0.009 0.009 0.009 0.084"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.009 0.000 0.009 0.084"

" 33 CATCHMENT 2013"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2013 Catchment 201-3"

" 75.000 % Impervious"

" 0.234 Total Area"

" 15.000 Flow length"

" 2.000 Overland Slope"

" 0.058 Pervious Area"

" 15.000 Pervious length"

" 2.000 Pervious slope"

" 0.176 Impervious Area"

" 15.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n' "

" 74.000 Pervious SCS Curve No."

" 0.730 Pervious Runoff coefficient"

" 0.100 Pervious Ia/S coefficient"

" 8.924 Pervious Initial abstraction"

" 0.015 Impervious Manning 'n' "

" 98.000 Impervious SCS Curve No."

" 0.963 Impervious Runoff coefficient"

" 0.100 Impervious Ia/S coefficient"

" 0.518 Impervious Initial abstraction"

" 0.034 0.000 0.009 0.084 c.m/sec"

" Catchment 2013 Pervious Impervious Total Area "

" Surface Area 0.058 0.176 0.234 hectare"

" Time of concentration 10.558 1.887 3.637 minutes"

" Time to Centroid 2500.416 2269.665 2316.233 minutes"

" Rainfall depth 285.000 285.000 285.000 mm"

" Rainfall volume 166.72 500.18 666.90 c.m"

" Rainfall losses 76.931 10.683 27.245 mm"

" Runoff depth 208.069 274.317 257.755 mm"

" Runoff volume 121.72 481.43 603.15 c.m"

" Runoff coefficient 0.730 0.963 0.904 "

" Maximum flow 0.008 0.026 0.034 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.034 0.034 0.009 0.084"

" 40 HYDROGRAPH Copy to Outflow"

" 8 Copy to Outflow"

" 0.034 0.034 0.034 0.084"

" 40 HYDROGRAPH Combine 900"

" 6 Combine "

" 900 Node #"

" SWMF"

" Maximum flow 0.118 c.m/sec"

" Hydrograph volume 2066.769 c.m"

" 0.034 0.034 0.034 0.118"

" 40 HYDROGRAPH Start - New Tributary"

" 2 Start - New Tributary"

" 0.034 0.000 0.034 0.118"

" 33 CATCHMENT 2014"

" 1 Triangular SCS"

" 1 Equal length"

" 1 SCS method"

" 2014 Catchment 201-4"

" 100.000 % Impervious"

" 0.065 Total Area"

" 10.000 Flow length"

" 2.000 Overland Slope"

" 0.000 Pervious Area"

" 10.000 Pervious length"

" 2.000 Pervious slope"

" 0.065 Impervious Area"

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"      10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.000 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.970 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.009 0.000 0.034 0.118 c.m/sec"
"      Catchment 2014 Pervious Impervious Total Area "
"      Surface Area 0.000 0.065 0.065 hectare"
"      Time of concentration 8.278 1.479 1.479 minutes"
"      Time to Centroid 2497.307 2266.252 2266.252 minutes"
"      Rainfall depth 285.000 285.000 285.000 mm"
"      Rainfall volume 0.00 185.25 185.25 c.m"
"      Rainfall losses 76.932 8.442 8.442 mm"
"      Runoff depth 208.068 276.558 276.558 mm"
"      Runoff volume 0.00 179.76 179.76 c.m"
"      Runoff coefficient 0.000 0.970 0.970 "
"      Maximum flow 0.000 0.009 0.009 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.009 0.009 0.034 0.118"
" 57 TRENCH Design d/s of 2014"
"      0.009 Peak inflow"
"      179.762 Hydrograph volume"
"      337.000 Ground elevation"
"      334.200 Downstream trench invert"
"      1.000 Trench height"
"      333.200 Water table elevation"
"      5.000 Trench top width"
"      5.000 Trench bottom width"
"      40.000 Voids ratio (%)"
"      30.000 Hydraulic conductivity"
"      0.000 Trench gradient (%)"
"      10.000 Trench length"
"      1.000 Include base width"
"      29. Number of stages"
"      Level Discharge Volume"
"      334.200 0.000 0.0"
"      334.300 0.000 2.0"
"      334.400 0.000 4.0"
"      334.500 0.000 6.0"
"      334.600 0.000 8.0"
"      334.700 0.000 10.0"
"      334.800 0.000 12.0"
"      334.900 0.000 14.0"
"      335.000 0.000 16.0"
"      335.100 0.000 18.0"
"      335.200 0.000 20.0"
"      335.300 0.004 20.1"
"      335.400 0.006 20.2"
"      335.500 0.008 20.3"
"      335.600 0.010 20.5"
"      335.700 0.011 20.6"
"      335.800 0.013 20.7"
"      335.900 0.014 20.8"
"      336.000 0.015 20.9"
"      336.100 0.016 21.0"
"      336.200 0.017 21.1"
"      336.300 0.018 21.2"

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"      336.400 0.019 21.4"
"      336.500 0.020 21.5"
"      336.600 0.020 21.6"
"      336.700 0.021 21.7"
"      336.800 0.022 21.8"
"      336.900 0.023 21.9"
"      337.000 0.023 22.0"
"      1. TRENCH PIPES"
"      Downstream Pipe Pipe Pipe Perf'ted? Offset"
"      Invert length diam. grade% 0=Yes distance"
"      335.200 20.000 0.000 1.000 1.000 0.000"
"      1. MANHOLE"
"      Access"
"      diameter"
"      1.200"
"      1. OUTFLOW PIPE"
"      0. Inflow at upstream end of trench: 1=True; 0=False"
"      Upstream Downstr'm Pipe Pipe Manning Entry"
"      invert invert Length Diameter 'n' loss Ke"
"      335.200 335.000 20.000 0.100 0.013 0.500"
"      Peak outflow 0.008 c.m/sec"
"      Outflow volume 83.978 c.m"
"      Peak exfiltration 0.001 c.m/sec"
"      Exfiltration volume 95.573 c.m"
"      Maximum level 335.523 metre"
"      Maximum storage 20.365 c.m"
"      Centroidal lag 45.082 hours"
"      Infiltration area 2 sides 20.000 sq.metre"
"      Infiltration Base area 50.000 sq.metre"
"      0.009 0.009 0.008 0.001 c.m/sec"
" 40 HYDROGRAPH Combine 900"
"      6 Combine "
"      900 Node #"
"      SWMF"
"      Maximum flow 0.127 c.m/sec"
"      Hydrograph volume 2150.744 c.m"
"      0.009 0.009 0.008 0.127"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.009 0.000 0.008 0.127"
" 33 CATCHMENT 2015"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2015 Catchment 201-5"
"      85.000 % Impervious"
"      0.074 Total Area"
"      15.000 Flow length"
"      2.000 Overland Slope"
"      0.011 Pervious Area"
"      15.000 Pervious length"
"      2.000 Pervious slope"
"      0.063 Impervious Area"
"      15.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.730 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.963 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"

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"	0.011	0.000	0.008	0.127 c.m/sec"
"	Catchment 2015	Pervious	Impervious	Total Area "
"	Surface Area	0.011	0.063	0.074 hectare"
"	Time of concentration	10.558	1.887	2.911 minutes"
"	Time to Centroid	2500.415	2269.665	2296.907 minutes"
"	Rainfall depth	285.000	285.000	285.000 mm"
"	Rainfall volume	31.64	179.26	210.90 c.m"
"	Rainfall losses	76.931	10.684	20.621 mm"
"	Runoff depth	208.069	274.316	264.379 mm"
"	Runoff volume	23.10	172.55	195.64 c.m"
"	Runoff coefficient	0.730	0.963	0.928 "
"	Maximum flow	0.002	0.009	0.011 c.m/sec"
" 40	HYDROGRAPH Add Runoff "			
"	4 Add Runoff "			
"	0.011	0.011	0.008	0.127"
" 40	HYDROGRAPH Copy to Outflow"			
"	8 Copy to Outflow"			
"	0.011	0.011	0.011	0.127"
" 40	HYDROGRAPH Combine	900"		
"	6 Combine "			
"	900 Node #"			
"	SWMF"			
"	Maximum flow		0.137	c.m/sec"
"	Hydrograph volume		2346.385	c.m"
"	0.011	0.011	0.011	0.137"
" 40	HYDROGRAPH Start - New Tributary"			
"	2 Start - New Tributary"			
"	0.011	0.000	0.011	0.137"
" 33	CATCHMENT 2016"			
"	1 Triangular SCS"			
"	1 Equal length"			
"	1 SCS method"			
"	2016 Catchment 201-6"			
"	100.000 % Impervious"			
"	0.032 Total Area"			
"	10.000 Flow length"			
"	2.000 Overland Slope"			
"	0.000 Pervious Area"			
"	10.000 Pervious length"			
"	2.000 Pervious slope"			
"	0.032 Impervious Area"			
"	10.000 Impervious length"			
"	2.000 Impervious slope"			
"	0.250 Pervious Manning 'n'"			
"	74.000 Pervious SCS Curve No."			
"	0.000 Pervious Runoff coefficient"			
"	0.100 Pervious Ia/S coefficient"			
"	8.924 Pervious Initial abstraction"			
"	0.015 Impervious Manning 'n'"			
"	98.000 Impervious SCS Curve No."			
"	0.970 Impervious Runoff coefficient"			
"	0.100 Impervious Ia/S coefficient"			
"	0.518 Impervious Initial abstraction"			
"	0.005	0.000	0.011	0.137 c.m/sec"
"	Catchment 2016	Pervious	Impervious	Total Area "
"	Surface Area	0.000	0.032	0.032 hectare"
"	Time of concentration	8.278	1.479	1.479 minutes"
"	Time to Centroid	2497.307	2266.252	2266.252 minutes"
"	Rainfall depth	285.000	285.000	285.000 mm"
"	Rainfall volume	0.00	91.20	91.20 c.m"
"	Rainfall losses	76.932	8.442	8.442 mm"
"	Runoff depth	208.068	276.558	276.558 mm"
"	Runoff volume	0.00	88.50	88.50 c.m"
"	Runoff coefficient	0.000	0.970	0.970 "
"	Maximum flow	0.000	0.005	0.005 c.m/sec"

" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.005	0.005	0.011	0.137"	
" 57	TRENCH Design d/s of 2016"				
"	0.005 Peak inflow"				
"	88.498 Hydrograph volume"				
"	337.000 Ground elevation"				
"	334.200 Downstream trench invert"				
"	1.000 Trench height"				
"	333.200 Water table elevation"				
"	5.000 Trench top width"				
"	5.000 Trench bottom width"				
"	40.000 Voids ratio (%)"				
"	30.000 Hydraulic conductivity"				
"	0.000 Trench gradient (%)"				
"	5.000 Trench length"				
"	1.000 Include base width"				
"	29. Number of stages"				
"	Level Discharge	Volume"			
"	334.200	0.000	0.0"		
"	334.300	0.000	1.0"		
"	334.400	0.000	2.0"		
"	334.500	0.000	3.0"		
"	334.600	0.000	4.0"		
"	334.700	0.000	5.0"		
"	334.800	0.000	6.0"		
"	334.900	0.000	7.0"		
"	335.000	0.000	8.0"		
"	335.100	0.000	9.0"		
"	335.200	0.000	10.0"		
"	335.300	0.002	10.1"		
"	335.400	0.006	10.2"		
"	335.500	0.008	10.3"		
"	335.600	0.010	10.5"		
"	335.700	0.011	10.6"		
"	335.800	0.013	10.7"		
"	335.900	0.014	10.8"		
"	336.000	0.015	10.9"		
"	336.100	0.016	11.0"		
"	336.200	0.017	11.1"		
"	336.300	0.018	11.2"		
"	336.400	0.019	11.4"		
"	336.500	0.020	11.5"		
"	336.600	0.020	11.6"		
"	336.700	0.021	11.7"		
"	336.800	0.022	11.8"		
"	336.900	0.023	11.9"		
"	337.000	0.023	12.0"		
"	1. TRENCH PIPES"				
"	Downstream	Pipe	Pipe	Pipe Perf'ted?	Offset"
"	Invert	length	diam.	grade%	0=Yes distance"
"	335.200	10.000	0.000	2.000	1.000 0.000"
"	1. MANHOLE"				
"	Access				
"	diameter"				
"	1.200"				
"	1. OUTFLOW PIPE"				
"	0. Inflow at upstream end of trench: 1=True; 0=False"				
"	Upstream Downstr'm	Pipe	Pipe	Manning	Entry"
"	invert	invert	Length	Diameter	'n' loss Ke
"	335.200	335.000	10.000	0.100	0.013 0.500"
"	Peak outflow		0.004	c.m/sec"	
"	Outflow volume		41.317	c.m"	
"	Peak exfiltration		0.000	c.m/sec"	
"	Exfiltration volume		47.255	c.m"	

" Maximum level 335.356 metre"  
" Maximum storage 10.176 c.m"  
" Centroidal lag 45.094 hours"  
" Infiltration area 2 sides 10.000 sq.metre"  
" Infiltration Base area 25.000 sq.metre"  
" 0.005 0.005 0.004 0.000 c.m/sec"  
" 40 HYDROGRAPH Combine 900"  
" 6 Combine "  
" 900 Node #"  
" SWMF"  
" Maximum flow 0.141 c.m/sec"  
" Hydrograph volume 2387.699 c.m"  
" 0.005 0.005 0.004 0.141"  
" 40 HYDROGRAPH Start - New Tributary"  
" 2 Start - New Tributary"  
" 0.005 0.000 0.004 0.141"  
" 33 CATCHMENT 2017"  
" 1 Triangular SCS"  
" 1 Equal length"  
" 1 SCS method"  
" 2017 Catchment 201-7 - SWMF"  
" 40.000 % Impervious"  
" 0.211 Total Area"  
" 10.000 Flow length"  
" 10.000 Overland Slope"  
" 0.127 Pervious Area"  
" 10.000 Pervious length"  
" 10.000 Pervious slope"  
" 0.084 Impervious Area"  
" 10.000 Impervious length"  
" 10.000 Impervious slope"  
" 0.250 Pervious Manning 'n'"  
" 74.000 Pervious SCS Curve No."  
" 0.729 Pervious Runoff coefficient"  
" 0.100 Pervious Ia/S coefficient"  
" 8.924 Pervious Initial abstraction"  
" 0.015 Impervious Manning 'n'"  
" 98.000 Impervious SCS Curve No."  
" 0.938 Impervious Runoff coefficient"  
" 0.100 Impervious Ia/S coefficient"  
" 0.518 Impervious Initial abstraction"  
" 0.029 0.000 0.004 0.141 c.m/sec"  
" Catchment 2017 Pervious Impervious Total Area "  
" Surface Area 0.127 0.084 0.211 hectare"  
" Time of concentration 5.108 0.913 3.171 minutes"  
" Time to Centroid 2493.048 2266.347 2388.354 minutes"  
" Rainfall depth 285.000 285.000 285.000 mm"  
" Rainfall volume 360.81 240.54 601.35 c.m"  
" Rainfall losses 77.349 17.727 53.500 mm"  
" Runoff depth 207.651 267.273 231.500 mm"  
" Runoff volume 262.89 225.58 488.46 c.m"  
" Runoff coefficient 0.729 0.938 0.812 "  
" Maximum flow 0.017 0.011 0.029 c.m/sec"  
" 40 HYDROGRAPH Add Runoff "  
" 4 Add Runoff "  
" 0.029 0.029 0.004 0.141"  
" 40 HYDROGRAPH Copy to Outflow"  
" 8 Copy to Outflow"  
" 0.029 0.029 0.029 0.141"  
" 40 HYDROGRAPH Combine 900"  
" 6 Combine "  
" 900 Node #"  
" SWMF"  
" Maximum flow 0.170 c.m/sec"  
" Hydrograph volume 2876.164 c.m"

" 0.029 0.029 0.029 0.170"  
" 40 HYDROGRAPH Confluence 900"  
" 7 Confluence "  
" 900 Node #"  
" SWMF"  
" Maximum flow 0.170 c.m/sec"  
" Hydrograph volume 2876.164 c.m"  
" 0.029 0.170 0.029 0.000"  
" 54 POND DESIGN"  
" 0.170 Current peak flow c.m/sec"  
" 0.045 Target outflow c.m/sec"  
" 2876.2 Hydrograph volume c.m"  
" 13. Number of stages"  
" 334.400 Minimum water level metre"  
" 335.600 Maximum water level metre"  
" 334.400 Starting water level metre"  
" 0 Keep Design Data: 1 = True; 0 = False"  
" Level Discharge Volume"  
" 334.400 0.000 0.000"  
" 334.500 0.00150 40.000"  
" 334.600 0.00230 85.000"  
" 334.700 0.00290 135.000"  
" 334.800 0.03110 189.000"  
" 334.900 0.04300 249.000"  
" 335.000 0.05220 314.000"  
" 335.100 0.05990 384.000"  
" 335.200 0.06680 457.000"  
" 335.300 0.07300 534.000"  
" 335.400 0.1628 614.000"  
" 335.500 0.4275 698.000"  
" 335.600 0.9140 786.000"  
" Peak outflow 0.130 c.m/sec"  
" Maximum level 335.363 metre"  
" Maximum storage 584.560 c.m"  
" Centroidal lag 42.524 hours"  
" 0.029 0.170 0.130 0.000 c.m/sec"  
" 40 HYDROGRAPH Next link "  
" 5 Next link "  
" 0.029 0.130 0.130 0.000"  
" 54 POND DESIGN"  
" 0.130 Current peak flow c.m/sec"  
" 0.045 Target outflow c.m/sec"  
" 2876.6 Hydrograph volume c.m"  
" 10. Number of stages"  
" 334.200 Minimum water level metre"  
" 335.100 Maximum water level metre"  
" 334.200 Starting water level metre"  
" 0 Keep Design Data: 1 = True; 0 = False"  
" Level Discharge Volume"  
" 334.200 0.000 0.000"  
" 334.300 0.00191 18.000"  
" 334.400 0.00207 38.000"  
" 334.500 0.00225 60.000"  
" 334.600 0.00243 83.000"  
" 334.700 0.00261 109.000"  
" 334.800 0.00281 136.000"  
" 334.900 0.1543 165.000"  
" 335.000 0.4628 196.000"  
" 335.100 0.9060 229.000"  
" Peak outflow 0.129 c.m/sec"  
" Maximum level 334.883 metre"  
" Maximum storage 160.155 c.m"  
" Centroidal lag 44.861 hours"  
" 0.029 0.130 0.129 0.000 c.m/sec"  
" 40 HYDROGRAPH Combine 800"



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"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.129 c.m/sec"
"      Hydrograph volume 2876.545 c.m"
"      0.029 0.130 0.129 0.129"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.029 0.000 0.129 0.129"
" 33 CATCHMENT 2021"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2021 Catchment 202-1"
"      0.000 % Impervious"
"      0.863 Total Area"
"      50.000 Flow length"
"      0.500 Overland Slope"
"      0.863 Pervious Area"
"      50.000 Pervious length"
"      0.500 Pervious slope"
"      0.000 Impervious Area"
"      50.000 Impervious length"
"      0.500 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.732 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.123 0.000 0.129 0.129 c.m/sec"
"      Catchment 2021 Pervious Impervious Total Area "
"      Surface Area 0.863 0.000 0.863 hectare"
"      Time of concentration 32.957 5.890 32.957 minutes"
"      Time to Centroid 2532.306 2271.773 2532.307 minutes"
"      Rainfall depth 285.000 285.000 285.000 mm"
"      Rainfall volume 2459.55 0.00 2459.55 c.m"
"      Rainfall losses 76.445 8.151 76.445 mm"
"      Runoff depth 208.555 276.849 208.555 mm"
"      Runoff volume 1799.83 0.00 1799.83 c.m"
"      Runoff coefficient 0.732 0.000 0.732 "
"      Maximum flow 0.123 0.000 0.123 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.123 0.123 0.129 0.129"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.123 0.123 0.123 0.129"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow 0.243 c.m/sec"
"      Hydrograph volume 4676.375 c.m"
"      0.123 0.123 0.123 0.243"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.123 0.000 0.123 0.243"
" 33 CATCHMENT 2022"
"      1 Triangular SCS"
"      1 Equal length"

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"      1 SCS method"
"      2022 Catchment 202-2"
"      24.000 % Impervious"
"      0.184 Total Area"
"      10.000 Flow length"
"      2.000 Overland Slope"
"      0.140 Pervious Area"
"      10.000 Pervious length"
"      2.000 Pervious slope"
"      0.044 Impervious Area"
"      10.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.730 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.924 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.970 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.026 0.000 0.123 0.243 c.m/sec"
"      Catchment 2022 Pervious Impervious Total Area "
"      Surface Area 0.140 0.044 0.184 hectare"
"      Time of concentration 8.278 1.479 6.268 minutes"
"      Time to Centroid 2497.306 2266.253 2428.997 minutes"
"      Rainfall depth 285.000 285.000 285.000 mm"
"      Rainfall volume 398.54 125.86 524.40 c.m"
"      Rainfall losses 76.932 8.442 60.495 mm"
"      Runoff depth 208.068 276.558 224.505 mm"
"      Runoff volume 290.96 122.13 413.09 c.m"
"      Runoff coefficient 0.730 0.970 0.788 "
"      Maximum flow 0.019 0.006 0.026 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.026 0.026 0.123 0.243"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.026 0.026 0.026 0.243"
" 40 HYDROGRAPH Combine 800"
"      6 Combine "
"      800 Node #"
"      TORRANCE CREEK"
"      Maximum flow 0.262 c.m/sec"
"      Hydrograph volume 5089.458 c.m"
"      0.026 0.026 0.026 0.262"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.026 0.000 0.026 0.262"
" 33 CATCHMENT 2031"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2031 Catchment 203-1"
"      30.000 % Impervious"
"      0.216 Total Area"
"      130.000 Flow length"
"      2.000 Overland Slope"
"      0.151 Pervious Area"
"      130.000 Pervious length"
"      2.000 Pervious slope"
"      0.065 Impervious Area"
"      130.000 Impervious length"
"      2.000 Impervious slope"

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"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.732	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.972	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.031	0.000	0.026	0.262 c.m/sec"	
"	Catchment 2031	Pervious	Impervious	Total Area	"
"	Surface Area	0.151	0.065	0.216	hectare"
"	Time of concentration	38.576	6.894	27.083	minutes"
"	Time to Centroid	2540.184	2273.879	2443.578	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	430.92	184.68	615.60	c.m"
"	Rainfall losses	76.446	7.973	55.904	mm"
"	Runoff depth	208.554	277.027	229.096	mm"
"	Runoff volume	315.33	179.51	494.85	c.m"
"	Runoff coefficient	0.732	0.972	0.804	"
"	Maximum flow	0.022	0.010	0.031	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.031	0.031	0.026	0.262"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.031	0.031	0.031	0.262"	
" 40	HYDROGRAPH Combine	700"			
"	6 Combine "				
"	700 Node #"				
"	CULVERT"				
"	Maximum flow		0.031	c.m/sec"	
"	Hydrograph volume		494.847	c.m"	
"	0.031	0.031	0.031	0.031"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.031	0.000	0.031	0.031"	
" 33	CATCHMENT 2032"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2032 Catchment 203-2"				
"	0.000 % Impervious"				
"	0.113 Total Area"				
"	10.000 Flow length"				
"	33.000 Overland Slope"				
"	0.113 Pervious Area"				
"	10.000 Pervious length"				
"	33.000 Pervious slope"				
"	0.000 Impervious Area"				
"	10.000 Impervious length"				
"	33.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.713 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.015	0.000	0.031	0.031 c.m/sec"	
"	Catchment 2032	Pervious	Impervious	Total Area	"

"	Surface Area	0.113	0.000	0.113	hectare"
"	Time of concentration	3.570	0.638	3.570	minutes"
"	Time to Centroid	2485.798	2237.542	2485.797	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	322.05	0.00	322.05	c.m"
"	Rainfall losses	81.898	25.077	81.898	mm"
"	Runoff depth	203.102	259.923	203.102	mm"
"	Runoff volume	229.51	0.00	229.51	c.m"
"	Runoff coefficient	0.713	0.000	0.713	"
"	Maximum flow	0.015	0.000	0.015	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.015	0.015	0.031	0.031"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.015	0.015	0.015	0.031"	
" 40	HYDROGRAPH Combine	700"			
"	6 Combine "				
"	700 Node #"				
"	CULVERT"				
"	Maximum flow		0.045	c.m/sec"	
"	Hydrograph volume		724.353	c.m"	
"	0.015	0.015	0.015	0.045"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.015	0.000	0.015	0.045"	
" 33	CATCHMENT 2033"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2033 Catchment 203-3"				
"	100.000 % Impervious"				
"	0.092 Total Area"				
"	10.000 Flow length"				
"	2.000 Overland Slope"				
"	0.000 Pervious Area"				
"	10.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.092 Impervious Area"				
"	10.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.000 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.970 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.013	0.000	0.015	0.045 c.m/sec"	
"	Catchment 2033	Pervious	Impervious	Total Area	"
"	Surface Area	0.000	0.092	0.092	hectare"
"	Time of concentration	8.278	1.479	1.479	minutes"
"	Time to Centroid	2497.307	2266.251	2266.253	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	0.00	262.20	262.20	c.m"
"	Rainfall losses	76.932	8.442	8.442	mm"
"	Runoff depth	208.068	276.558	276.558	mm"
"	Runoff volume	0.00	254.43	254.43	c.m"
"	Runoff coefficient	0.000	0.970	0.970	"
"	Maximum flow	0.000	0.013	0.013	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.013	0.013	0.015	0.045"
" 57	TRENCH Design d/s of 2033"				
"	0.013	Peak inflow"			
"	254.433	Hydrograph volume"			
"	337.000	Ground elevation"			
"	334.500	Downstream trench invert"			
"	1.000	Trench height"			
"	333.500	Water table elevation"			
"	1.000	Trench top width"			
"	1.000	Trench bottom width"			
"	40.000	Voids ratio (%)"			
"	30.000	Hydraulic conductivity"			
"	0.000	Trench gradient (%)"			
"	60.000	Trench length"			
"	1.000	Include base width"			
"	26.	Number of stages"			
"		Level	Discharge	Volume"	
"		334.500	0.000	0.0"	
"		334.600	0.000	2.4"	
"		334.700	0.000	4.8"	
"		334.800	0.000	7.2"	
"		334.900	0.000	9.6"	
"		335.000	0.000	12.0"	
"		335.100	0.000	14.4"	
"		335.200	0.000	16.8"	
"		335.300	0.000	19.2"	
"		335.400	0.000	21.6"	
"		335.500	0.000	24.0"	
"		335.600	0.005	24.1"	
"		335.700	0.010	24.2"	
"		335.800	0.016	24.3"	
"		335.900	0.020	24.5"	
"		336.000	0.024	24.6"	
"		336.100	0.027	24.7"	
"		336.200	0.030	24.8"	
"		336.300	0.032	24.9"	
"		336.400	0.035	25.0"	
"		336.500	0.037	25.1"	
"		336.600	0.039	25.2"	
"		336.700	0.041	25.4"	
"		336.800	0.043	25.5"	
"		336.900	0.045	25.6"	
"		337.000	0.047	25.7"	
"	1.	TRENCH PIPES"			
"		Downstream	Pipe	Pipe	Pipe Perf'ted? Offset"
"		Invert	length	diam.	grade% 0=Yes distance"
"		335.500	20.000	0.000	1.000 1.000 0.000"
"	1.	MANHOLE"			
"		Access"			
"		diameter"			
"		1.200"			
"	1.	OUTFLOW PIPE"			
"	0.	Inflow at upstream end of trench: 1=True; 0=False"			
"		Upstream Downstr'm	Pipe	Pipe	Manning Entry"
"		invert	invert	Length	Diameter 'n' loss Ke"
"		335.500	335.300	20.000	0.150 0.013 0.500"
"		Peak outflow	0.011	c.m/sec"	
"		Outflow volume	92.641	c.m"	
"		Peak exfiltration	0.002	c.m/sec"	
"		Exfiltration volume	161.871	c.m"	
"		Maximum level	335.721	metre"	
"		Maximum storage	24.249	c.m"	
"		Centroidal lag	45.242	hours"	
"		Infiltration area 2 sides	120.000	sq.metre"	
"		Infiltration Base area	60.000	sq.metre"	

"		0.013	0.013	0.011	0.002 c.m/sec"
" 40	HYDROGRAPH Combine 700"				
"	6	Combine "			
"	700	Node #"			
"	CULVERT"				
"		Maximum flow	0.056	c.m/sec"	
"		Hydrograph volume	816.992	c.m"	
"		0.013	0.013	0.011	0.056"
" 40	HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"			
"		0.013	0.000	0.011	0.056"
" 33	CATCHMENT 204"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	204	Catchment 204"			
"	0.000	% Impervious"			
"	0.198	Total Area"			
"	8.000	Flow length"			
"	20.000	Overland Slope"			
"	0.198	Pervious Area"			
"	8.000	Pervious length"			
"	20.000	Pervious slope"			
"	0.000	Impervious Area"			
"	8.000	Impervious length"			
"	20.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.712	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.026	0.000	0.011	0.056 c.m/sec"
"		Catchment 204	Pervious	Impervious	Total Area "
"		Surface Area	0.198	0.000	0.198 hectare"
"		Time of concentration	3.629	0.649	3.629 minutes"
"		Time to Centroid	2485.856	2238.670	2485.855 minutes"
"		Rainfall depth	285.000	285.000	285.000 mm"
"		Rainfall volume	564.30	0.00	564.30 c.m"
"		Rainfall losses	82.021	24.612	82.021 mm"
"		Runoff depth	202.979	260.388	202.979 mm"
"		Runoff volume	401.90	0.00	401.90 c.m"
"		Runoff coefficient	0.712	0.000	0.712 "
"		Maximum flow	0.026	0.000	0.026 c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"		0.026	0.026	0.011	0.056"
" 40	HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"			
"		0.026	0.026	0.026	0.056"
" 40	HYDROGRAPH Combine 700"				
"	6	Combine "			
"	700	Node #"			
"	CULVERT"				
"		Maximum flow	0.081	c.m/sec"	
"		Hydrograph volume	1218.890	c.m"	
"		0.026	0.026	0.026	0.081"
" 40	HYDROGRAPH Confluence 700"				
"	7	Confluence "			
"	700	Node #"			
"	CULVERT"				

"	Maximum flow	0.081	c.m/sec"
"	Hydrograph volume	1218.890	c.m"
"	0.026 0.081 0.026	0.000"	
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.026 0.081 0.081	0.000"	
" 40	HYDROGRAPH Combine 800"		
"	6 Combine "		
"	800 Node #"		
"	TORRANCE CREEK"		
"	Maximum flow	0.332	c.m/sec"
"	Hydrograph volume	6308.347	c.m"
"	0.026 0.081 0.081	0.332"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.026 0.000 0.081	0.332"	
" 33	CATCHMENT 2051"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2051 Catchment 205-1"		
"	25.000 % Impervious"		
"	0.098 Total Area"		
"	15.000 Flow length"		
"	2.000 Overland Slope"		
"	0.073 Pervious Area"		
"	15.000 Pervious length"		
"	2.000 Pervious slope"		
"	0.024 Impervious Area"		
"	15.000 Impervious length"		
"	2.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	74.000 Pervious SCS Curve No."		
"	0.730 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.963 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.014 0.000 0.081	0.332 c.m/sec"	
"	Catchment 2051	Pervious	Impervious Total Area "
"	Surface Area	0.073 0.024 0.098	hectare"
"	Time of concentration	10.558 1.887 7.911	minutes"
"	Time to Centroid	2500.415 2269.665 2429.968	minutes"
"	Rainfall depth	285.000 285.000 285.000	mm"
"	Rainfall volume	209.48 69.82 279.30	c.m"
"	Rainfall losses	76.931 10.684 60.369	mm"
"	Runoff depth	208.069 274.316 224.631	mm"
"	Runoff volume	152.93 67.21 220.14	c.m"
"	Runoff coefficient	0.730 0.963 0.788	"
"	Maximum flow	0.010 0.004 0.014	c.m/sec"
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.014 0.014 0.081	0.332"	
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.014 0.014 0.014	0.332"	
" 40	HYDROGRAPH Combine 600"		
"	6 Combine "		
"	600 Node #"		
"	Arkell Rd"		
"	Maximum flow	0.029	c.m/sec"
"	Hydrograph volume	473.437	c.m"
"	0.015 0.015 0.015	0.029"	
" 40	HYDROGRAPH Confluence 600"		
"	7 Confluence "		
"	600 Node #"		
"	Arkell Rd"		
"	Maximum flow	0.029	c.m/sec"
"	Hydrograph volume	473.437	c.m"
"	0.015 0.029 0.015	0.000"	
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.015 0.029 0.029	0.000"	
" 40	HYDROGRAPH Combine 800"		

"	0.014 0.014 0.014 0.014"		
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.014 0.000 0.014 0.014"		
" 33	CATCHMENT 2052"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	2052 Catchment 205-2"		
"	40.000 % Impervious"		
"	0.108 Total Area"		
"	25.000 Flow length"		
"	5.000 Overland Slope"		
"	0.065 Pervious Area"		
"	25.000 Pervious length"		
"	5.000 Pervious slope"		
"	0.043 Impervious Area"		
"	25.000 Impervious length"		
"	5.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	74.000 Pervious SCS Curve No."		
"	0.730 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	8.924 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.962 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.015 0.000 0.014 0.014 c.m/sec"		
"	Catchment 2052	Pervious	Impervious Total Area "
"	Surface Area	0.065 0.043 0.108	hectare"
"	Time of concentration	10.898 1.948 6.711	minutes"
"	Time to Centroid	2500.806 2267.411 2391.624	minutes"
"	Rainfall depth	285.000 285.000 285.000	mm"
"	Rainfall volume	184.68 123.12 307.80	c.m"
"	Rainfall losses	76.968 10.709 50.464	mm"
"	Runoff depth	208.032 274.291 234.536	mm"
"	Runoff volume	134.80 118.49 253.30	c.m"
"	Runoff coefficient	0.730 0.962 0.823	"
"	Maximum flow	0.009 0.006 0.015	c.m/sec"
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.015 0.015 0.014 0.014"		
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.015 0.015 0.015 0.014"		
" 40	HYDROGRAPH Combine 600"		
"	6 Combine "		
"	600 Node #"		
"	Arkell Rd"		
"	Maximum flow	0.029	c.m/sec"
"	Hydrograph volume	473.437	c.m"
"	0.015 0.015 0.015	0.029"	
" 40	HYDROGRAPH Confluence 600"		
"	7 Confluence "		
"	600 Node #"		
"	Arkell Rd"		
"	Maximum flow	0.029	c.m/sec"
"	Hydrograph volume	473.437	c.m"
"	0.015 0.029 0.015	0.000"	
" 40	HYDROGRAPH Copy to Outflow"		
"	8 Copy to Outflow"		
"	0.015 0.029 0.029 0.000"		
" 40	HYDROGRAPH Combine 800"		

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"      6  Combine "
"      800  Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.358      c.m/sec"
"      Hydrograph volume 6781.785      c.m"
"      0.015      0.029      0.029      0.358"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.015      0.000      0.029      0.358"
" 33  CATCHMENT 206"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      206  Catchment 206"
"      70.000  % Impervious"
"      0.035  Total Area"
"      20.000  Flow length"
"      1.250  Overland Slope"
"      0.011  Pervious Area"
"      20.000  Pervious length"
"      1.250  Pervious slope"
"      0.024  Impervious Area"
"      20.000  Impervious length"
"      1.250  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.731  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.963  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.005      0.000      0.029      0.358 c.m/sec"
"      Catchment 206      Pervious      Impervious Total Area "
"      Surface Area      0.011      0.024      0.035      hectare"
"      Time of concentration 14.448      2.582      5.495      minutes"
"      Time to Centroid 2505.975      2268.321      2326.666      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      29.93      69.82      99.75      c.m"
"      Rainfall losses      76.699      10.643      30.460      mm"
"      Runoff depth      208.301      274.357      254.540      mm"
"      Runoff volume      21.87      67.22      89.09      c.m"
"      Runoff coefficient 0.731      0.963      0.893      "
"      Maximum flow      0.001      0.004      0.005      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.005      0.005      0.029      0.358"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.005      0.005      0.005      0.358"
" 40  HYDROGRAPH Combine 800"
"      6  Combine "
"      800  Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.362      c.m/sec"
"      Hydrograph volume 6870.874      c.m"
"      0.005      0.005      0.005      0.362"
" 40  HYDROGRAPH Confluence 800"
"      7  Confluence "
"      800  Node #"
"      TORRANCE CREEK"
"      Maximum flow      0.362      c.m/sec"
"      Hydrograph volume 6870.875      c.m"

```

```

"      0.005      0.362      0.005      0.000"
" 38  START/RE-START TOTALS 800"
"      3  Runoff Totals on EXIT"
"      Total Catchment area      3.108      hectare"
"      Total Impervious area      1.071      hectare"
"      Total % impervious      34.445"
" 19  EXIT"

```

## Appendix D

---

# **Proposed SWM Facility Design Calculations**



**Arkell Road**  
**STORMWATER MANAGEMENT**  
 Guelph, Ontario

Project Number: 42063-104  
 Date: October 8, 2021  
 Design By: AJC  
 File: Q:\42063\104\SWM\September 2021\42063-104 Master SWM Facility Design Sheet.xlsx

**Step 1: Choose Level of Water Quality Control**

Enhanced 80% long-term S.S. removal

**Step 2: Choose Type of Facility**

Wet Pond

**Step 3: Define Catchment area and Imperviousness**

Catchment Area (ha)

1.201

Imperviousness (%)

64.7

Interpolated Storage Volume Requirement (m<sup>3</sup>/ha)

212.63

Permanent Pool Required (m<sup>3</sup>)

207.33

Extended Detention Volume Required (m<sup>3</sup>)

48.04

Table 3.2 Water Quality Storage Requirements based on Receiving Waters (from MOE Stormwater Management Planning and Design Manual, March 2003)					
Protection Level	SWMP Type	Storage Volume (m <sup>3</sup> /ha) for Impervious Level			
		35	55	70	85
Enhanced 80% long-term S.S. removal	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250
Normal 70% long-term S.S. Removal	Wetlands	60	70	80	90
	Hybrid Wet Pond/Wetland	75	90	105	120
	Wet Pond	90	110	130	150
Basic 60% long-term S.S. Removal	Wetlands	60	60	60	60
	Hybrid Wet Pond/Wetland	60	70	75	80
	Wet Pond	60	75	85	95
	Dry Pond (Continuous Flow)	90	150	200	240

35	55	70	85
----	----	----	----



Arkell Road  
STORMWATER MANAGEMENT  
Guelph, Ontario

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Orifice Calculations			
$Q_o = C_d \cdot A_o \cdot (2 \cdot g \cdot H_o)^{0.5}$			
	Orifice 1	Orifice 2	Orifice 3
C <sub>d</sub>	0.63	0.63	0.63
Invert (m)	334.40	334.70	500.00
Width (m)			
Diameter/Height (m)	0.050	0.200	
Type (H/V)	V	H	V

C <sub>d</sub>	Description
0.63	Orifice Plate
0.80	Orifice Tube

Weir Calculations		
$Q_w = \frac{2}{3} \cdot C_d \cdot (2g)^{1/2} \cdot L \cdot H_w^{3/2} + \frac{8}{15} \cdot C_d \cdot (2g)^{1/2} \cdot \tan \theta \cdot H_w^{5/2}$		
C <sub>d</sub>	0.50	0.50
Invert (m)	335.30	500.00
Length (m)	1.000	
Side Slope (H:V)	10	1
Side Slope (rad)	1.471	0.785

STAGE-DISCHARGE RELATIONSHIP (WET CELL)																	Extended Detention	Erosion Control	
Stage	Active Volume	Orifice 1			Orifice 2			Orifice 3			Weir 1 Flow	Weir 2 Flow	Total Flow		Average Discharge	Increment Volume	Increment Dewatering Time	Cumulative Dewatering Time	Cumulative Dewatering Time
		Area	H <sub>o</sub>	Flow	Area	H <sub>o</sub>	Flow	Area	H <sub>o</sub>	Flow									
<i>m</i>	<i>m<sup>3</sup></i>	<i>m<sup>2</sup></i>	<i>m</i>	<i>m<sup>3</sup>/s</i>	<i>m<sup>2</sup></i>	<i>m</i>	<i>m<sup>3</sup>/s</i>	<i>m<sup>2</sup></i>	<i>m</i>	<i>m<sup>3</sup>/s</i>	<i>m<sup>3</sup>/s</i>		<i>m<sup>3</sup>/s</i>		<i>m<sup>3</sup>/s</i>	<i>m<sup>3</sup></i>	<i>hours</i>	<i>hours</i>	<i>hours</i>
334.40	0	0.00	0.00	0.0000	0.03	0.00	0.0000	0.00	0.00	0.0000	0.0000	0.0000	0.0000		0.0008	40	14.90	29.80	29.80
334.50	40	0.00	0.08	0.0015	0.03	0.00	0.0000	0.00	0.00	0.0000	0.0000	0.0000	0.0015		0.0019	45	6.57	14.90	14.90
334.60	85	0.00	0.18	0.0023	0.03	0.00	0.0000	0.00	0.00	0.0000	0.0000	0.0000	0.0023		0.0026	50	5.33	8.33	8.33
334.70	135	0.00	0.28	0.0029	0.03	0.00	0.0000	0.00	0.00	0.0000	0.0000	0.0000	0.0029		0.0170	55	0.89	3.00	3.00
334.80	189	0.00	0.38	0.0034	0.03	0.10	0.0277	0.00	0.00	0.0000	0.0000	0.0000	0.0311		0.0370	60	0.45	2.10	2.10
334.90	249	0.00	0.48	0.0038	0.03	0.20	0.0392	0.00	0.00	0.0000	0.0000	0.0000	0.0430		0.0476	65	0.38	1.65	1.65
335.00	314	0.00	0.58	0.0042	0.03	0.30	0.0480	0.00	0.00	0.0000	0.0000	0.0000	0.0522		0.0561	70	0.35	1.27	1.27
335.10	384	0.00	0.68	0.0045	0.03	0.40	0.0554	0.00	0.00	0.0000	0.0000	0.0000	0.0599		0.0634	73	0.32	0.93	0.93
335.20	457	0.00	0.78	0.0048	0.03	0.50	0.0620	0.00	0.00	0.0000	0.0000	0.0000	0.0668		0.0699	77	0.30	0.61	0.61
335.30	534	0.00	0.88	0.0051	0.03	0.60	0.0679	0.00	0.00	0.0000	0.0000	0.0000	0.0730		0.1179	80	0.19	0.30	0.30
335.40	614	0.00	0.98	0.0054	0.03	0.70	0.0733	0.00	0.00	0.0000	0.0840	0.0000	0.1628		0.2951	84	0.08	0.12	0.12
335.50	698	0.00	1.08	0.0057	0.03	0.80	0.0784	0.00	0.00	0.0000	0.3434	0.0000	0.4275		0.6707	88	0.04	0.04	0.04
335.60	786	0.00	1.18	0.0059	0.03	0.90	0.0832	0.00	0.00	0.0000	0.8249	0.0000	0.9140						





**Arkell Road**  
**STORMWATER MANAGEMENT**  
**Guelph, Ontario**

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**STAGE-STORAGE RELATIONSHIP (WET CELL)**

Stage	Active Depth	Wet Cell			Total Pond Volume	Active Storage Volume	Volume Summary	Ponding Elevation	Comments	Stage
		Area	Volume	Cumulative Volume						
<i>m</i>	<i>m</i>	<i>m</i> <sup>2</sup>	<i>m</i> <sup>3</sup>	<i>m</i> <sup>3</sup>	<i>m</i> <sup>3</sup>	<i>m</i> <sup>3</sup>	<i>m</i> <sup>3</sup>	<i>m</i>		<i>m</i>
333.20		54	0	0	0				Bottom of Cell	333.20
333.30		69	6	6	6					333.30
333.40		84	8	14	14					333.40
333.50		101	9	23	23					333.50
333.60		118	11	34	34					333.60
333.70		136	13	47	47					333.70
333.80		154	15	61	61					333.80
333.90		186	17	78	78					333.90
334.00		221	20	99	99					334.00
334.10		257	24	122	122					334.10
334.20		296	28	150	150					334.20
334.30		337	32	182	182					334.30
334.40	0.00	380	36	218	218	0			Permanent Pool MOE Extended Detention 25mm4hr 2yr 5yr 10yr 25yr 50yr 100yr	334.40
334.50	0.10	425	40	258	258	40	48	334.52		334.50
334.60	0.20	472	45	303	303	85	119	334.67		334.60
334.70	0.30	520	50	352	352	135	155	334.74		334.70
334.80	0.40	571	55	407	407	189	203	334.82		334.80
334.90	0.50	625	60	467	467	249	255	334.91		334.90
335.00	0.60	680	65	532	532	314	328	335.02		335.00
335.10	0.70	713	70	602	602	384	396	335.12		335.10
335.20	0.80	747	73	675	675	457	464	335.21		335.20
335.30	0.90	784	77	751	751	534				335.30
335.40	1.00	821	80	831	831	614			Top of Berm	335.40
335.50	1.10	857	84	915	915	698				335.50
335.60	1.20	905	88	1003	1003	786				335.60

**Arkell Road**  
**STORMWATER MANAGEMENT**  
Guelph, Ontario



Project Number: 42063-104  
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**INFILTRATION CELL**  
**STAGE-STORAGE RELATIONSHIP**

Stage	Area	Depth	Inc. volume	Total Volume	Infiltration*	Weir Flow	Total Flow
<i>m</i>	<i>m</i> <sup>2</sup>	<i>m</i>	<i>m</i> <sup>3</sup>	<i>m</i> <sup>3</sup>	<i>m</i> <sup>3</sup> /s	<i>m</i> <sup>3</sup> /s	<i>m</i> <sup>3</sup> /s
334.2	177	0	0	0	0.00000	0.0000	0.00000
334.3	191	0.1	18	18	0.00191	0.0000	0.00191
334.4	207	0.2	20	38	0.00207	0.0000	0.00207
334.5	225	0.3	22	60	0.00225	0.0000	0.00225
334.6	243	0.4	23	83	0.00243	0.0000	0.00243
334.7	261	0.5	25	109	0.00261	0.0000	0.00261
334.8	281	0.6	27	136	0.00281	0.0000	0.00281
334.9	301	0.7	29	165	0.00301	0.1513	0.15429
335	322	0.8	31	196	0.00322	0.4596	0.46279
335.1	350	0.9	34	229	0.00350	0.9025	0.90601

Notes:

\* based on Darcy Law ,  $Q=K*A*i$

hydraulic conductivity  $k$  (cm/s)= 1.00E-03 PML Geotech, Oct 2018 San/ Sand and Gravel

gradient  $i$  (m/m) = 1

A Design Manual for Sizing Infiltration Ponds, Joel A. Massman,  
Washington State Department Of Transportation Technical Monitor

**Weir Calculations**

$$Q_w = 2/3 * C_d * (2g)^{1/2} * L * H_w^{3/2} + 8/15 * C_d * (2g)^{1/2} * \tan\theta * H_w^{5/2}$$

$C_d$  0.50  
Invert (m) 334.80  
Length (m) 3.000  
Side Slope (H:V) 3  
Side Slope (rad) 1.249



Arkell Road  
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FOREBAY DESIGN CALCULATIONS  
MOE SWM Planning and Design Manual, 2003

Forebay Design Flows

Flow into forebay during the 1:5-year return period event 0.198 m<sup>3</sup>/s  
Flow into forebay during the 25 mm - 4 hour design storm event 0.088 m<sup>3</sup>/s  
Peak flow from main pond outlet for the 25mm design storm (from MIDUSS) 0.003 m<sup>3</sup>/s

Forebay Characteristics

b = 2.5 m bottom width  
y = 1.2 m depth  
z = 3 :1 side slope  
w = 6.1 m average width  
R = 0.73 m hydraulic radius  
A = 7.3 m<sup>2</sup> cross-sectional area

1. Length Calculation Based on Settling Velocity

L = forebay flow length (m)  
r = length-to-width ratio  
Q<sub>p</sub> = peak flow rate through forebay (m<sup>3</sup>/s)  
v<sub>s</sub> = settling velocity (m/s)

Equation 4.5: Forebay Settling Length

a) Required Settling Length (assuming Q<sub>p</sub> = forebay through-flow & v<sub>s</sub> = 0.0055 m/s)

Q<sub>p</sub> = 0.09 m<sup>3</sup>/s peak flow rate through forebay  
v<sub>s</sub> = 0.0055 m/s settling velocity  
r = 0.43 length-to-width ratio  
L = 2.6 m required settling length  
L = 2.6 m trial length

b) Required Settling Length (assuming Q<sub>p</sub> = pond discharge & v<sub>s</sub> = 0.0003 m/s)

Q<sub>p</sub> = 0.003 m<sup>3</sup>/s peak flow rate through forebay  
v<sub>s</sub> = 0.0003 m/s settling velocity  
r = 0.28 length-to-width ratio  
L = 1.7 m required settling length  
L = 1.7 m trial length

Table 1: Average settling velocities

	Mass Removed	Particle Size Range	Average Settling Velocity
	%	µm	m/s
	80 - 100	x ≤ 20	0.00000254
Enhanced:	70 - 80	20 < x ≤ 40	0.00001300
Normal:	60 - 70	40 < x ≤ 60	0.00002540
Basic:	40 - 60	60 < x ≤ 130	0.00012700
Medium Sand:	20 - 40	130 < x ≤ 400	0.00059267
Gross Grit:	0 - 20	400 < x ≤ 4000	0.00550333

2. Length Calculation Based on Flow Dispersion Length

Q = 0.20 m<sup>3</sup>/s inlet flow rate  
d = 1.2 m depth of permanent pool in forebay  
V<sub>f</sub> = 0.50 m/s desired velocity in forebay (typical value ≤ 0.50 m/s)  
L = 2.6 m required length of dispersion

Equation 4.6: Dispersion Length

3. Required Forebay Length

L = 2.6 m design length  
r = 0.43 design length-to-width ratio (typical minimum of 2.0)

NOTE: LENGTH TO WIDTH RATIO IS LESS THAN THE TYPICAL MINIMUM

4. Scour Velocity

v<sub>s</sub> = 0.15 m/s scour velocity (typical value = 0.15 m/s)  
v = 0.027 m/s actual velocity

OK The actual velocity through the forebay is less than the scour velocity.

5. Weir Flow From Forebay

L = 1 m length of crest of weir  
α = 1.65 coefficient  
H = 0.5 m head  
Q = 0.58 m<sup>3</sup>/s discharge

Equation 4.4: Weir Flow

OK The weir flow from the forebay exceeds the flow entering the forebay

6. Estimated Cleanout Frequencies

a) Forebay

Forebay volume 218 m<sup>3</sup>  
Estimated TSS removal efficiency 80%  
Impervious level 65%  
Estimated annual sediment loading 2.5 m<sup>3</sup>/ha  
Contributing area 1.20 ha  
Annual sediment volume 2 m<sup>3</sup>/yr  
Cleanout frequency for 33% volume reduction 29.5 years

Table 2: Annual sediment loading

Impervious Level	Annual Loading
%	m <sup>3</sup> /ha
35%	0.6
55%	1.9
70%	2.8
85%	3.8



# **Arkell Road** **STORMWATER MANAGEMENT** **Guelph, Ontario**

Project Number: 42063-104  
 Date: October 8, 2021  
 Design By: AJC  
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## **FALLING HEAD DRAWDOWN CALCULATION (25mm4hr)**

MOE SWM Planning and Design Manual, 2003

$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o} \quad \text{Equation 4.11}$$

where

t =	80969.7 s	
	22.5 hr	drawdown time
A <sub>p</sub> =	506.99 m <sup>2</sup>	surface area of the pond
C =	0.63	discharge coefficient
d =	50 mm	diameter of the orifice
A <sub>o</sub> =	0.00196 m <sup>2</sup>	cross-sectional area of the orifice
g =	9.81 m/s <sup>2</sup>	gravitational acceleration constant
h <sub>1</sub> =	334.67 m	starting water elevation above the orifice
h <sub>2</sub> =	334.40 m	ending water elevation above the orifice
h =	0.27 m	maximum water elevation above the orifice
C <sub>2</sub> =	477	slope coefficient from the area-depth linear regression
C <sub>3</sub> =	378.2	intercept from the area-depth linear regression

	ELEVATION <i>m</i>	STAGE <i>m</i>	AREA <i>m</i> <sup>2</sup>	COMMENTS
1	334.400	0	380	Permanent pool
2	334.500	0.1	425	
3	334.600	0.2	472	
4	334.700	0.3	520	
5	334.800	0.4	571	

**DRAWDOWN TIME:** 80970 s  
 22.5 hr

### Regression Output:

m <sub>1</sub> =	477.00	slope coefficient from the area-depth linear regression
b =	378.20	intercept from the area-depth linear regression
se <sub>1</sub> =	6.40	standard error for coefficient m <sub>1</sub>
se <sub>b</sub> =	1.57	standard error for constant b
R <sup>2</sup> =	0.9995	coefficient of determination
se <sub>y</sub> =	2.02	standard error of the y estimate
F =	5549.49	F statistic
df =	3	degrees of freedom
SS <sub>reg</sub> =	22753	regression sum of squares
SS <sub>resid</sub> =	12	residual sum of squares



# **Arkell Road** **STORMWATER MANAGEMENT** **Guelph, Ontario**

Project Number: 42063-104  
 Date: October 8, 2021  
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 File: Q:\42063\104\SWM\September 2021\42063-104 Master SWM Facility Design Sheet.xlsx

## **FALLING HEAD DRAWDOWN CALCULATION (MOE EXT DET)**

MOE SWM Planning and Design Manual, 2003

$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o} \quad \text{Equation 4.11}$$

where

$t = 50950.2 \text{ s}$   
 $14.2 \text{ hr}$  drawdown time  
 $A_p = 435.44 \text{ m}^2$  surface area of the pond  
 $C = 0.63$  discharge coefficient  
 $d = 50 \text{ mm}$  diameter of the orifice  
 $A_o = 0.00196 \text{ m}^2$  cross-sectional area of the orifice  
 $g = 9.81 \text{ m/s}^2$  gravitational acceleration constant  
 $h_1 = 334.52 \text{ m}$  starting water elevation above the orifice  
 $h_2 = 334.40 \text{ m}$  ending water elevation above the orifice  
 $h = 0.12 \text{ m}$  maximum water elevation above the orifice  
 $C_2 = 477$  slope coefficient from the area-depth linear regression  
 $C_3 = 378.2$  intercept from the area-depth linear regression

	ELEVATION <i>m</i>	STAGE <i>m</i>	AREA <i>m</i> <sup>2</sup>	COMMENTS
1	334.400	0	380	Permanent pool
2	334.500	0.1	425	
3	334.600	0.2	472	
4	334.700	0.3	520	
5	334.800	0.4	571	

**DRAWDOWN TIME:** 50950 s  
 14.2 hr

### Regression Output:

$m_1 = 477.00$  slope coefficient from the area-depth linear regression  
 $b = 378.20$  intercept from the area-depth linear regression  
 $se_1 = 6.40$  standard error for coefficient  $m_1$   
 $se_b = 1.57$  standard error for constant  $b$   
 $R^2 = 0.9995$  coefficient of determination  
 $se_y = 2.02$  standard error of the  $y$  estimate  
 $F = 5549.49$  F statistic  
 $df = 3$  degrees of freedom  
 $SS_{reg} = 22753$  regression sum of squares  
 $SS_{resid} = 12$  residual sum of squares

**Arkel Road Subdivision**  
**STORMWATER MANAGEMENT**  
**Guelph, Ontario**



Project Number: 42063-104  
 Date: October 8, 2021  
 Design By: AJC  
 File: Q:\42063\104\SWM\September 2021\42063-104 Roof Infiltration Galleries.xlsx

**ROOF INFILTRATION GALLERIES**

	25mm
Rainfall Depth (mm)*	25

Soil	
hydraulic conductivity k (cm/s)=	1X10 <sup>-3</sup>
Percolation Rate (mm/h)	35.0
Porosity	0.4

Retention Time (h)	24
--------------------	----

Block Area	Roof Area	Volume of 25mm Rainfall	Required Trench Bottom Area
(ha)	(m <sup>2</sup> )	(m <sup>3</sup> )	(m <sup>2</sup> )
Block 1	648	16	48
Block 2	324	8	24
Block 3 + Block 4	920	23	68

Notes:

\* from MIDUSS, 3h-100yr Chicago Storm

Approximate Potential Dimensions

	Depth	Width	Length	Volume of Stone	Volume of Water Stored
	(m)	(m)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )
Block 1	1.0	5.0	10.0	50	20
Block 2	1.0	5.0	5.0	25	10
Block 3 + Block 4	1.0	1.0	60.0	60	24

# Stormceptor®EF Sizing Report

## STORMCEPTOR®

### ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

12/01/2021

Province:	Ontario
City:	Guelph
Nearest Rainfall Station:	WATERLOO WELLINGTON AP
Climate Station Id:	6149387
Years of Rainfall Data:	34

Project Name:	190-216 Arkell
Project Number:	42063-104
Designer Name:	Alex Cressman
Designer Company:	MTE Consultants Inc.
Designer Email:	acressman@mte85.com
Designer Phone:	519-743-6500
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	190-216 Arkell
------------	----------------

Drainage Area (ha):	1.201
---------------------	-------

% Imperviousness:	64.70
-------------------	-------

Runoff Coefficient 'c': 0.68

Particle Size Distribution:	CA ETV
-----------------------------	--------

Target TSS Removal (%):	60.0
-------------------------	------

Required Water Quality Runoff Volume Capture (%):	
---	--

Estimated Water Quality Flow Rate (L/s):	32.90
--	-------

Oil / Fuel Spill Risk Site?	No
-----------------------------	----

Upstream Flow Control?	No
------------------------	----

Peak Conveyance (maximum) Flow Rate (L/s):	
--	--

Site Sediment Transport Rate (kg/ha/yr):	
--	--

### Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EF4	49
EF6	56
EF8	61
EF10	64
EF12	66

**Recommended Stormceptor EF Model: EF8**

**Estimated Net Annual Sediment (TSS) Load Reduction (%): 61**

**Water Quality Runoff Volume Capture (%): > 90**

## Stormceptor® EF Sizing Report

### THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

### PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

### PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5



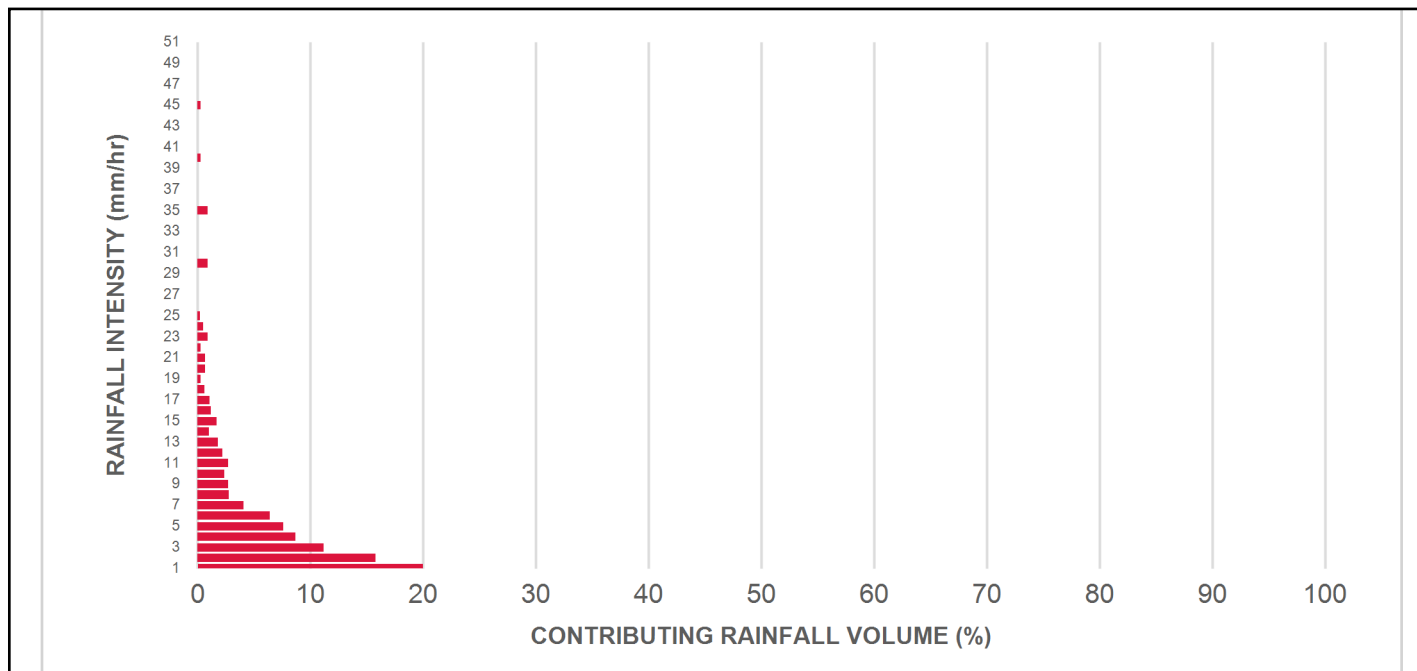
# Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
1	20.0	20.0	2.30	138.0	29.0	70	14.1	14.1
2	15.8	35.8	4.60	276.0	59.0	69	10.9	24.9
3	11.2	47.0	6.89	414.0	88.0	64	7.1	32.1
4	8.7	55.7	9.19	551.0	117.0	62	5.4	37.5
5	7.6	63.3	11.49	689.0	147.0	59	4.5	41.9
6	6.4	69.7	13.79	827.0	176.0	57	3.6	45.6
7	4.1	73.8	16.08	965.0	205.0	54	2.2	47.8
8	2.8	76.7	18.38	1103.0	235.0	53	1.5	49.3
9	2.7	79.4	20.68	1241.0	264.0	52	1.4	50.7
10	2.4	81.7	22.98	1379.0	293.0	51	1.2	51.9
11	2.7	84.5	25.28	1517.0	323.0	50	1.4	53.3
12	2.2	86.7	27.57	1654.0	352.0	50	1.1	54.4
13	1.8	88.4	29.87	1792.0	381.0	49	0.9	55.2
14	1.0	89.5	32.17	1930.0	411.0	48	0.5	55.7
15	1.7	91.2	34.47	2068.0	440.0	48	0.8	56.6
16	1.2	92.3	36.76	2206.0	469.0	47	0.6	57.1
17	1.1	93.5	39.06	2344.0	499.0	47	0.5	57.7
18	0.6	94.1	41.36	2482.0	528.0	47	0.3	57.9
19	0.3	94.3	43.66	2619.0	557.0	47	0.1	58.1
20	0.7	95.0	45.95	2757.0	587.0	46	0.3	58.4
21	0.7	95.7	48.25	2895.0	616.0	46	0.3	58.7
22	0.3	96.0	50.55	3033.0	645.0	46	0.1	58.8
23	0.9	96.9	52.85	3171.0	675.0	46	0.4	59.2
24	0.5	97.4	55.15	3309.0	704.0	46	0.2	59.5
25	0.2	97.6	57.44	3447.0	733.0	45	0.1	59.5
30	0.9	98.5	68.93	4136.0	880.0	45	0.4	60.0
35	0.9	99.4	80.42	4825.0	1027.0	44	0.4	60.4
40	0.3	99.7	91.91	5515.0	1173.0	46	0.1	60.5
45	0.3	100.0	103.40	6204.0	1320.0	48	0.1	60.6
50	0.0	100.0	114.89	6893.0	1467.0	47	0.0	60.6
Estimated Net Annual Sediment (TSS) Load Reduction =								61 %

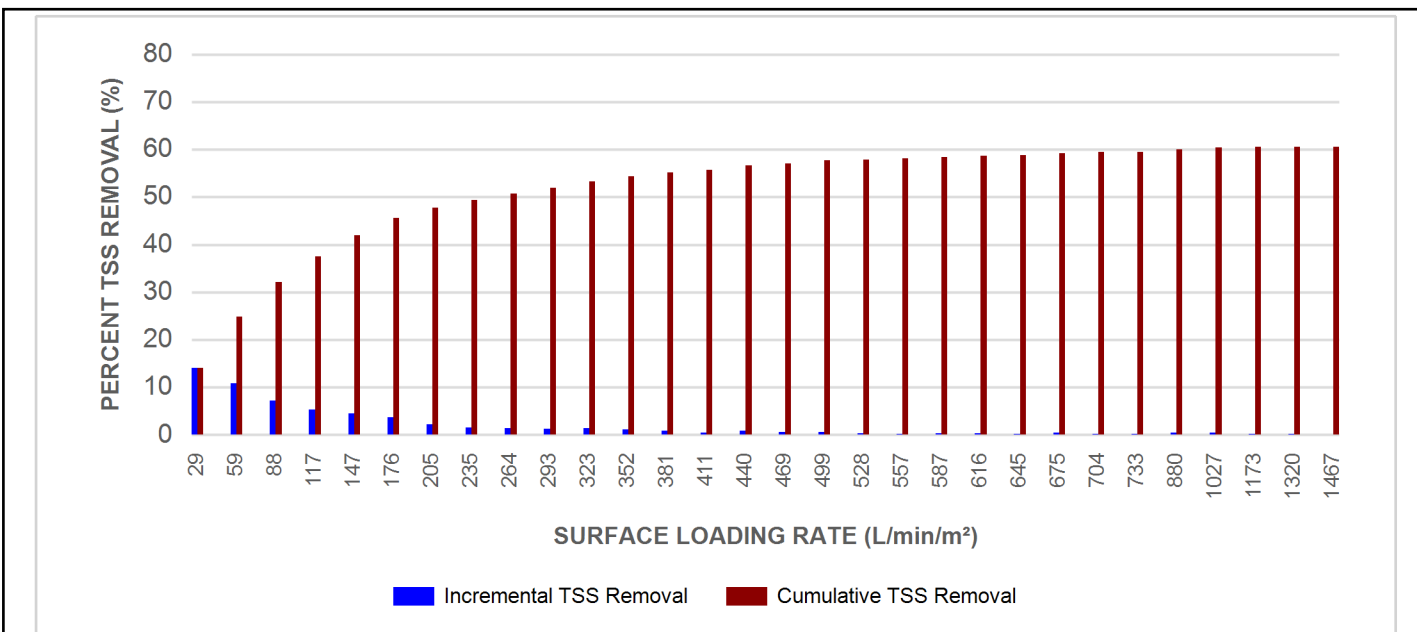
Climate Station ID: 6149387 Years of Rainfall Data: 34

# Stormceptor®EF Sizing Report

## RAINFALL DATA FROM WATERLOO WELLINGTON AP RAINFALL STATION



## INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



## Stormceptor® EF Sizing Report

### Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

### SCOUR PREVENTION AND ONLINE CONFIGURATION

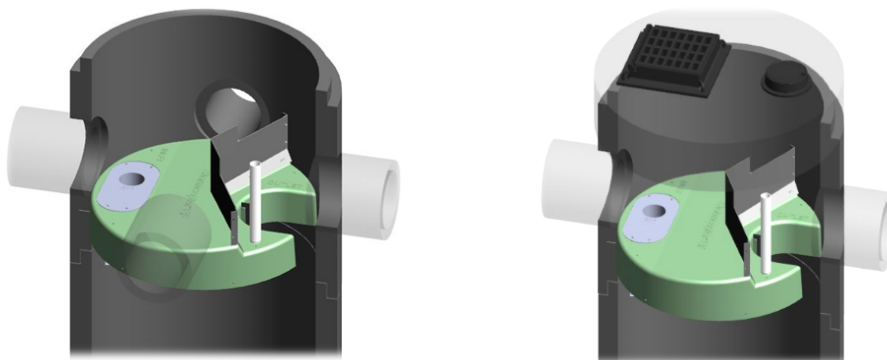
► **Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

### DESIGN FLEXIBILITY

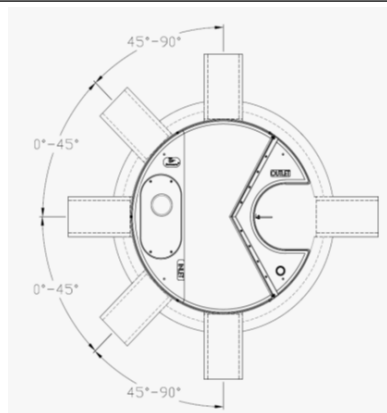
► **Stormceptor® EF and EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

### OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



## Stormceptor® EF Sizing Report



### INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

### HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

### Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

\*Increased sump depth may be added to increase sediment storage capacity

\*\* Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

### STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

### STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

## Stormceptor® EF Sizing Report

**Table of TSS Removal vs Surface Loading Rate Based on Third-Party Test Results  
Stormceptor® EF**

SLR (L/min/m²)	TSS % REMOVAL	SLR (L/min/m²)	TSS % REMOVAL	SLR (L/min/m²)	TSS % REMOVAL	SLR (L/min/m²)	TSS % REMOVAL
1	70	660	46	1320	48	1980	35
30	70	690	46	1350	48	2010	34
60	67	720	45	1380	49	2040	34
90	63	750	45	1410	49	2070	33
120	61	780	45	1440	48	2100	33
150	58	810	45	1470	47	2130	32
180	56	840	45	1500	46	2160	32
210	54	870	45	1530	45	2190	31
240	53	900	45	1560	44	2220	31
270	52	930	44	1590	43	2250	30
300	51	960	44	1620	42	2280	30
330	50	990	44	1650	42	2310	30
360	49	1020	44	1680	41	2340	29
390	48	1050	45	1710	40	2370	29
420	48	1080	45	1740	39	2400	29
450	48	1110	45	1770	39	2430	28
480	47	1140	46	1800	38	2460	28
510	47	1170	46	1830	37	2490	28
540	47	1200	47	1860	37	2520	27
570	46	1230	47	1890	36	2550	27
600	46	1260	47	1920	36	2580	27
630	46	1290	48	1950	35		

## Stormceptor® EF Sizing Report

### STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

#### PART 1 – GENERAL

##### 1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

##### 1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**.

##### 1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

#### PART 2 – PRODUCTS

##### 2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m <sup>3</sup> sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m <sup>3</sup> sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m <sup>3</sup> sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m <sup>3</sup> sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m <sup>3</sup> sediment / 2,476 L oil

#### PART 3 – PERFORMANCE & DESIGN

##### 3.1 GENERAL

## Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

### 3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing shall be determined using historical rainfall data and a sediment removal performance curve derived from the actual third-party verified laboratory testing data. The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

### 3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m<sup>2</sup>.

## Appendix E

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# Monthly Water Balance Calculations



190-216 Arkell Road  
SITE WATER BUDGET (INFILTRATION) ANALYSIS  
Guelph, Ontario



Project Number: 42063-104  
Date: September 29, 2021  
Design By: YXX/JNJ  
File: Q:\42063\104\SWM\September 2021\42063-104 Water Balance (Thornthwaite-Mather) Oct 2021.xlsx

PRE-DEVELOPMENT CONDITION

Contributing Catchments:	101, 102, 103, 104, 105	Soil Type:	Silt,Sand	Runoff Factor:	0.42
Contributing Areas:	3.11 ha	Vegetation:	Majorly Grass	Evapotranspiration	
Percent Impervious	11.5 %	Topography:	Flat Land	Factor for Impervious	
Weather Station:	Guelph Arboretum	Soil Moisture Retention Capacity:	75 mm	Surfaces:	0.33

Month	Daily Average Temperature (C°)	Monthly Heat Index	Unadjusted Daily PE (mm)	Correction Factor	Adjusted PE (mm)	Average Precipitation (mm)	P-PE (mm)	Accum. Pot. Water Loss (mm)	Storage (mm)	ΔS (mm)	Pervious ET (mm)	Actual ET (mm)	Moisture Surplus (mm)	Water Runoff (mm)	Snow Melt Runoff (mm)	Total Recharge & Runoff (mm)	Total Recharge & Runoff (m³)	Total Infiltration Depth (mm)	Total Infiltration Volume (m³)	Actual Runoff (mm)	Runoff Volume (m³)
Jan	-7.6	0.00	0.0	24.3	0.0	56.4	56.4	0.0	209.1	0.0	0.0	0.0	0.0	10.4	0.0	10.4	324	6.0	186	4.4	138
Feb	-6.9	0.00	0.0	24.5	0.0	50.8	50.8	0.0	259.9	0.0	0.0	0.0	0.0	5.2	0.0	5.2	162	3.0	93	2.2	69
Mar	-1.3	0.00	0.0	30.6	0.0	72.1	72.1	0.0	332.0	0.0	0.0	0.0	0.0	2.6	0.0	2.6	81	1.5	47	1.1	34
Apr	5.9	1.28	0.9	33.6	31.8	78.3	46.5	0.0	75.0	0.0	31.8	29.4	48.9	25.8	25.6	51.4	1,597	29.6	919	21.8	678
May	12.3	3.91	2.0	38.0	77.2	79.9	2.7	0.0	75.0	0.0	77.2	71.2	8.7	17.2	115.3	132.5	4,119	76.2	2,370	56.3	1,749
Jun	16.9	6.32	2.8	38.6	109.0	76	-33.0	-33.0	47.0	-28.0	104.0	96.0	8.0	12.6	57.6	70.3	2,184	40.4	1,256	29.8	927
Jul	19.7	7.97	3.3	38.9	128.8	88.5	-40.3	-73.3	27.0	-20.0	108.5	100.1	8.4	10.5	28.8	39.3	1,222	22.6	703	16.7	519
Aug	18.6	7.31	3.1	36.0	112.3	95.9	-16.4	-89.7	22.0	-5.0	100.9	93.1	7.8	9.1	14.4	23.5	731	13.5	421	10.0	311
Sep	14.1	4.80	2.3	31.2	73.0	92.1	19.1	0.0	41.1	19.1	73.0	67.4	5.6	7.4	7.2	14.6	453	8.4	261	6.2	192
Oct	7.9	2.00	1.3	28.5	36.5	69.2	32.7	0.0	73.8	32.7	36.5	33.7	2.8	5.1	3.6	8.7	270	5.0	155	3.7	115
Nov	2.4	0.33	0.4	24.2	9.0	86.3	77.3	0.0	75.0	1.2	9.0	8.3	76.7	40.9	1.8	42.7	1,328	24.6	764	18.1	564
Dec	-4	0.00	0.0	23.0	0.0	77.7	77.7	0.0	152.7	0.0	0.0	0.0	0.0	20.9	1.8	22.7	704	13.0	405	9.6	299
Total		33.9	16.2		577.6	923.2	345.6				499.3		166.9	167.7	256.2	423.9	13,174	243.9	7,580	180.0	5,595

Note: P - Precipitation, PE - Potential Evapotranspiration, ΔS- Change in Soil Moisture Storage, ET - Evapotranspiration

190-216 Arkell Road  
SITE WATER BUDGET (INFILTRATION) ANALYSIS  
Guelph, Ontario



Project Number: 42063-104  
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File: Q:\42063\104\SWM\September 2021\42063-104 Water Balance (Thorntwaite-Mather) Oct 2021.xlsx

POST-DEVELOPMENT CONDITION

Contributing Catchments: 201,202,203,204,205-1, 205-2,206,207

Contributing Areas: 3.11 ha

Percent Impervious: 34.3 %

Weather Station: Guelph Arboretum

Soil Type: Silt,Sand

Vegetation: Urban Lawn

Topography: Flat Land

Soil Moisture Retention Capacity: 75 mm

Runoff Factor: 0.57

Evapotranspiration Factor for Impervious Surfaces: 0.33

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily PE	Correction Factor	Adjusted PE	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual ET	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Total Recharge & Runoff	Runoff before Enhanced Infiltration	Runoff before Enhanced Infiltration	Total Enhanced Recharge*	Total Enhanced Recharge	Recharge Pervious	Recharge Pervious	Total Recharge	Total Recharge	Acutal Runoff Volume	Acutal Runoff
	(C°)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m³)	(mm)	(mm)	(m³)	(mm)	(m³)	(mm)	(m³)	(mm)	(m³)	(mm)
Jan	-7.6	0.00	0.0	24.3	0.0	56.4	56.4	0.0	209.1	0.0	0.0	0.0	0.0	12.6	0.0	12.6	392	7.2	225	0	0	168	5.4	168	5.4	225	7.2
Feb	-6.9	0.00	0.0	24.5	0.0	50.8	50.8	0.0	259.9	0.0	0.0	0.0	0.0	6.3	0.0	6.3	196	3.6	112	0	0	84	2.7	84	2.7	112	3.6
Mar	-1.3	0.00	0.0	30.6	0.0	72.1	72.1	0.0	332.0	0.0	0.0	0.0	0.0	3.2	0.0	3.2	98	1.8	56	0	0	42	1.3	42	1.3	56	1.8
Apr	5.9	1.28	0.9	33.6	31.8	78.3	46.5	0.0	75.0	0.0	31.8	24.5	53.8	28.5	25.3	53.7	1,670	30.8	957	129	4	713	22.9	842	27.1	828	26.6
May	12.3	3.91	2.0	38.0	77.2	79.9	2.7	0.0	75.0	0.0	77.2	59.4	20.5	24.5	113.7	138.2	4,294	79.2	2,461	123	4	1,834	59.0	1,956	63.0	2,338	75.2
Jun	16.9	6.32	2.8	38.6	109.0	76	-33.0	-33.0	47.0	-28.0	104.0	80.1	23.9	24.2	56.9	81.1	2,518	46.4	1,443	136	4	1,075	34.6	1,211	39.0	1,307	42.1
Jul	19.7	7.97	3.3	38.9	128.8	88.5	-40.3	-73.3	27.0	-20.0	108.5	83.6	24.9	24.6	28.4	53.0	1,647	30.4	944	145	5	703	22.6	848	27.3	799	25.7
Aug	18.6	7.31	3.1	36.0	112.3	95.9	-16.4	-89.7	22.0	-5.0	100.9	77.7	23.2	23.9	14.2	38.1	1,184	21.8	678	138	4	505	16.3	643	20.7	541	17.4
Sep	14.1	4.80	2.3	31.2	73.0	92.1	19.1	0.0	41.1	19.1	73.0	56.2	16.8	20.3	7.1	27.4	853	15.7	489	140	4	364	11.7	504	16.2	349	11.2
Oct	7.9	2.00	1.3	28.5	36.5	69.2	32.7	0.0	73.8	32.7	36.5	28.1	8.4	14.4	3.6	17.9	557	10.3	319	122	4	238	7.7	360	11.6	197	6.3
Nov	2.4	0.33	0.4	24.2	9.0	86.3	77.3	0.0	75.0	1.2	9.0	6.9	78.1	46.2	1.8	48.0	1,492	27.5	855	159	5	637	20.5	796	25.6	696	22.4
Dec	-4	0.00	0.0	23.0	0.0	77.7	77.7	0.0	152.7	0.0	0.0	0.0	0.0	25.3	1.8	27.0	840	15.5	481	5	0	359	11.5	363	11.7	477	15.3
Total		33.9	16.2		577.6	923.2	345.6				416.6		249.6	253.9	252.7	506.6	15,740	290.3	9,019	1,095	35	6,721	216.3	7,816	251.6	7,924	255.0

Note: P - Precipitation, PE - Potential Evapotranspiration, ΔS- Change in Soil Moisture Storage, ET - Evapotranspiration  
\* Enhanced recharge volume was estimated by a continuous hydrologic model, based on the design of infiltration facility and condition of its contributing area:

190-216 Arkell Road  
WETLAND WATER BUDGET (RUNOFF) ANALYSIS  
Guelph, Ontario



Project Number: 42063-104  
Date: September 29, 2021  
Design By: YXX/JNJ  
File: Q:\42063\104\SWM\September 2021\42063-104 Water Balance (Thorntwaite-Mather) Oct 2021.xlsx

**POST-DEVELOPMENT CONDITION (Area Draining to Wetland)**  
Contributing Catchments: 201,202,203,204,205-2,206,207      Soil Type: Silt,Sand      Runoff Factor 0.57  
Contributing Areas: 2.90 ha      Vegetation: Urban Lawn      Evapotranspiration  
Percent Impervious 34.4 %      Topography: Flat Land      Factor for Impervious  
Weather Station: Guelph Arboretum      Soil Moisture Retention Capacity 75 mm      Surfaces 0.33

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily PE	Correction Factor	Adjusted PE	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual ET	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Total Recharge & Runoff	Runoff before Enhanced Infiltration	Runoff before Enhanced Infiltration	Total Enhanced Recharge*	Total Enhanced Recharge	Recharge Pervious	Recharge Pervious	Total Recharge	Total Recharge	Acutal Runoff Volume	Acutal Runoff
	(C°)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m³)	(mm)	(mm)	(m³)	(mm)	(m³)	(mm)	(m³)	(mm)	(m³)	(mm)
Jan	-7.6	0.00	0.0	24.3	0.0	56.4	56.4	0.0	209.1	0.0	0.0	0.0	0.0	12.7	0.0	12.7	370	7.3	212	0	0	158	5.4	158	5.4	212	7.3
Feb	-6.9	0.00	0.0	24.5	0.0	50.8	50.8	0.0	259.9	0.0	0.0	0.0	0.0	6.4	0.0	6.4	185	3.7	106	0	0	79	2.7	79	2.7	106	3.7
Mar	-1.3	0.00	0.0	30.6	0.0	72.1	72.1	0.0	332.0	0.0	0.0	0.0	0.0	3.2	0.0	3.2	92	1.8	53	0	0	39	1.4	39	1.4	53	1.8
Apr	5.9	1.28	0.9	33.6	31.8	78.3	46.5	0.0	75.0	0.0	31.8	24.5	53.8	28.5	25.2	53.7	1,559	30.8	895	129	4	665	22.9	794	27.4	765	26.4
May	12.3	3.91	2.0	38.0	77.2	79.9	2.7	0.0	75.0	0.0	77.2	59.4	20.5	24.5	113.5	138.1	4,006	79.2	2,299	123	4	1,708	58.8	1,831	63.1	2,176	75.0
Jun	16.9	6.32	2.8	38.6	109.0	76	-33.0	-33.0	47.0	-28.0	104.0	80.0	24.0	24.3	56.8	81.0	2,351	46.5	1,349	136	5	1,002	34.5	1,138	39.2	1,213	41.8
Jul	19.7	7.97	3.3	38.9	128.8	88.5	-40.3	-73.3	27.0	-20.0	108.5	83.5	25.0	24.6	28.4	53.0	1,539	30.4	883	145	5	656	22.6	801	27.6	738	25.4
Aug	18.6	7.31	3.1	36.0	112.3	95.9	-16.4	-89.7	22.0	-5.0	100.9	77.6	23.3	24.0	14.2	38.1	1,107	21.9	635	138	5	472	16.3	610	21.0	497	17.1
Sep	14.1	4.80	2.3	31.2	73.0	92.1	19.1	0.0	41.1	19.1	73.0	56.2	16.8	20.4	7.1	27.5	798	15.8	458	140	5	340	11.7	480	16.5	318	11.0
Oct	7.9	2.00	1.3	28.5	36.5	69.2	32.7	0.0	73.8	32.7	36.5	28.1	8.4	14.4	3.5	18.0	521	10.3	299	122	4	222	7.7	344	11.9	177	6.1
Nov	2.4	0.33	0.4	24.2	9.0	86.3	77.3	0.0	75.0	1.2	9.0	6.9	78.1	46.3	1.8	48.0	1,394	27.6	800	159	5	594	20.5	753	26.0	641	22.1
Dec	-4	0.00	0.0	23.0	0.0	77.7	77.7	0.0	152.7	0.0	0.0	0.0	0.0	25.5	1.8	27.3	791	15.6	454	5	0	337	11.6	342	11.8	449	15.5
Total		33.9	16.2		577.6	923.2	345.6				416.2		250.0	254.7	252.3	507.0	14,714	290.9	8,442	1,095	38	6,272	216.1	7,367	253.9	7,347	253.2

Note: P - Precipitation, PE - Potential Evapotranspiration, ΔS- Change in Soil Moisture Storage, ET - Evapotranspiration  
\* Enhanced recharge volume was estimated by a continuous hydrologic model, based on the design of infiltration facility and condition of its contributing area:

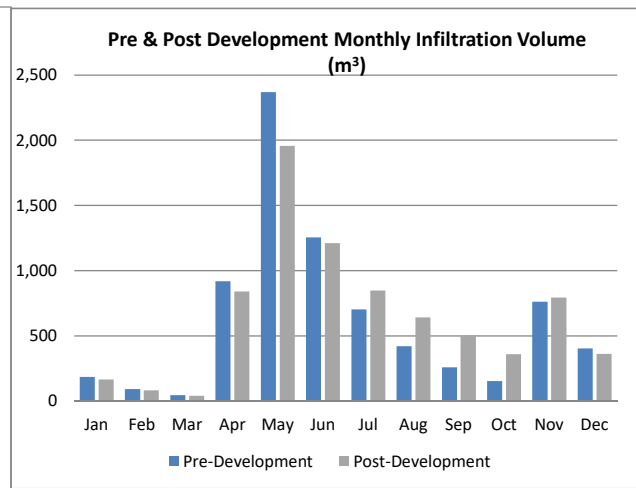
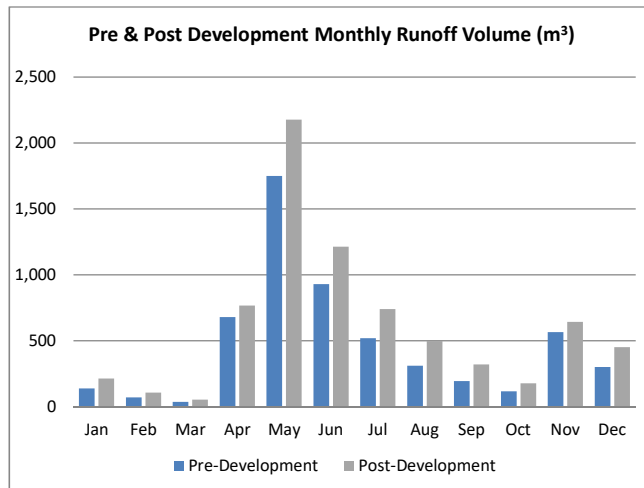
**190-216 Arkell Road**  
**SITE WATER BUDGET ANALYSIS**  
Guelph, Ontario



Project Number: 42063-104  
Date: September 29, 2021  
Design By: YXX/JNJ  
File: 104 Water Balance (Thornthwaite-Mather)  
Oct 2021.xlsx

Month	Total Runoff Volume to Weland (m <sup>3</sup> )			Total Infiltration Volume (m <sup>3</sup> )		
	Pre-development	Post-development	Difference	Pre-development	Post-development	Difference
Jan	138	212	74	186	168	-19
Feb	69	106	37	93	84	-9
Mar	34	53	19	47	42	-5
Apr	678	765	87	919	842	-76
May	1,749	2,176	427	2,370	1,956	-413
Jun	927	1,213	286	1,256	1,211	-45
Jul	519	738	219	703	848	145
Aug	311	497	187	421	643	222
Sep	192	318	126	261	504	243
Oct	115	177	62	155	360	204
Nov	564	641	77	764	796	32
Dec	299	449	150	405	363	-42
Total	5,595	7,347	1,752	7,580	7,816	236

Note: Negative sign indicate a decrease under post-development conditions.



# Appendix F

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## Geotechnical Report



**GEOTECHNICAL INVESTIGATION  
PROPOSED ARKELL ROAD SUBDIVISION  
GUELPH, ONTARIO**

**for**

**CRESCENT HOMES INC.  
c/o MTE CONSULTANTS INC.**

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1 cc: PML Kitchener

PML Ref.: 17KF002  
Report: 1  
September 28, 2018

September 28, 2018

PML Ref.: 17KF002

Report: 1

Mr. Nitin Jain  
Crescent Homes  
c/o Mr. Jason Cabral, C.E.T.  
MTE Consultants Inc.  
520 Bingemans Centre Drive,  
Kitchener, Ontario  
N2B 3X9

Dear Mr. Jain

**Geotechnical Investigation  
Proposed Arkell Road Subdivision  
Guelph, Ontario**

Peto MacCallum Ltd. (PML) is pleased to report the results of the geotechnical investigation recently completed at the above noted project site. Authorization to proceed with this assignment was provided verbally from Mr. Nitin Jain of Crescent Homes Inc., with a signed Engineering Services Agreement to be returned.

The project involves the proposed development of a residential subdivision on the north side of Arkell Road (at Summerfield Drive), in Guelph, Ontario. It is understood that the proposed development site is currently comprised of several residential dwellings, which will be demolished as part of the project. The site is approximately 2.54 ha in size, however, the northern third of the site will not be developed. The development will include 74, three storey town-house units, with associated parked areas as well as one roadway.

The purpose of the geotechnical investigation was to explore the subsurface soil and ground water conditions at the site. Based on the findings, we have prepared an engineering report with geotechnical recommendations pertaining to design and construction of the proposed residential subdivision. Specific considerations to be addressed in this report include:

- A description of the site and the field investigation procedure;
- A summary of the subsurface soil and ground water conditions encountered, including the presence of any topsoil, organic, fill or other anomalous features below grade;
- Log of borehole sheets, a borehole location plan drawing, and geotechnical laboratory test results;



- Foundation design options, including shallow foundation recommendations, bearing resistances, settlement projections and site class for seismic design;
- Slab-on-grade floor recommendations, including compaction requirements, perimeter and underfloor drainage requirements, and geotechnical suitability of onsite soils for re-use;
- Excavation recommendations, including safe side slopes and dewatering requirements,
- Pipe bedding, cover and backfill requirements, including material and compaction requirements, suitability of excavated soils for reuse as backfill;
- Ground water infiltration; and,
- Pavement design recommendations, including component thicknesses, compaction requirements, and drainage requirements.

The comments and recommendations provided in this report are based on the site conditions at the time of the investigation, and are for preliminary design purposes only. Any changes in plans will require review by PML to assess the applicability of the report, and may require modified recommendations, additional analysis and / or investigation. When the project design is complete, the general recommendations given in this report should be reviewed to ensure their applicability.

A limited chemical testing program of select soil samples was also completed. It should be noted that the scope of work did not include a Phase One or Phase Two Environmental Site Assessment (ESA), and the chemical testing program might not have identified all potential or actual occurrences of soil or ground water impairment at the site.

### **Investigation Procedure**

The field work for the geotechnical investigation was completed on February 13 and March 21, 2017. Boreholes were drilled at six locations (BH1 to BH6) as shown on the appended Borehole Location Plan, Drawing 1. The field work included the installation of a total of four monitoring wells in BH2, BH3, BH4 and BH5.

The boreholes were advanced using a Diedrich D50 track mounted drillrig equipped with an automatic hammer and continuous flight hollow stem augers. The drilling equipment was supplied and operated by specialist contractors working under subcontract to PML.





Representative samples of the overburden were recovered at regular intervals throughout the depths explored. Standard penetration tests (SPT) were carried out during sampling operations of the boreholes using conventional split spoon equipment. Ground water observations were made in the boreholes during and upon completion of drilling. The boreholes were backfilled and compacted in accordance with O.Reg.903 upon completion of drilling.

The field work was supervised throughout by a member of PML's engineering staff who directed the drilling and sampling operation, prepared the stratigraphic logs, monitored ground water conditions, and processed the recovered samples.

The borehole and monitoring well locations were established in the field by Peto MacCallum Ltd. The ground surface elevations were surveyed by MTE Consultants Inc., and provided to PML on a borehole location plan.

All soil samples collected during the investigation were returned to PML's laboratory for detailed visual examination and testing. The geotechnical testing program included natural moisture content determinations on all recovered samples and two particle size distribution analyses carried out on samples of the major soil types encountered.

### **Summarized Site and Subsurface Conditions**

The site is currently comprised of several residential dwellings, which will be demolished as part of the project. However, the northern third of the site will not be developed. The total area of the site is approximately 2.54 ha in size and relatively flat, with a gentle slope to the north, to the wetland area adjacent to the site. It is noted that the adjacent development to the east is approximately 5 m higher than the subject site.

### **Subsurface Conditions**

Reference is made to the appended Log of Borehole sheets for details of the field work including soil descriptions, inferred stratigraphy, standard penetration test (SPT) N values, ground water observations and laboratory moisture content determinations.



Due to the soil sampling procedures and the limited size of samples, the depth/elevation demarcations on the borehole logs must be viewed as "transitional" zones, and cannot be construed as exact geologic boundaries between layers.

In general, the subsurface stratigraphy encountered at the borehole locations consists of surficial topsoil and localized fill overlying cohesionless native deposits.

#### Topsoil / Topsoil Fill

Between 100 and 300 mm (average thickness of 220 mm) of dark brown silt topsoil or topsoil fill was contacted from the surface in all of the boreholes. The topsoil was typically described as damp to moist, dark brown silt, trace sand with rootlets.

#### Fill

Below the topsoil / topsoil fill in BH1 and BH6, fill was penetrated, extending to depths of 0.46 m to 0.69 m below existing grades. The fill was variable in composition, comprising either sand and gravel or silt. Occasional rootlets were observed within the fill deposits in BH6.

Within the fill, SPT N values typically between 7 and 18 blows per 0.30 m penetration of the split spoon sampler indicate that a variable degree of compaction was used to place the fill soils. The fill soils were described as damp and moist, as demonstrated by laboratory moisture contents in the range of 5 to 20%.

#### Native Deposits

Native cohesionless deposits encountered below the surficial topsoil and fill were variable and generally comprised silt / sand / sand and gravel extending to the borehole termination depths. A deposit of silt till was also contacted in BH6, extending from 5.8 to 6.6 m. Generally, the encountered native cohesionless soil deposits were compact to very dense, with typical SPT N values ranging from 10 to greater than 50 blows per 0.3 m penetration of the split spoon sampler. Localized loose / very loose zones were contacted in BH1 (between 0.46 to 0.69 m), BH2



(between 0.25 to 0.69 m) and BH3 (between 0.2 to 1.4 m). Moisture contents typically ranging between 3 and 20% were indicative of variable damp to saturated conditions, with depth.

Two soil samples of the sand / sand and gravel were collected and analyzed for particle size distribution analysis, with results presented on Figure 1 and Figure 2 attached. Based on the results, the soil classification was generally consistent with those observed during the field work as included on the appended Log of Borehole sheets.

#### Ground Water Conditions

Ground water observations carried out during and upon completion of drilling are fully summarized on the appended Log of Borehole Sheets.

Ground water was first contacted at depths of 0.7 to 2.9 m below grade in the boreholes, corresponding to elevations of 333.4 to 331.7 (metric, geodetic), respectively.

An initial water level was also taken within the monitoring wells once installed. Ground water was measured at depths of 0.75 to 3.4 m below grade in the monitoring wells, corresponding to elevations of between 333.38 and 331.8 (metric, geodetic), respectively. Follow up ground water levels by MTE Consultants Inc., completed between March, 2017 and June, 2018 measured ground water at depths of surface level (MW4) to 3.65 m below grade (MW2), corresponding to an elevation range of between 330.38 (MW2 and MW5) to 333.99 (MW4) (metric, geodetic).

Based on the ground water observations, the ground water level appears to generally slope down from north to south, away from the wetland area.

The ground water levels at the site are subject to seasonal fluctuations and precipitation patterns.

#### Discussion and Recommendations

The site is an approximately 2.54 ha, rectangular shaped piece of land which is relatively flat located on the north side of Arkell Road at Summerfield Drive, Guelph, Ontario.



It is understood that the proposed development site is currently comprised of several residential dwellings, which will be demolished as part of the project. The development will include 74, three storey town-house units, with associated parked areas as well as one roadway. However, the northern third of the site will not be developed.

Once the design details for the proposed development are finalized, the recommendations in this report should be revisited to confirm that they remain applicable.

In general, the subsurface stratigraphy encountered at the borehole locations consist of surficial topsoil and localized fill overlying cohesionless native deposits.

#### Site Grading

As noted, the site is relatively flat with a total relief of approximately 1 m. The adjacent development to the east is approximately 5 m higher than the subject site. Consideration is being given to infilling the site.

Due to the inherent variability of the existing fill materials and the lack of consistent compactive effort utilized during fill placement, these materials are not considered suitable for support of building foundations, floor slabs, pavements, or other settlement sensitive structures. Also, the loose to very loose native materials (BH1, BH2 and BH3) are not considered suitable for the support of building foundations. In this regard, all existing fill and localized very loose / loose materials should be completely subexcavated from beneath any settlement sensitive structures (i.e., building envelopes, pavements, etc.) and replaced with well compacted, suitable engineered fill materials.

Following the stripping / removal of all surficial topsoil and any other deleterious material, and approval of the subgrade, the grades may then be raised where required. Surficial topsoil / organic thicknesses across the site were typically between 100 and 300 mm. In calculating the approximate quantity of topsoil to be stripped, we recommend that the topsoil thickness shown on the individual borehole logs be increased by 50 mm to account for variations and some stripping of the mineral soil below.



Prior to any fill placement, the subgrade surface should be proofrolled with a heavy vibratory compactor under the full time supervision of qualified geotechnical personnel. Any soft spots encountered during the proofrolling process should be subexcavated to the level of competent soils.

Fill used to raise grades should comprise either on site native inorganic cut soils or approved imported material. All engineered fill materials should be pre-approved by the geotechnical consultant prior to placement. Engineered fill material should be placed in maximum 300 mm thick lifts and compacted to at least 98% standard Proctor maximum dry density (SPMDD) below footings and 95% SPMDD below floors and pavements. Further, generic recommendations for fill subgrade preparation and engineered fill construction are provided in Appendix A.

It is noted that materials generated from grade cuts will generally consist of native cohesionless soil deposits. In general, the native on site cohesionless soils will be suitable for reuse as engineered fill, subject to geotechnical verification during construction, providing all organic, wet or saturated soils, and otherwise deleterious soils are discarded. Silty soils described as wet or saturated on the borehole logs should be dried prior to reuse.

The silty soils (i.e. silt) are frost susceptible and highly susceptible to moisture content variations, and are not well suited for engineering fill construction. Compaction to 98% SPMDD may be difficult to achieve; however, these insitu soils should be acceptable for use as engineered fill where compaction to 95% SPMDD is specified.

### Foundations

For preliminary design purposes, conventional strip / spread footings founded at least 0.30 m into the competent compact to dense native deposits, or on engineered structural fill compacted to 98% SPMDD, may be designed for a net bearing resistance of 150 kPa at the serviceability limit state (SLS) and a factored bearing resistance of 225 kPa at the ultimate limit state (ULS). If very loose / loose soils are contacted at the proposed footing level, the loose soils should be subexcavated to the level of competent founding soils.



Accordingly, footings designed in accordance with the Ontario Building Code for residential housing will be satisfactory. The following table summarizes the minimum foundation depths based on the borehole findings:

LOCATION	MINIMUM FOUNDATION DEPTH (m)	CORRESPONDING ELEVATION (METRIC, GEODETIC)
BH1	1.0	333.50
BH2	1.0	334.10
BH3	1.7	332.70
BH4	0.6	333.50
BH5	0.6	334.30
BH6	0.7	333.30

Although in general, footings are anticipated to be placed on native insitu soils, where required the footings may be supported on engineered structural fill, placed in accordance with the generic recommendations for engineered fill construction provided in Appendix A. Prior to placement of engineered fill, all existing fill must be removed and the soils should be subexcavated to the level of competent native overburden soils noted in the table above. For engineered fill supporting footing loads, compaction to a minimum 98% of the materials SPMDD, should be specified as per recommendations outlined in the preceding 'Site Grading' section of this report and in Appendix B.

Footings supported on the structural fill may also be designed using the values for a net factored resistance at ULS and SLS of 225 and 150 kPa, respectively. Full time inspection of any structural fill placement by PML personnel is recommended to approve subgrade conditions, fill materials and to verify that the specified compaction levels are being achieved. Prior to concrete placement, all founding surfaces should be examined by PML personnel to check the competency of the founding surfaces.



Total settlements of footings founded on the approved engineered fill or compact to dense native overburden deposits, designed as outlined above are not expected to exceed 25 mm, with differential settlements between footings being no more than 50% of this value.

All exterior footings should be provided with a minimum 1.2 m of earth cover or the thermal insulation equivalent to provide adequate insulation against potential frost damage. A 25 mm thick layer of polystyrene insulation is thermally equivalent to 600 mm of soil cover.

Prior to concrete placement, all founding surfaces should be examined by PML personnel to check the competency of the founding surfaces.

For earthquake design, a site Class D seismic response classification may be assumed, in accordance with the 2012 Ontario Building Code.

#### Basement / Slab-on-Grade Floor Slabs

In general, the ground water level at the site was first contacted below depths of 0.7 to 2.9 m (Elevation 333.4 to 331.7) with follow up ground water monitoring showing ground water depths of surface level to 3.65 m below grade (Elevation 330.38 to 333.99). Basements, if any, must be located at least 1.0 m above the high ground water level. Conventional slab-on-grade construction of basement floor slabs is feasible on compact to dense native soil deposits, or on engineered structural fill compacted to 95% SPMDD.

Preparation of the floor slab subgrade should include stripping of the topsoil, and other deleterious material followed by proofrolling of the exposed subgrade with a heavy roller to ensure uniform adequate support. Excessively loose, soft or compressible materials revealed during the proofrolling operations should be subexcavated and replaced with well compacted approved material.

Fill placed under the floor slab to achieve finished subgrade levels or as foundation excavation backfill should comprise approved inorganic material having a moisture content within 3% of the optimum value, placed in maximum 200 mm thick lifts, and compacted to at least 95% of SPMDD.



A minimum 150 mm thick layer of well compacted clear stone (or equivalent) is recommended directly beneath the slab-on-grade. A polyethylene vapour barrier should be placed at the surface of the stone if a moisture sensitive finish is to be placed on the floor.

For slab-on-grade (basement less) structures, exterior grades should be maintained at least 150 mm below the finished floor slab-on-grade level and sloped to promote drainage away from the building.

#### Foundation Drainage and Earth Pressure Parameters

Foundation drainage measures should be taken for units with basements. Perforated drainage pipe should be laid around the outside edge of the footings, and connected to a frost free sump system. It is recommended that the drainage pipes be surrounded with a granular filter protected with filter fabric, or alternatively wrapped with filter cloth and surrounded by concrete sand.

A “free draining” granular material, or an equivalent, approved drainage board product must be provided for the basement walls, in accordance with the Ontario Building Code. The onsite native cohesionless deposits may be suitable for use as basement wall backfill. However, it should be noted that soils with high silt content (i.e. silt) are not suitable for use as basement wall backfill unless a drainage board product is provided. Backfilling should not take place until the ground floor has been constructed, in order to provide lateral support for the wall.

In conjunction with the granular material, a weeping tile system should be installed to minimize the build-up of hydrostatic pressure behind the wall. The weeping tile should be surrounded by a properly designed graded granular filter or wrapped with approved geotextile to prevent migration of fines into the system. The drainage pipe should be placed on a positive grade and lead to a frost-free sump or outlet.

The following earth pressure design parameters may be assumed for calculation of backfill materials compacted to 95% SPMDD:





PARAMETER	OPS GRANULAR B	Onsite SAND / SAND AND GRAVEL
Angle of Internal Friction (degrees)	32	30
Unit Weight (kN/m <sup>3</sup> )	21	20
Coefficient of Active Earth Pressure ( $K_a$ )	0.30	0.33
Coefficient of Earth Pressure At Rest ( $K_o$ )	0.47	0.50
Coefficient of Passive Earth Pressure ( $K_p$ )	3.23	2.77

Note: Earth pressure coefficients assume Rankin analysis (wall friction ignored, non-sloping backfill)

It is assumed that basement floors will be more than 1.0 m above the ground water table and as such, underfloor drainage systems will not be required.

#### Excavation and Dewatering

It is assumed that excavations for site grading, footings and service trenches will extend through the surficial topsoil and into the native cohesionless soils, which are classified as Type 3 materials as defined in the Occupational Health and Safety Act (OHSA). Subject to inspection and providing adequate ground water control is achieved, excavations within Type 3 soils that are to be entered by workers should be inclined from the base of the excavation at one horizontal to one vertical (1H:1V) or flatter.

Ground water was first contacted at depths of 0.7 to 2.9 m below grade in the boreholes, corresponding to elevations of 333.4 to 331.7 (metric, geodetic), respectively.

An initial water level was also taken within the monitoring wells once installed. Ground water was measured at depths of 0.75 to 3.4 m below grade in the monitoring wells, corresponding to elevations of between 333.38 and 331.8 (metric, geodetic), respectively. Follow up ground water levels by MTE Consultants Inc., completed between March 2017 and June 2018 showed ground water depths of surface level to 3.65 m below grade (Elevation 330.38 to 333.99). The extent of ground water control will depend on the depth of excavation below the ground water level.



Shallow excavations extending less than 0.5 m below the ground water level can be dewatered using conventional sump pumping techniques. Deeper excavations, extending more than 0.5 m below the ground water level may require extensive ground water control measures such as keg wells or well point dewatering. The actual dewatering methods should be established at the contractor's discretion within the context of a performance specification for the project. Regardless of the dewatering method chosen, the hydraulic head and ground water inflow must be properly controlled to ensure a stable and safe excavation and to facilitate construction. The design of the dewatering system should be specified to maintain and control ground water at least 0.30 m below the excavation base level, in order to provide a stable excavation base throughout construction.

It should be noted that under the Ontario Water Resources Act, the Water Taking and Transfer Regulation 387/04, and in compliance with the Ministry of Environment and Climate Change's (MOECC) policy and Permit to Take Water (PTTW) Manual (April 2005), an application should be filed to the MOECC for the subject project construction dewatering PTTW, if the dewatering discharge is greater than 400,000 L/day, or about 4.6 L/s. If the dewatering discharge is between 50,000 L/day (or about 0.6 L/s) and 400,000 L/day (or about 4.6 L/s), dewatering activities need to be registered on the Environmental Activity and Sector Registry (EASR). Reference is made to the hydrological report by MTE Consultants Inc. for further details.

At the time of tendering, test pits should be excavated on site to allow prospective Contractors to judge the ground water conditions and to determine the appropriate control methods required closer to the time of construction. Ground water conditions are subject to seasonal variations. In this regard, a later summer construction schedule would be preferable.

#### Pipe Bedding and Backfilling

No bearing problems are anticipated for pipes founded in the native cohesionless soils or structural fill. On stable subgrade, a minimum 150 mm thick bedding course of Granular A material compacted to 95% SPMDD is recommended beneath the pipes. The Granular A material should extend around the pipe to at least 300 mm above the pipe obvert or as set out by Ontario Provincial Standards (OPS), or the local authority.



Backfill below pavements, floor slabs and other settlement sensitive features should be similarly compacted to 95% SPMDD. Backfill should be placed in 300 mm maximum lifts. Material that is too wet for compaction to a minimum of 95% SPMDD should be allocated for use in landscaped / non settlement sensitive locations, and compacted to at least 90% SPMDD.

The trenching and backfilling operations should be carried out in a manner which minimizes the length of trench left open yet accommodates efficient pipe laying and compaction activities.

#### Pavement Construction

Prior to the construction of the new pavements, surficial topsoil, fill and loose to very loose deposits should be removed. If some settlement is acceptable, the loose to very loose soils can remain in place. Based on the anticipated traffic patterns, frost susceptibility, and strength of the expected subgrade soils, the following pavement component thicknesses are considered suitable for local residential and parking lot traffic categories (no truck / heavy vehicle use).

<b>PAVEMENT COMPONENT</b>	<b>THICKNESS (mm)</b>
Asphalt	80
Granular A Base	150
Granular B Subbase	350

The flexible pavement designs provided above consider that construction will be carried out during the drier time of the year and the subgrade is stable, as determined by proofrolling inspected by PML personnel. If the subgrade is wet and unstable, additional granular subbase will be required.

The pavement materials should conform to current OPS specifications. The Granular A base and Granular B subbase courses should be placed in thin lifts and compacted to a minimum of 100% SPMDD, and asphalt should be placed to a minimum of 92% of the material's maximum relative density (MRD). Reference is made to OPS Specification 310, as revised.



During construction, testing should be conducted to confirm the gradation and compactibility characteristics of the granular base and subbase materials and the mix design properties of the asphalt.

Proofrolling procedures and the placement and compaction of all the fill and granular materials and asphalt for the pavement construction and backfilling at the site should be inspected on a continuous basis by PML technicians.

If relatively impermeable silty soils are present at a shallow depth beneath the pavement structure, pavement subdrains should be provided to prevent water accumulation on the pavement subgrade surface. The subgrade should be graded so that water is directed to the catch basin structures or to the pavement edge. Subdrains should be discharged in to the catch basins. The subdrains may consist of filter wrapped, 100 mm diameter perforated plastic pipe, set within the subbase layer at the subgrade surface.

#### Soil Infiltration

Soil infiltration rates for storm water management (SWM) and roof water infiltration systems were determined for the major near surface soil units and are as follows:

SOIL TYPE	ESTIMATED COEFFICIENT OF PERMEABILITY (cm/sec)	INFILTRATION RATE (mm/hr)
Sand / Sand and Gravel	$1 \times 10^{-3}$	30

Any SWM ponds should be inspected by PML personnel during construction to verify the presence of a suitable subgrade. In general, the slopes of the storm water management pond should be constructed at 5H:1V or shallower and be provided with vegetation cover to minimize the potential for erosion and sloughing of the side slopes.



### **Limited Chemical Testing Program**

As noted, a limited chemical testing program was completed on samples recovered during geotechnical investigation. PML understands that excess soil may be generated during construction, the volume of which is unknown at this time. The chemical testing program was completed to check the geoenvironmental quality of the site soils at selected sampling locations in order to provide commentary regarding on site or off site re-use and / or disposal options of potentially excess soils.

The soil sampling and testing was conducted as a limited testing program. A Phase One Environmental Site Assessment (ESA) was not within the scope of work for this assignment. Accordingly, soil and ground water impairment that has not been identified by the limited chemical testing program may exist elsewhere at the site. The limited chemical testing program does not constitute an ESA as defined under the Environmental Protection Act and O. Reg. 153/04, as amended.

### **Chemical Testing Protocol**

Representative samples collected during the geotechnical investigation were returned to our laboratory for detailed visual examination. Soil samples were submitted for chemical analysis to AGAT Laboratories Limited (AGAT), a Canadian Association for Laboratory Accreditation Inc. (CALA) accredited laboratory in Mississauga, Ontario. The chemical analyses conducted by AGAT were in accordance with the O. Reg. 153/04, as amended Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act dated March 9, 2004, amended as of July 1, 2011.

As part of the geoenvironmental procedural protocol, all recovered soil samples were examined for visual and olfactory evidence of potential contamination. In addition, soil vapour concentrations (SVCs) were measured in the headspace of the recovered samples. The measured SVCs were typically 0 to 5 parts per million, which are not considered significant.



Five soil samples were submitted for chemical analysis for metals and inorganic parameters, and two samples were submitted for analysis for organochlorine (OC) pesticides. Selection of samples was based on visual and olfactory indications of contamination, SVCs and for general coverage. Details of the samples submitted for chemical testing are as follows:

SAMPLE ID	BOREHOLE	SAMPLE NUMBER	DEPTH (m)	SOIL TYPE	PARAMETERS TESTED
BH4 SS1	1	1	0 to 0.6	Topsoil	M&I and OC pesticides
BH5 SS1	5	1	0 to 0.6	Topsoil	M&I
BH5 SS4	5	4	2.3 to 2.7	Native	M&I
BH6 SS1	6	1	0 to 0.6	Topsoil / Fill	M&I and OC pesticides
BH6 SS3	6	3	1.5 to 2.1	Native	M&I

### **Site Condition Standards**

The Ministry of the Environment, Conservation and Parks (MECP) has developed a set of Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act (April 15, 2011) and O.Reg. 153/04, as amended. The standards consist of nine tables (Table 1 through Table 9) that provide criteria for maximum concentrations of various contaminants. In general, the applicable Table and corresponding Site Condition Standards (SCSs) depend on the site location, land use, soil texture, bedrock depth, soil pH and source of potable water at the site.

The site is currently comprised of several residential dwellings and it is to be developed into a residential subdivision. The site is bordered by the Torrance Creek Wetland Complex to the north, which is a provincially significant wetland as identified by the Ministry of Natural Resources. Based on review of the above factors, PML selected the Generic Criteria of the O.Reg. 153/04, Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act dated April 15, 2011. In particular, the Table 1 (T1) Full Depth Background Site Condition Standards for Residential / Parkland / Institutional / Industrial / Commercial / Community (RPI/ICC) property use would likely apply to the site; however a full evaluation of applicable SCSs in accordance with Sections 41 and 43.1 of O.Reg. 153/04, as amended, was not within the scope of this assignment and further environmental work would be required to confirm this.



For off site re-use with minimal environmental restrictions, the O.Reg. 153/04, as amended, Full Depth Background Table 1 (T1) SCSs for RPI/ICC property uses were utilized. In addition, the Full Depth Generic SCSs (T2) in a Portable Ground Water Condition for ICC property use are also examined.

It is noted that a comparison to the Table 3 SCSs for full depth generic condition, Tables 4 and 5 SCSs for stratified site condition, Tables 6 and 7 SCSs for shallow bedrock condition and Table 8 and Table 9 for use within 30 m of a water body for a non-potable ground water condition were not conducted as part of this assignment. If the potential receiving site for excess soil falls within one of these categories, additional evaluation by PML will be required to confirm conformance.

### **Analytical Findings and Conclusions**

Laboratory certificates of analysis compared to the Table 1 and 2 SCSs are included in Appendix C. The measured values and corresponding Standards (labelled as G/S for Guideline / Standard) are shown on the certificates of analysis. In the event of an exceedance of the SCSs, the level is shown in **bold** text, where applicable.

#### **On Site Re-use**

Based on the results of chemical testing, the measured concentrations of the tested parameters met the T1 RPI/ICC SCSs, with the exception of zinc in two samples (BH5 SS1 and BH6 SS3).

It is noted that there is no legal imperative to remove or treat the soil that exceeds the applicable SCSs, provided it is demonstrated that there is no off site impact or adverse effect. However, if contaminated soil is left on site, the landowner assumes liability associated with the contamination. The liability concerns could include potential scrutiny from the MECP, neighbouring property owners and the public; potential for decreased value of the land and issues during potential divesting of the property due to environmental liability concerns on the part of future owners or their financiers/insurers.



### Off Site Re-use

As noted, the measured concentrations of the tested parameters met the T1 RPI/ICC SCSs, with the exception of zinc in two samples (BH5 SS1 and BH6 SS3). When compared to the T2 ICC Standards, one sample (BH6 SS3) exceeded the SCS for zinc.

If the soil is to be removed from the site for off site re-use, the following conditions must be met:

- The extent of the material that exceeds the applicable SCSs is delineated;
- All analytical results and environmental assessment reports must be fully disclosed to the receiving site owners / authorities and they have agreed to receive the material;
- The work must be completed in accordance with local by-laws governing soil movement and/or placement at other sites;
- The applicable SCSs for the receiving site have been determined, as confirmed by the environmental consultant and the SCSs are consistent with the chemical quality of the soil originating at the source site;
- Transportation and placement of the excess soil is monitored by the environmental consultant to check the material is appropriately placed at the pre-approved site;
- The excess soil cannot be taken to a property for which a RSC is being filed as outlined in O.Reg. 153/04, as amended, unless the chemical testing program is completed in accordance with the regulation;
- The excess soil cannot be taken to a property for which a RSC has been previously filed unless the soil quality meets the SCSs contained in the RSC;
- The receiving site must be arranged and/or approved well in advance of excavation in order to avoid delays during construction. As well, it is noted the chemical testing requirements for various receiving sites is site-specific and additional testing may be required, beyond that provided in this report; and





- The excavation work should be conducted in accordance with a Soil Management Plan prepared by a qualified professional to ensure that all surplus excavated material is tested and managed appropriately, and that imported fill material is of suitable quality and meets the SCSs applicable to the site. Re-use of excess excavated soil on site is also subject to acceptance for re-use by the geotechnical consultant at the time of construction based on geotechnical considerations.

If landfill disposal of excess soils is considered, PML recommends toxicity characteristic leaching procedure (TCLP) testing be completed in accordance with O. Reg. 347/558, Schedule 4, as amended.

It is recommended that transportation of fill material from the Source Site(s) to the Receiving Site(s) be carried out in accordance with the MECP document Management of Excess Soil – A guideline for Best Management Practices dated January, 2014.

Additional sampling and chemical testing should be carried out during construction to verify the chemical quality of the excess soil to assess the appropriate management/disposal options for the soil leaving the site.

It should be noted that the soil conditions may differ from those encountered during this assignment. PML should be contacted if impacted soil conditions become apparent to further assess and appropriately handle the materials, if any, and to evaluate whether modifications to the conclusions documented in this report are necessary.

#### Geotechnical Review and Construction Inspection and Testing

It is recommended that the design drawings be submitted to PML for general geotechnical review for compatibility with the site conditions and recommendations of this report.



Earthworks operations should be carried out under the supervision of PML to approve subgrade preparation, backfill materials, placement and compaction procedures, and verify the specified degree of compaction is achieved uniformly throughout fill materials.

The comments and preliminary recommendations provided in this report are based on the information revealed in the boreholes. Conditions away from and between boreholes may vary. Geotechnical review during construction should be on going to confirm the subsurface conditions are substantially similar to those encountered in the boreholes, which may otherwise require modification to the original recommendations.

### **Closure**

This assignment is subject to the Statement of Limitations that is included in Appendix B and must be read in conjunction with this report.

We trust this report has been completed within our terms of reference, and is sufficient for your immediate requirements. If you have any questions or require further information, please do not hesitate to contact our office.



Sincerely

Peto MacCallum Ltd.

Hassen Shinwary, BASc  
Project Supervisor  
Geotechnical and Geoenvironmental Services

Ken Hanes, P.Eng.  
Project Engineer  
Geotechnical and Geoenvironmental Services

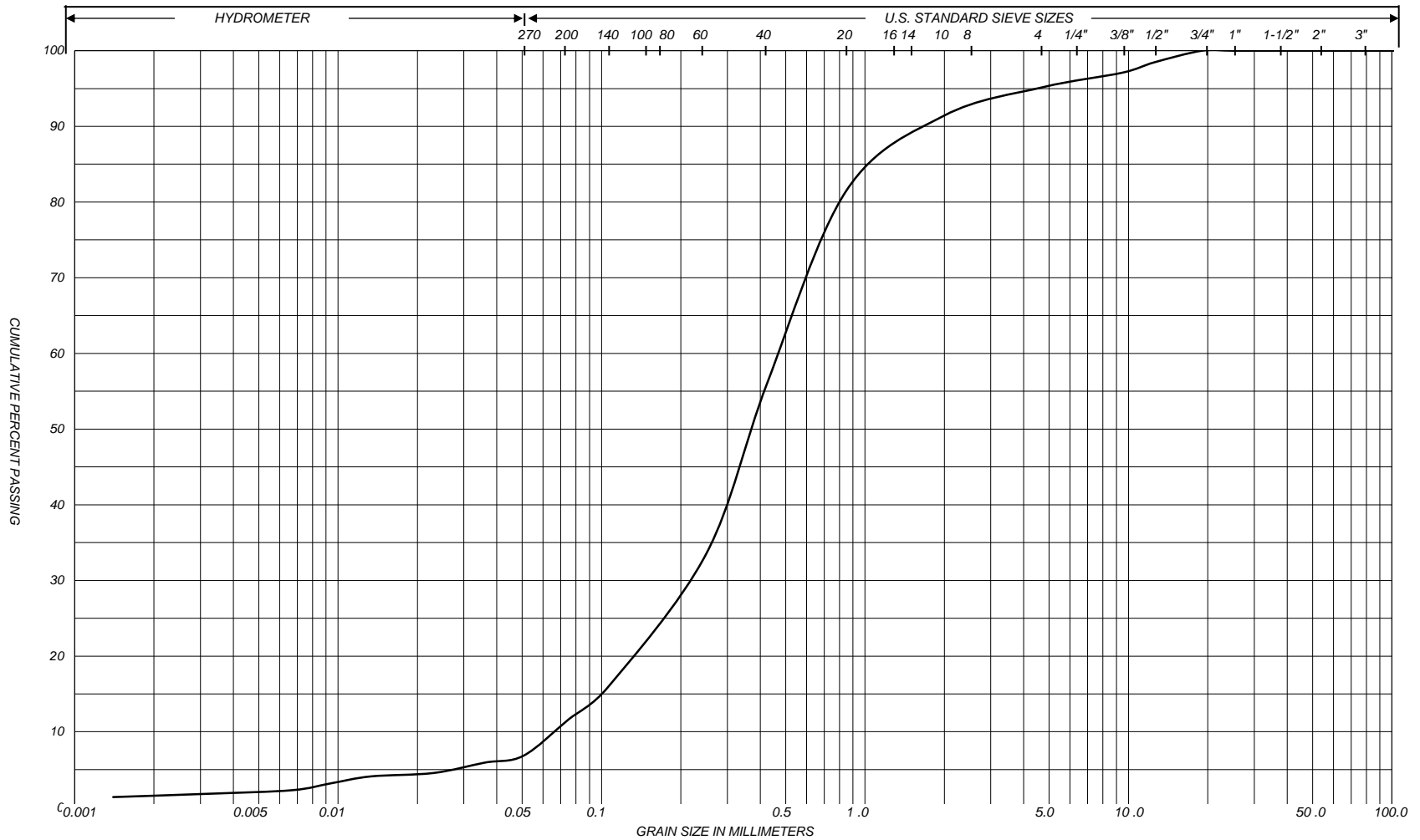
Gerry Mitchell, MEng, P.Eng.  
Vice President

HS/KH:sh

Enclosures:

Figures 1 to 2 – Particle Size Distribution Charts  
List of Abbreviations  
Log of Boreholes 1 to 6  
Drawing 1 – Borehole Location Plan  
Appendix A – Engineered Fill  
Appendix B – Statement of Limitations  
Appendix C – AGAT Certificates of Analysis

## PARTICLE SIZE DISTRIBUTION CHART

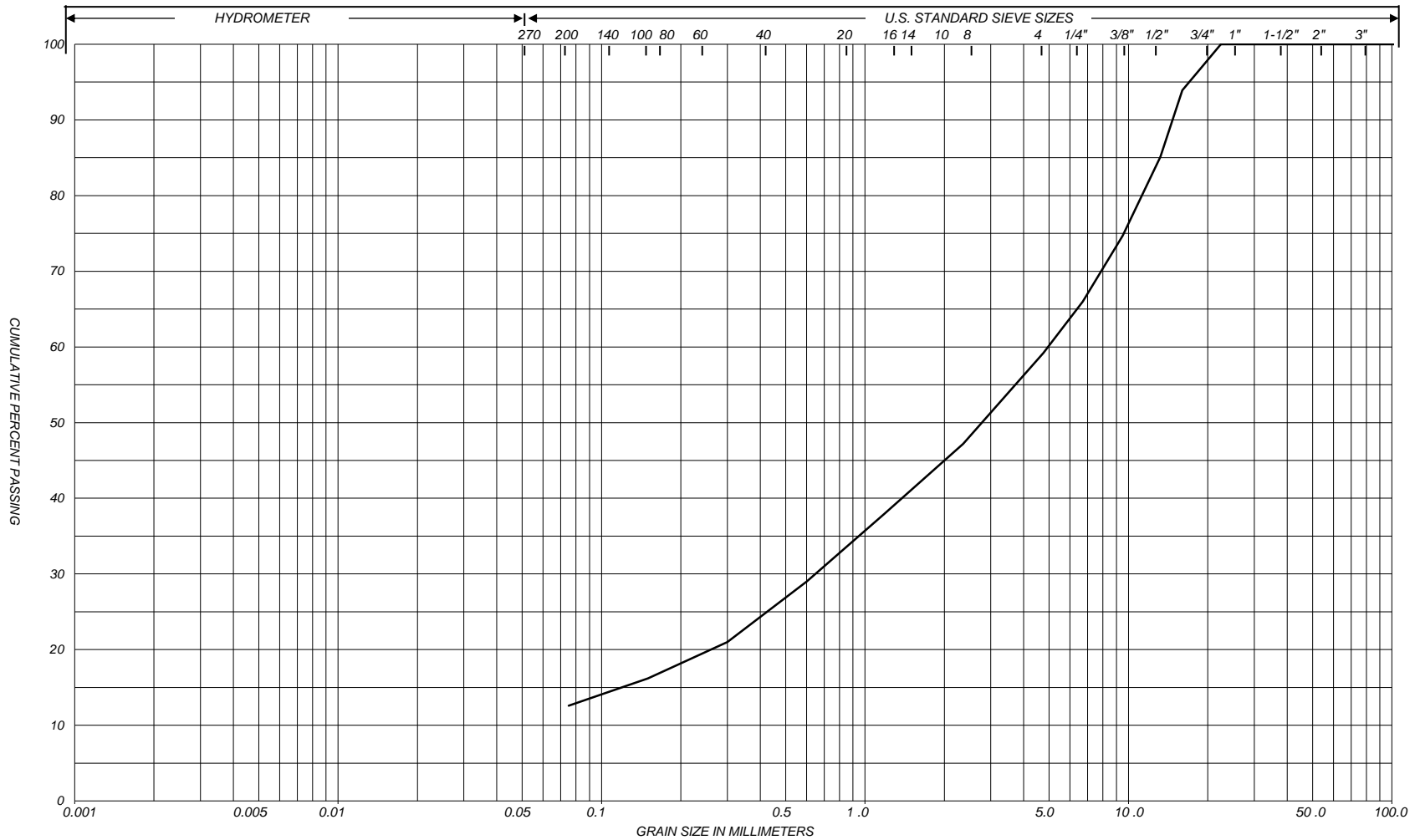


SILT & CLAY				FINE		MEDIUM		COARSE	GRAVEL			COBBLES	UNIFIED		
				SAND											
CLAY	FINE		MEDIUM		COARSE	FINE		MEDIUM		COARSE	GRAVEL			COBBLES	M.I.T.
	SILT				SAND										
CLAY		SILT			VERY FINE	FINE		MEDIUM	COARSE	GRAVEL					U.S. BUREAU
					SAND										

REMARKS Borehole 1, Sample SS6, Depth 4.5 to 5.0 m

SAND, SOME SILT, TRACE GRAVEL

## PARTICLE SIZE DISTRIBUTION CHART



SILT & CLAY					FINE		MEDIUM		COARSE	GRAVEL			COBBLES	UNIFIED		
CLAY	FINE		MEDIUM		COARSE	FINE		MEDIUM		COARSE		GRAVEL			COBBLES	M.I.T.
	SILT					FINE		SAND								
CLAY		SILT			VERY FINE	FINE	MEDIUM	COARSE	GRAVEL						U.S. BUREAU	
					SAND											

REMARKS Borehole 2, Sample SS2, Depth 0.7 to 1.2 m

SAND AND GRAVEL, SOME SILT

# LIST OF ABBREVIATIONS



## PENETRATION RESISTANCE

Standard Penetration Resistance N: - The number of blows required to advance a standard split spoon sampler 0.3 m into the subsoil. - Driven by means of a 63.5 kg hammer falling freely a distance of 0.76 m.

Dynamic Penetration Resistance: The number of blows required to advance a 51 mm, 60 degree cone, fitted to the end of drill rods, 0.3 m into the subsoil. The driving energy being 475 J per blow.

## DESCRIPTION OF SOIL

The consistency of cohesive soils and the relative density or denseness of cohesionless soils are described in the following terms:

<u>CONSISTENCY</u>	<u>N (blows/0.3 m)</u>	<u>c (kPa)</u>	<u>DENSENESS</u>	<u>N (blows/0.3 m)</u>
Very Soft	0 - 2	0 - 12	Very Loose	0 - 4
Soft	2 - 4	12 - 25	Loose	4 - 10
Firm	4 - 8	25 - 50	Compact	10 - 30
Stiff	8 - 15	50 - 100	Dense	30 - 50
Very Stiff	15 - 30	100 - 200	Very Dense	> 50
Hard	> 30	> 200		
WTPL	Wetter Than Plastic Limit			
APL	About Plastic Limit			
DTPL	Drier Than Plastic Limit			

## TYPE OF SAMPLE

SS	Split Spoon	TW	Thinwall Open
WS	Washed Sample	TP	Thinwall Piston
SB	Scraper Bucket Sample	OS	Oesterberg Sample
AS	Auger Sample	FS	Foil Sample
CS	Chunk Sample	RC	Rock Core
ST	Slotted Tube Sample	USS	Undisturbed Shear Strength
PH	Sample Advanced Hydraulically	RSS	Remoulded Shear Strength
PM	Sample Advanced Manually		

## SOIL TESTS

Qu	Unconfined Compression	LV	Laboratory Vane
Q	Undrained Triaxial	FV	Field Vane
Qcu	Consolidated Undrained Triaxial	C	Consolidation
Qd	Drained Triaxial		

## LOG OF BOREHOLE NO. 1

**PROJECT** Proposed Arkell Road Subdivision

**LOCATION** Arkell Road, Guelph, Ontario

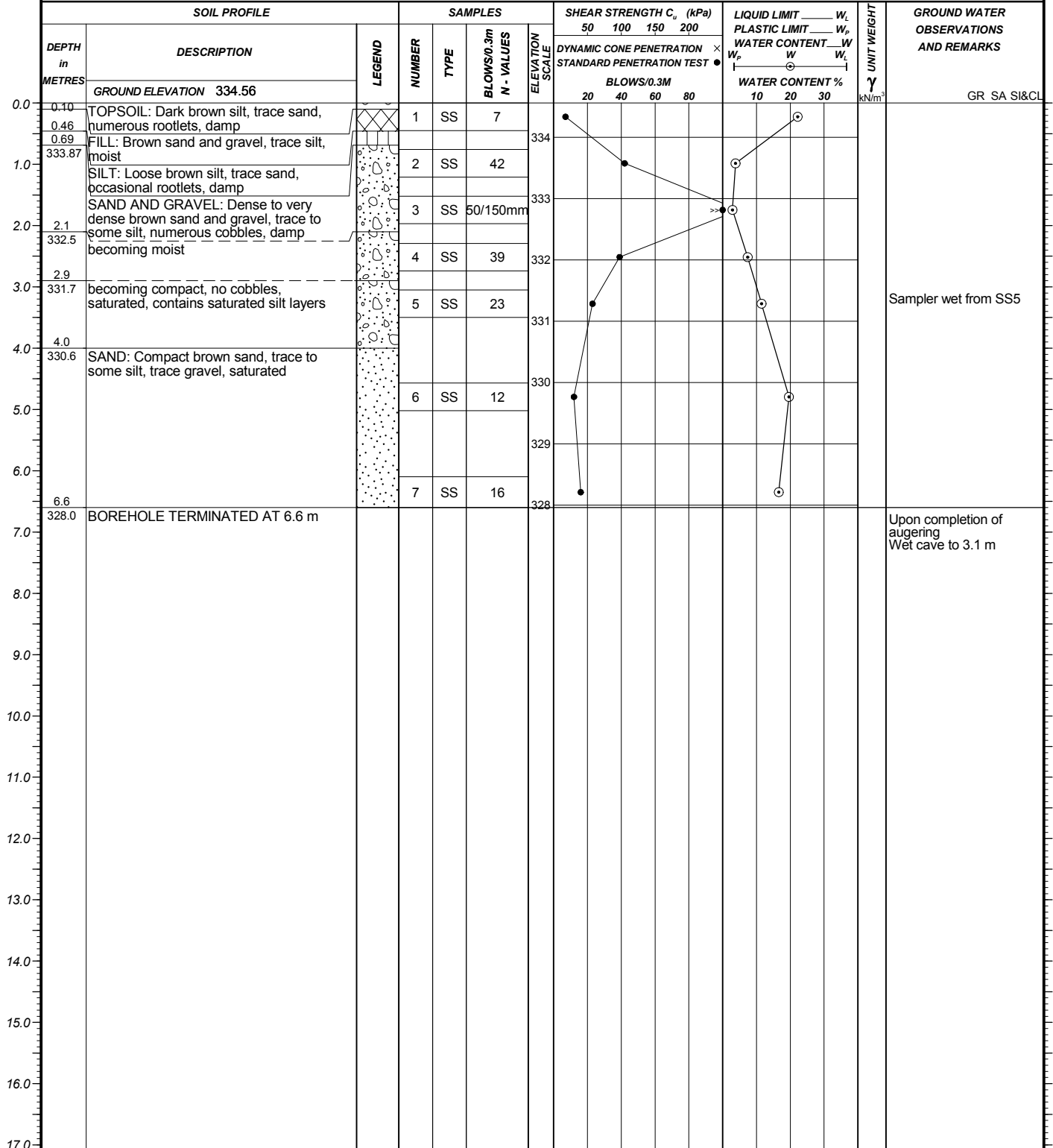
**BORING METHOD** Continuous Flight Hollow Stem Augers

**BORING DATE:** 2017 02 13

**PML REF.:** 17KF002

**ENGINEER** K. Hanes

**TECHNICIAN** H. Shinwary



**NOTES:** Headspace: SS1 0ppm, SS2 0ppm, SS3 0ppm, SS4 0ppm, SS5 0ppm, SS6 0ppm, SS7 0ppm

WATER LEVEL OBSERVED DURING / UPON COMPLETION OF DRILLING  
WATER LEVEL MEASURED IN MONITORING WELL

UNDISTURBED FIELD VANE  
REMOLDED FIELD VANE  
LAB SHEAR TEST  
POCKET PENETROMETER  
POCKET TORVANE  
CHECKED BY KH

## LOG OF BOREHOLE NO. 2

**PROJECT** Proposed Arkell Road Subdivision

**LOCATION** Arkell Road, Guelph, Ontario

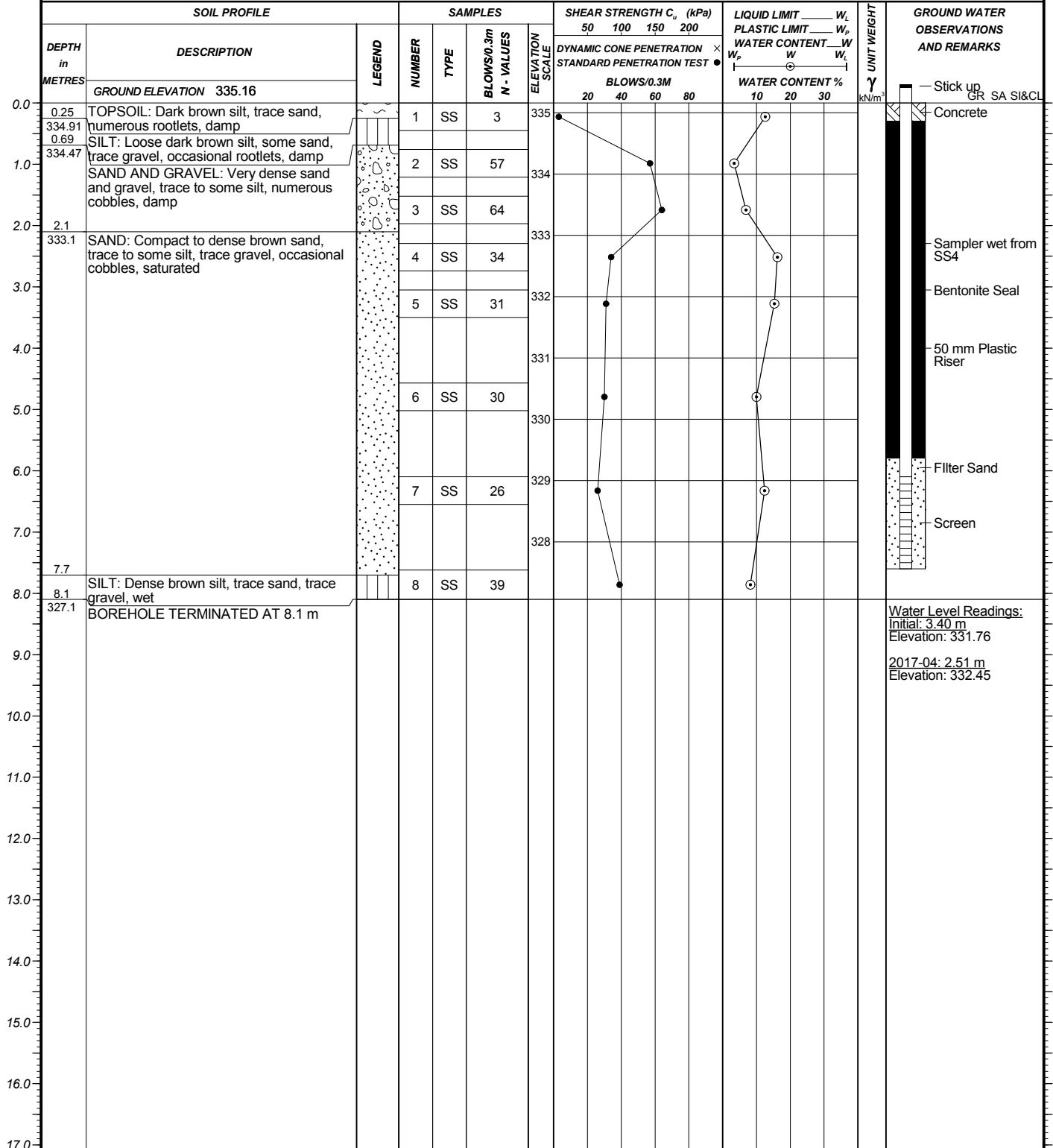
**BORING METHOD** Continuous Flight Hollow Stem Augers

**BORING DATE:** 2017 02 13

**PML REF.:** 17KF002

**ENGINEER** K. Hanes

**TECHNICIAN** H. Shinwary



**NOTES:** Headspace: SS1 0ppm, SS2 0ppm, SS3 0ppm, SS4 0ppm, SS5 5ppm, SS6 0ppm, SS7 0ppm, SS8 0ppm

WATER LEVEL OBSERVED DURING / UPON COMPLETION OF DRILLING  
WATER LEVEL MEASURED IN MONITORING WELL

UNDISTURBED FIELD VANE  
REMOLDED FIELD VANE  
LAB SHEAR TEST  
POCKET PENETROMETER  
POCKET TORVANE  
CHECKED BY KH



## LOG OF BOREHOLE NO. 3

**PROJECT** Proposed Arkell Road Subdivision

**LOCATION** Arkell Road, Guelph, Ontario

**BORING METHOD** Continuous Flight Hollow Stem Augers

**BORING DATE:** 2017 02 13

**PML REF.:** 17KF002

**ENGINEER** K. Hanes

**TECHNICIAN** H. Shinwary

SOIL PROFILE		LEGEND	SAMPLES			SHEAR STRENGTH $C_u$ (kPa)		LIQUID LIMIT $W_L$		UNIT WEIGHT $\gamma$ kN/m <sup>3</sup>	GROUND WATER OBSERVATIONS AND REMARKS
DEPTH in METRES	DESCRIPTION		NUMBER	TYPE	BLOWS/0.3m N - VALUES	ELEVATION SCALE	50 100 150 200	PLASTIC LIMIT $W_p$	WATER CONTENT $W$		
	GROUND ELEVATION 334.42										
0.0	0.20 TOPSOIL: Dark brown silt, trace sand, numerous rootlets, moist		1	SS	5	334					Stick up
0.51	333.91 SILT: Loose dark brown silt, some sand, occasional rootlets		2	SS	3	334					Concrete
1.0	1.4 SAND AND GRAVEL: Very loose brown sand and gravel, trace to some silt, occasional cobbles, damp		3	SS	35	333					Sampler wet from SS3
2.0	333.1 becoming compact, saturated		4	SS	31	332					Bentonite Seal
2.9	331.5 SAND: Compact to dense brown sand, trace to some silt, trace gravel, occasional cobbles, saturated		5	SS	24	331					50 mm Plastic Riser
3.0						330					
4.0						329					
5.0			6	SS	42	329					
6.0						328					Filter Sand
7.0			7	SS	27	328					Screen
7.8						327					
8.0	8.0 SILT: Compact brown silt, trace sand, trace gravel, wet		8	SS	21	327					
8.0	326.4 BOREHOLE TERMINATED AT 8.0 m										Water Level Readings: Initial: 1.95 m Elevation: 332.47 2017-04: 1.86 m Elevation: 332.56
9.0											
10.0											
11.0											
12.0											
13.0											
14.0											
15.0											
16.0											
17.0											

**NOTES:** Headspace: SS1 0ppm, SS2 0ppm, SS3 0ppm, SS4 0ppm, SS5 5ppm, SS6 0ppm, SS7 5ppm, SS8 0ppm

WATER LEVEL OBSERVED DURING / UPON COMPLETION OF DRILLING  
WATER LEVEL MEASURED IN MONITORING WELL

UNDISTURBED FIELD VANE  
REMOLDED FIELD VANE  
LAB SHEAR TEST  
POCKET PENETROMETER  
POCKET TORVANE  
CHECKED BY KH



## LOG OF BOREHOLE NO. 5

**PROJECT** Proposed Arkell Road Subdivision

**LOCATION** Arkell Road, Guelph, Ontario

**BORING METHOD** Continuous Flight Hollow Stem Augers

**BORING DATE:** 2017 03 21

**PML REF.:** 17KF002

**ENGINEER** K. Hanes

**TECHNICIAN** H. Shinwary

SOIL PROFILE		LEGEND	SAMPLES			SHEAR STRENGTH $C_u$ (kPa)		LIQUID LIMIT $W_L$		UNIT WEIGHT $\gamma$ kN/m <sup>3</sup>	GROUND WATER OBSERVATIONS AND REMARKS
DEPTH in METRES	DESCRIPTION		NUMBER	TYPE	BLOWS/0.3m N-VALUES	ELEVATION SCALE	50 100 150 200	PLASTIC LIMIT $W_p$	WATER CONTENT $W$		
	GROUND ELEVATION 334.97										
0.0	0.25 TOPSOIL: Dark brown silt, trace sand, trace gravel, numerous rootlets, damp		1	SS	13						
334.72											
1.0	SAND AND GRAVEL: Dense brown sand and gravel, trace to some silt, numerous cobbles, damp		2	SS	49	334					
2.0			3	SS	31	333					
2.2											
332.8	SAND: Compact brown sand, some gravel, trace to some silt, occasional cobbles, saturated		4	SS	24	332					
3.0			5	SS	27	331					
4.0											
5.0			6	SS	14	330					
5.6											
329.4	becoming very dense		7	SS	51	329					
6.0											
7.0											
7.1											
327.9	SAND AND GRAVEL: Very dense brown sand and gravel, trace silt, numerous cobbles, saturated		8	SS	52	327					
8.0											
8.1	BOREHOLE TERMINATED AT 8.1 m										
326.9											
9.0											
10.0											
11.0											
12.0											
13.0											
14.0											
15.0											
16.0											
17.0											

**NOTES:** Headspace: SS1 0ppm, SS2 0ppm, SS3 5ppm, SS4 5ppm, SS5 0ppm, SS6 0ppm, SS7 0ppm, SS8 0ppm

WATER LEVEL OBSERVED DURING / UPON COMPLETION OF DRILLING  
WATER LEVEL MEASURED IN MONITORING WELL

UNDISTURBED FIELD VANE  
REMOLDED FIELD VANE  
LAB SHEAR TEST  
POCKET PENETROMETER  
POCKET TORVANE  
CHECKED BY KH

## LOG OF BOREHOLE NO. 6

**PROJECT** Proposed Arkell Road Subdivision

**LOCATION** Arkell Road, Guelph, Ontario

**BORING METHOD** Continuous Flight Hollow Stem Augers

**BORING DATE:** 2017 03 21

**PML REF.:** 17KF002

**ENGINEER** K. Hanes

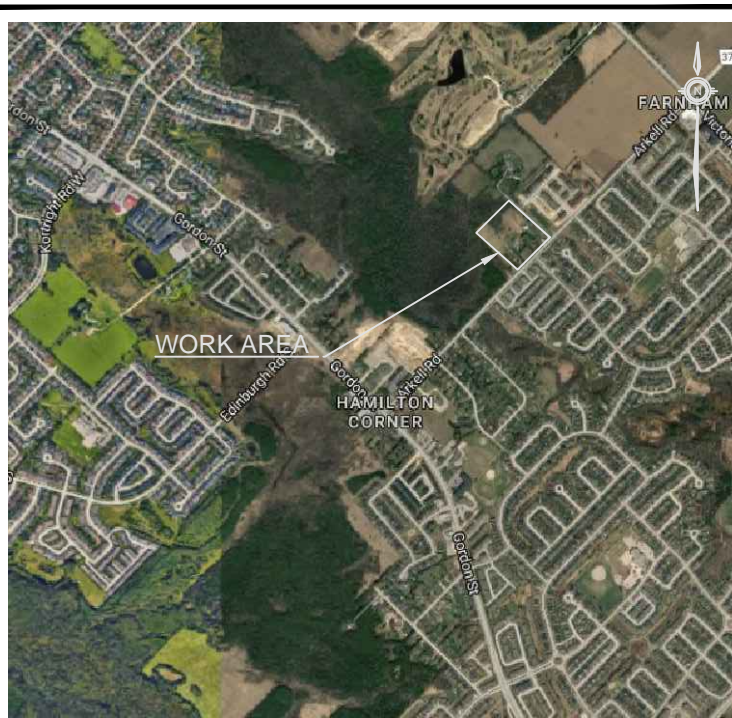
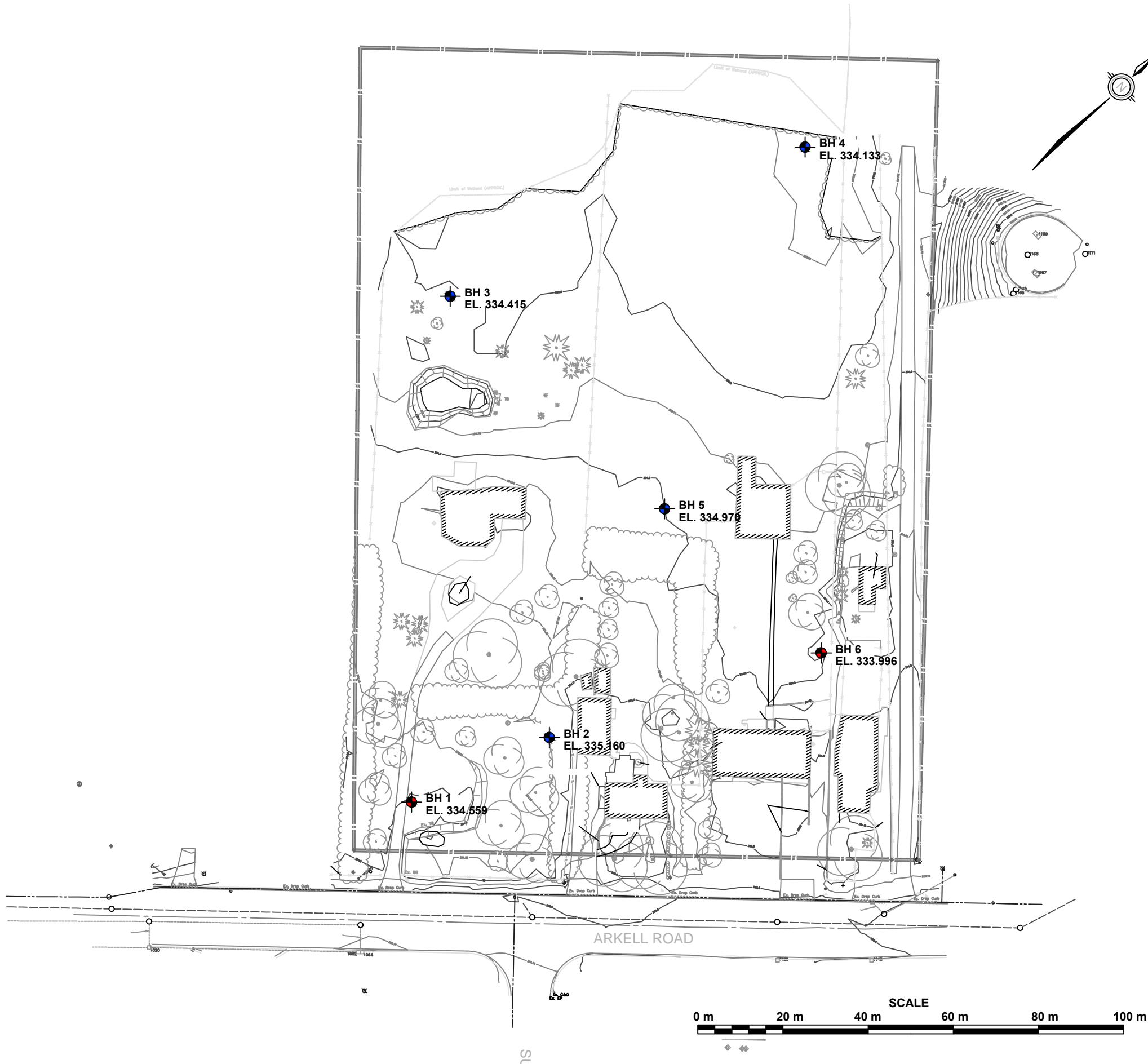
**TECHNICIAN** H. Shinwary

SOIL PROFILE			SAMPLES			SHEAR STRENGTH $C_u$ (kPa)		LIQUID LIMIT $W_L$		UNIT WEIGHT $\gamma$	GROUND WATER OBSERVATIONS AND REMARKS
DEPTH in METRES	DESCRIPTION	LEGEND	NUMBER	TYPE	BLOWS/0.3m N - VALUES	ELEVATION SCALE	50 100 150 200	PLASTIC LIMIT $W_p$	WATER CONTENT $W$		
	GROUND ELEVATION 334.0										
0.0	0.20 TOPSOIL: Dark brown silt, trace sand, numerous rootlets, damp		1	SS	18						
0.41	0.69 FILL: Dark brown silt, some sand, trace gravel, occasional rootlets, damp		2	SS	45						
1.0	333.31 SAND AND GRAVEL: Dense brown sand and gravel, trace to some silt, numerous cobbles, damp		3	SS	36						
1.5	332.6 becoming moist										
2.0	331.8 becoming saturated		4	SS	12						
2.2											
3.0			5	SS	10						
4.0											
5.0			6	SS	16						
5.8											
6.0	328.2 SILT TILL: Very dense brown silt, some sand, some gravel, occasional cobbles, damp		7	SS	50/75mm						
6.6	327.4 BOREHOLE TERMINATED AT 6.6 m										
7.0											Upon completion of augering Cave to 2.0 m Free water at 1.83 m
8.0											
9.0											
10.0											
11.0											
12.0											
13.0											
14.0											
15.0											
16.0											
17.0											

**NOTES:** Headspace: SS1 0ppm, SS2 0ppm, SS3 0ppm, SS4 5ppm, SS5 0ppm, SS6 0ppm, SS7 0ppm



WATER LEVEL OBSERVED DURING / UPON COMPLETION OF DRILLING  
WATER LEVEL MEASURED IN MONITORING WELL

+ UNDISTURBED FIELD VANE  
⊕ REMOLDED FIELD VANE  
⊗ LAB SHEAR TEST  
▲ POCKET PENETROMETER  
◆ POCKET TORVANE  
CHECKED BY KH



KEY PLAN

LEGEND:

-  BOREHOLE
-  BOREHOLE WITH MONITORING WELL

REFERENCE:

BOREHOLE LOCATION PLAN REPRODUCED FROM DRAWING SUPPLIED BY CLIENT.

NOTE:

THE INFERRED STRATIGRAPHY REFERRED TO IN THE REPORT IS BASED ON THE DATA FROM THESE BOREHOLES SUPPLEMENTED BY GEOLOGICAL EVIDENCE. THE ACTUAL STRATIGRAPHY BETWEEN THE BOREHOLES MAY VARY.

CRESCENT HOMES

PROPOSED ARKELL ROAD SUBDIVISION  
ARKELL ROAD  
GUELPH, ONTARIO

BOREHOLE LOCATION PLAN



DRAWN	D. BRICE	DATE	SCALE	PML REF.	DWG. NO.
CHECKED	H. SHINWARY	SEPTEMBER 2018	AS SHOWN	17KF002	1
APPROVED	W. LOGHRIN				



## **APPENDIX A**

### ENGINEERED FILL

The information presented in this appendix is intended for general guidance only. Site specific conditions and prevailing weather may require modification of compaction standards, backfill type or procedures. Each site must be discussed, and procedures agreed with Peto MacCallum Ltd. prior to the start of the earthworks and must be subject to ongoing review during construction. This appendix is not intended to apply to embankments. Steeply sloping ravine residential lots require special consideration.

For fill to be classified as engineered fill suitable for supporting structural loads, a number of conditions must be satisfied, including but not necessarily limited to the following:

## 1. Purpose

The site specific purpose of the engineered fill must be recognized. In advance of construction, all parties should discuss the project and its requirements and agree on an appropriate set of standards and procedures.

## 2. Minimum Extent

The engineered fill envelope must extend beyond the footprint of the structure to be supported. The minimum extent of the envelope should be defined from a geotechnical perspective by:

- at founding level, extend a minimum 1.0 m beyond the outer edge of the foundations, greater if adequate layout has not yet been completed as noted below; and
- extend downward and outward at a slope no greater than 45° to meet the subgrade

All fill within the envelope established above must meet the requirements of engineered fill in order to support the structure safely. Other considerations such as survey control, or construction methods may require an envelope that is larger, as noted in the following sections.

Once the minimum envelope has been established, structures must not be moved or extended without consultation with Peto MacCallum Ltd. Similarly, Peto MacCallum Ltd. should be consulted prior to any excavation within the minimum envelope.

## 3. Survey Control

Accurate survey control is essential to the success of an engineered fill project. The boundaries of the engineered fill must be laid out by a surveyor in consultation with engineering staff from Peto MacCallum Ltd. Careful consideration of the maximum building envelope is required.

During construction it is necessary to have a qualified surveyor provide total station control on the three dimensional extent of filling.

## 4. Subsurface Preparation

Prior to placement of fill, the subgrade must be prepared to the satisfaction of Peto MacCallum Ltd. All deleterious material must be removed and in some cases, excavation of native mineral soils may be required.

Particular attention must be paid to wet subgrades and possible additional measures required to achieve sufficient compaction. Where fill is placed against a slope, benching may be necessary and natural drainage paths must not be blocked.

## 5. Suitable Fill Materials

All material to be used as fill must be approved by Peto MacCallum Ltd. Such approval will be influenced by many factors and must be site and project specific. External fill sources must be sampled, tested and approved prior to material being hauled to site.

## 6. Test Section

In advance of the start of construction of the engineered fill pad, the Contractor should conduct a test section. The compaction criterion will be assessed in consultation with Peto MacCallum Ltd. for the various fill material types using different lift thicknesses and number of passes for the compaction equipment proposed by the Contractor.

Additional test sections may be required throughout the course of the project to reflect changes in fill sources, natural moisture content of the material and weather conditions.

The Contractor should be particularly aware of changes in the moisture content of fill material. Site review by Peto MacCallum Ltd. is required to ensure the desired lift thickness is maintained and that each lift is systematically compacted, tested and approved before a subsequent lift is commenced.

## 7. Inspection and Testing

Uniform, thorough compaction is crucial to the performance of the engineered fill and the supported structure. Hence, all subgrade preparation, filling and compacting must be carried out under the full time inspection by Peto MacCallum Ltd.

All founding surfaces for all buildings and residential dwellings or any part thereof (including but not limited to footings and floor slabs) on structural fill or native soils must be inspected and approved by PML engineering personnel prior to placement of the base/subbase granular material and/or concrete. The purpose of the inspection is to ensure the subgrade soils are capable of supporting the building/house foundation and floor slab loads and to confirm the building/house envelope does not extend beyond the limits of any structural fill pads.



## 8. Protection of Fill

Fill is generally more susceptible to the effects of weather than natural soil. Fill placed and approved to the level at which structural support is required must be protected from excessive wetting, drying, erosion or freezing. Where adequate protection has not been provided, it may be necessary to provide deeper footings or to strip and recompact some of the fill.

## 9. Construction Delay Time Considerations

The integrity of the fill pad can deteriorate due to the harsh effects of our Canadian weather. Hence, particular care must be taken if the fill pad is constructed over a long time period.

It is necessary therefore, that all fill sources are tested to ensure the material compactability prior to the soil arriving at site. When there has been a lengthy delay between construction periods of the fill pad, it is necessary to conduct subgrade proof rolling, test pits or boreholes to verify the adequacy of the exposed subgrade to accept new fill material.

When the fill pad will be constructed over a lengthy period of time, a field survey should be completed at the end of each construction season to verify the areal extent and the level at which the compacted fill has been brought up to, tested and approved.

In the following spring, subexcavation may be necessary if the fill pad has been softened attributable to ponded surface water or freeze/thaw cycles.

A new survey is required at the beginning of the next construction season to verify that random dumping and/or spreading of fill has not been carried out at the site.

## 10. Approved Fill Pad Surveillance

It should be appreciated that once the fill pad has been brought to final grade and documented by field survey, there must be ongoing surveillance to ensure that the integrity of the fill pad is not threatened.

Grading operations adjacent to fill pads can often take place several months or years after completion of the fill pad.

It is imperative that all site management and supervision staff, the staff of Contractors and earthwork operators be fully aware of the boundaries of all approved engineered fill pads.

Excavation into an approved engineered fill pad should never be contemplated without the full knowledge, approval and documentation by the geotechnical consultant.

If the fill pad is knowingly built several years in advance of ultimate construction, the areal limits of the fill pad should be substantially overbuilt laterally to allow for changes in possible structure location and elevation and other earthwork operations and competing interests on the site. The overbuilt distance required is project and/or site specified.

Iron bars should be placed at the corner/intermediate points of the fill pad as a permanent record of the approved limits of the work for record keeping purposes.

## 11. Unusual Working Conditions

Construction of fill pads may at times take place at night and/or during periods of freezing weather conditions because of the requirements of the project schedule. It should be appreciated therefore, that both situations present more difficult working conditions. The Owner, Contractor, Design Consultant and Geotechnical Engineer must be willing to work together to revise site construction procedures, enhance field testing and surveillance, and incorporate design modifications as necessary to suit site conditions.

When working at night there must be sufficient artificial light to properly illuminate the fill pad and borrow areas.

Placement of material to form an engineered fill pad during winter and freezing temperatures has its own special conditions that must be addressed. It is imperative that each day prior to placement of new fill, the exposed subgrade must be inspected and any overnight snow or frozen material removed. Particular attention should be given to the borrow source inspection to ensure only nonfrozen fill is brought to the site.

The Contractor must continually assess the work program and have the necessary spreading and compacting equipment to ensure that densification of the fill material takes place in a minimum amount of time. Changes may be required to the spreading methods, lift thickness, and compaction techniques to ensure the desired compaction is achieved uniformly throughout each fill lift.

The Contractor should adequately protect the subgrade at the end of each shift to minimize frost penetration overnight. Since water cannot be added to the fill material to facilitate compaction, it is imperative that densification of the fill be achieved by additional compaction effort and an appropriate reduced lift thickness. Once the fill pad has been completed, it must be properly protected from freezing temperatures and ponding of water during the spring thaw period.

If the pad is unusually thick or if the fill thickness varies dramatically across the width or length of the fill pad, Peto MacCallum Ltd. should be consulted for additional recommendations. In this case, alternative special provisions may be recommended, such as providing a surcharge preload for a limited time or increase the degree of compaction of the fill.



## **APPENDIX B**

### STATEMENT OF LIMITATIONS

# STATEMENT OF LIMITATIONS

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This report is prepared for and made available for the sole use of the client named. Peto MacCallum Ltd. (PML) hereby disclaims any liability or responsibility to any person or entity, other than those for whom this report is specifically issued, for any loss, damage, expenses, or penalties that may arise or result from the use of any information or recommendations contained in this report. The contents of this report may not be used or relied upon by any other person without the express written consent and authorization of PML.

This report shall not be relied upon for any purpose other than as agreed with the client named without the written consent of PML. It shall not be used to express or imply warranty as to the fitness of the property for a particular purpose. A portion of this report may not be used as a separate entity: that is to say the report is to be read in its entirety at all times.

The report is based solely on the scope of services which are specifically referred to in this report. No physical or intrusive testing has been performed, except as specifically referenced in this report. This report is not a certification of compliance with past or present regulations, codes, guidelines and policies.

The scope of services carried out by PML is based on details of the proposed development and land use to address certain issues, purposes and objectives with respect to the specific site as identified by the client. Services not expressly set forth in writing are expressly excluded from the services provided by PML. In other words, PML has not performed any observations, investigations, study analysis, engineering evaluation or testing that is not specifically listed in the scope of services in this report. PML assumes no responsibility or duty to the client for any such services and shall not be liable for failing to discover any condition, whose discovery would require the performance of services not specifically referred to in this report.

The findings and comments made by PML in this report are based on the conditions observed at the time of PML's site reconnaissance. No assurances can be made and no assurances are given with respect to any potential changes in site conditions following the time of completion of PML's field work. Furthermore, regulations, codes and guidelines may change at any time subsequent to the date of this report and these changes may effect the validity of the findings and recommendations given in this report.

# STATEMENT OF LIMITATIONS

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The results and conclusions with respect to site conditions are therefore in no way intended to be taken as a guarantee or representation, expressed or implied, that the site is free from any contaminants from past or current land use activities or that the conditions in all areas of the site and beneath or within structures are the same as those areas specifically sampled.

Any investigation, examination, measurements or sampling explorations at a particular location may not be representative of conditions between sampled locations. Soil, ground water, surface water, or building material conditions between and beyond the sampled locations may differ from those encountered at the sampling locations and conditions may become apparent during construction which could not be detected or anticipated at the time of the intrusive sampling investigation.

Budget estimates contained in this report are to be viewed as an engineering estimate of probable costs and provided solely for the purposes of assisting the client in its budgeting process. It is understood and agreed that PML will not in any way be held liable as a result of any budget figures provided by it.

The Client expressly waives its right to withhold PML's fees, either in whole or in part, or to make any claim or commence any action or bring any other proceedings, whether in contract, tort, or otherwise against PML in anyway connected with advice or information given by PML relating to the cost estimate or Environmental Remediation/Cleanup and Restoration or Soil and Ground Water Management Plan Cost Estimate.



## **APPENDIX C**

### AGAT CERTIFICATES OF ANALYSIS



**O.Reg. 153/04, As Amended, Table 1 Standards (Soil)**

(Residential / Parkland / Institutional / Industrial / Commercial / Community  
Property Use)

CLIENT NAME: PETO MACCALLUM LIMITED  
16 FRANKLIN STREET SOUTH  
KITCHENER, ON N2C1R4  
(519) 893-7500

ATTENTION TO: Ken Hanes

PROJECT: 17KF002

AGAT WORK ORDER: 17T199091

SOIL ANALYSIS REVIEWED BY: Amanjot Bhela, Inorganic Coordinator

TRACE ORGANICS REVIEWED BY: Neli Popnikolova, Senior Chemist

DATE REPORTED: Apr 18, 2017

PAGES (INCLUDING COVER): 7

VERSION\*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

\*NOTES

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.





## Certificate of Analysis

AGAT WORK ORDER: 17T199091

PROJECT: 17KF002

5835 COOPERS AVENUE  
MISSISSAUGA, ONTARIO  
CANADA L4Z 1Y2  
TEL (905)712-5100  
FAX (905)712-5122  
<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY:

### O. Reg. 153(511) - Metals & Inorganics (Soil)

DATE RECEIVED: 2017-03-23

DATE REPORTED: 2017-04-18

Parameter	Unit	SAMPLE DESCRIPTION:		BH4-SS1	BH5-SS1	BH6-SS1
		SAMPLE TYPE:		Soil	Soil	Soil
		DATE SAMPLED:		2017-03-21	2017-03-21	2017-03-21
		G / S	RDL	8276142	8276150	8276151
Antimony	µg/g	1.3	0.8	<0.8	<0.8	<0.8
Arsenic	µg/g	18	1	3	5	6
Barium	µg/g	220	2	15	45	48
Beryllium	µg/g	2.5	0.5	<0.5	<0.5	<0.5
Boron	µg/g	36	5	6	6	<5
Boron (Hot Water Soluble)	µg/g	NA	0.10	0.17	0.23	0.27
Cadmium	µg/g	1.2	0.5	<0.5	0.7	0.6
Chromium	µg/g	70	2	8	13	13
Cobalt	µg/g	21	0.5	1.9	4.4	4.9
Copper	µg/g	92	1	8	11	11
Lead	µg/g	120	1	40	62	53
Molybdenum	µg/g	2	0.5	0.6	0.9	0.5
Nickel	µg/g	82	1	5	10	10
Selenium	µg/g	1.5	0.4	<0.4	0.4	0.5
Silver	µg/g	0.5	0.2	<0.2	<0.2	<0.2
Thallium	µg/g	1	0.4	<0.4	<0.4	<0.4
Uranium	µg/g	2.5	0.5	0.5	0.5	0.5
Vanadium	µg/g	86	1	11	22	24
Zinc	µg/g	290	5	182	313	254
Chromium VI	µg/g	0.66	0.2	<0.2	<0.2	<0.2
Cyanide	µg/g	0.051	0.040	<0.040	<0.040	<0.040
Mercury	µg/g	0.27	0.10	<0.10	<0.10	<0.10
Electrical Conductivity	mS/cm	0.57	0.005	0.177	0.233	0.173
Sodium Adsorption Ratio	NA	2.4	NA	0.125	0.142	0.053
pH, 2:1 CaCl <sub>2</sub> Extraction	pH Units		NA	6.74	6.90	7.07

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Table 1: Full Depth Background Site Condition Standards - Soil - Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use

8276142-8276151 EC & SAR were determined on the DI water extract obtained from the 2:1 leaching procedure (2 parts DI water:1 part soil). pH was determined on the 0.01M CaCl<sub>2</sub> extract prepared at 2:1 ratio.

Certified By:

*Amanjot Bhela*



## Certificate of Analysis

AGAT WORK ORDER: 17T199091

PROJECT: 17KF002

5835 COOPERS AVENUE  
MISSISSAUGA, ONTARIO  
CANADA L4Z 1Y2  
TEL (905)712-5100  
FAX (905)712-5122  
<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY:

### O. Reg. 153(511) - OC Pesticides (Soil)

DATE RECEIVED: 2017-03-23

DATE REPORTED: 2017-04-18

		SAMPLE DESCRIPTION:		BH4-SS1	BH6-SS1
		SAMPLE TYPE:		Soil	Soil
		DATE SAMPLED:		2017-03-21	2017-03-21
Parameter	Unit	G / S	RDL	8276142	8276151
Hexachloroethane	µg/g	0.01	0.01	<0.01	<0.01
Gamma-Hexachlorocyclohexane	µg/g	0.01	0.005	<0.005	<0.005
Heptachlor	µg/g	0.05	0.005	<0.005	<0.005
Aldrin	µg/g	0.05	0.005	<0.005	<0.005
Heptachlor Epoxide	µg/g	0.05	0.005	<0.005	<0.005
Endosulfan	µg/g	0.04	0.005	<0.005	<0.005
Chlordane	µg/g	0.05	0.007	<0.007	<0.007
DDE	µg/g	0.05	0.007	<0.007	<0.007
DDD	µg/g	0.05	0.007	<0.007	<0.007
DDT	µg/g	1.4	0.007	<0.007	<0.007
Dieldrin	µg/g	0.05	0.005	<0.005	<0.005
Endrin	µg/g	0.04	0.005	<0.005	<0.005
Methoxychlor	µg/g	0.05	0.005	<0.005	<0.005
Hexachlorobenzene	µg/g	0.01	0.005	<0.005	<0.005
Hexachlorobutadiene	µg/g	0.01	0.01	<0.01	<0.01
Moisture Content	%		0.1	33.0	6.7
Surrogate	Unit	Acceptable Limits			
TCMX	%	50-140		70	66
Decachlorobiphenyl	%	60-130		72	88

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Table 1: Full Depth Background Site Condition Standards - Soil - Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use

8276142-8276151 Results are based on the dry weight of the soil.

Note: DDT applies to the total of op'DDT and pp'DDT, DDD applies to the total of op'DDD and pp'DDD and DDE applies to the total of op'DDE and pp'DDE. Endosulfan applies to the total of Endosulfan I and Endosulfan II.

Chlordane applies to the total of Alpha-Chlordane and Gamma-Chlordane.

Certified By:

*N Popmukolof*



**AGAT** Laboratories

## Guideline Violation

AGAT WORK ORDER: 17T199091

PROJECT: 17KF002

5835 COOPERS AVENUE  
MISSISSAUGA, ONTARIO  
CANADA L4Z 1Y2  
TEL (905)712-5100  
FAX (905)712-5122  
<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLEID	SAMPLE TITLE	GUIDELINE	ANALYSIS PACKAGE	PARAMETER	UNIT	GUIDEVALUE	RESULT
8276150	BH5-SS1	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Zinc	µg/g	290	313



## Quality Assurance

CLIENT NAME: PETO MACCALLUM LIMITED

PROJECT: 17KF002

SAMPLING SITE:

AGAT WORK ORDER: 17T199091

ATTENTION TO: Ken Hanes

SAMPLED BY:

Soil Analysis															
RPT Date: Apr 18, 2017			DUPLICATE			Method Blank	REFERENCE MATERIAL			METHOD BLANK SPIKE			MATRIX SPIKE		
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 153(511) - Metals & Inorganics (Soil)															
Antimony	8272855		3.6	3.6	NA	< 0.8	126%	70%	130%	105%	80%	120%	96%	70%	130%
Arsenic	8272855		9	7	25.0%	< 1	108%	70%	130%	105%	80%	120%	103%	70%	130%
Barium	8272855		76	75	1.3%	< 2	101%	70%	130%	98%	80%	120%	101%	70%	130%
Beryllium	8272855		<0.5	<0.5	NA	< 0.5	83%	70%	130%	105%	80%	120%	89%	70%	130%
Boron	8272855		6	6	NA	< 5	82%	70%	130%	107%	80%	120%	93%	70%	130%
Boron (Hot Water Soluble)	8272855		0.41	0.42	NA	< 0.10	112%	60%	140%	103%	70%	130%	99%	60%	140%
Cadmium	8272855		0.8	0.8	NA	< 0.5	110%	70%	130%	106%	80%	120%	105%	70%	130%
Chromium	8272855		18	18	0.0%	< 2	96%	70%	130%	114%	80%	120%	112%	70%	130%
Cobalt	8272855		5.5	5.5	0.0%	< 0.5	102%	70%	130%	110%	80%	120%	99%	70%	130%
Copper	8272855		63	62	1.6%	< 1	101%	70%	130%	117%	80%	120%	85%	70%	130%
Lead	8272855		190	197	3.6%	< 1	105%	70%	130%	101%	80%	120%	70%	70%	130%
Molybdenum	8272855		1.3	1.3	NA	< 0.5	107%	70%	130%	103%	80%	120%	105%	70%	130%
Nickel	8272855		24	25	4.1%	< 1	103%	70%	130%	112%	80%	120%	100%	70%	130%
Selenium	8272855		0.9	1.0	NA	< 0.4	128%	70%	130%	99%	80%	120%	106%	70%	130%
Silver	8272855		<0.2	<0.2	NA	< 0.2	98%	70%	130%	115%	80%	120%	110%	70%	130%
Thallium	8272855		<0.4	<0.4	NA	< 0.4	103%	70%	130%	104%	80%	120%	98%	70%	130%
Uranium	8272855		<0.5	<0.5	NA	< 0.5	98%	70%	130%	93%	80%	120%	95%	70%	130%
Vanadium	8272855		20	20	0.0%	< 1	99%	70%	130%	109%	80%	120%	109%	70%	130%
Zinc	8272855		205	199	3.0%	< 5	102%	70%	130%	117%	80%	120%	84%	70%	130%
Chromium VI	8277762		<0.2	<0.2	NA	< 0.2	93%	70%	130%	98%	80%	120%	100%	70%	130%
Cyanide	8278916		<0.040	<0.040	NA	< 0.040	102%	70%	130%	108%	80%	120%	94%	70%	130%
Mercury	8272855		0.15	0.17	NA	< 0.10	100%	70%	130%	88%	80%	120%	93%	70%	130%
Electrical Conductivity	8277893		0.376	0.369	1.9%	< 0.005	93%	90%	110%	NA			NA		
Sodium Adsorption Ratio	8276363		0.057	0.053	7.3%	NA	NA			NA			NA		
pH, 2:1 CaCl2 Extraction	8277854		7.37	7.42	0.7%	NA	101%	80%	120%	NA			NA		

Comments: NA signifies Not Applicable.

Duplicate Qualifier: As the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Certified By:

*Amanjot Bhela*



## Quality Assurance

CLIENT NAME: PETO MACCALLUM LIMITED

PROJECT: 17KF002

SAMPLING SITE:

AGAT WORK ORDER: 17T199091

ATTENTION TO: Ken Hanes

SAMPLED BY:

### Trace Organics Analysis

RPT Date: Apr 18, 2017

RPT Date: Apr 18, 2017			DUPLICATE			Method Blank	REFERENCE MATERIAL		METHOD BLANK SPIKE			MATRIX SPIKE			
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper

#### O. Reg. 153(511) - OC Pesticides (Soil)

Hexachloroethane	8267227		< 0.01	< 0.01	NA	< 0.01	82%	50%	140%	96%	50%	140%	64%	50%	140%
Gamma-Hexachlorocyclohexane	8267227		< 0.005	< 0.005	NA	< 0.005	92%	50%	140%	78%	50%	140%	66%	50%	140%
Heptachlor	8267227		< 0.005	< 0.005	NA	< 0.005	80%	50%	140%	90%	50%	140%	80%	50%	140%
Aldrin	8267227		< 0.005	< 0.005	NA	< 0.005	109%	50%	140%	94%	50%	140%	68%	50%	140%
Heptachlor Epoxide	8267227		< 0.005	< 0.005	NA	< 0.005	90%	50%	140%	96%	50%	140%	82%	50%	140%
Endosulfan	8267227		< 0.005	< 0.005	NA	< 0.005	89%	50%	140%	88%	50%	140%	69%	50%	140%
Chlordane	8267227		< 0.007	< 0.007	NA	< 0.007	87%	50%	140%	91%	50%	140%	78%	50%	140%
DDE	8267227		< 0.007	< 0.007	NA	< 0.007	88%	50%	140%	98%	50%	140%	78%	50%	140%
DDD	8267227		< 0.007	< 0.007	NA	< 0.007	94%	50%	140%	94%	50%	140%	84%	50%	140%
DDT	8267227		< 0.007	< 0.007	NA	< 0.007	88%	50%	140%	87%	50%	140%	78%	50%	140%
Dieldrin	8267227		< 0.005	< 0.005	NA	< 0.005	84%	50%	140%	90%	50%	140%	80%	50%	140%
Endrin	8267227		< 0.005	< 0.005	NA	< 0.005	84%	50%	140%	76%	50%	140%	82%	50%	140%
Methoxychlor	8267227		< 0.005	< 0.005	NA	< 0.005	76%	50%	140%	82%	50%	140%	96%	50%	140%
Hexachlorobenzene	8267227		< 0.005	< 0.005	NA	< 0.005	92%	50%	140%	100%	50%	140%	92%	50%	140%
Hexachlorobutadiene	8267227		< 0.01	< 0.01	NA	< 0.01	93%	50%	140%	100%	50%	140%	68%	50%	140%

Comments: When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

*N Popmukohof*

## Method Summary

CLIENT NAME: PETO MACCALLUM LIMITED

AGAT WORK ORDER: 17T199091

PROJECT: 17KF002

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY:

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Soil Analysis			
Antimony	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Arsenic	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Barium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Beryllium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron (Hot Water Soluble)	MET-93-6104	EPA SW 846 6010C; MSA, Part 3, Ch.21	ICP/OES
Cadmium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Cobalt	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Copper	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Lead	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Molybdenum	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Nickel	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Selenium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Silver	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Thallium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Uranium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Vanadium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Zinc	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium VI	INOR-93-6029	SM 3500 B; MSA Part 3, Ch. 25	SPECTROPHOTOMETER
Cyanide	INOR-93-6052	MOE CN-3015 & E 3009 A; SM 4500 CN	TECHNICON AUTO ANALYZER
Mercury	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Electrical Conductivity	INOR-93-6036	McKeague 4.12, SM 2510 B	EC METER
Sodium Adsorption Ratio	INOR-93-6007	McKeague 4.12 & 3.26 & EPA SW-846 6010B	ICP/OES
pH, 2:1 CaCl <sub>2</sub> Extraction	INOR-93-6031	MSA part 3 & SM 4500-H+ B	PH METER
Trace Organics Analysis			
Hexachloroethane	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Gamma-Hexachlorocyclohexane	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Heptachlor	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Aldrin	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Heptachlor Epoxide	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Endosulfan	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Chlordane	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
DDE	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
DDD	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
DDT	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Dieldrin	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Endrin	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Methoxychlor	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Hexachlorobenzene	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Hexachlorobutadiene	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
TCMX	ORG-91-5112	EPA SW-846 3541,3620 & 8081	GC/ECD
Decachlorobiphenyl	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Moisture Content		MOE E3139	BALANCE

CLIENT NAME: PETO MACCALLUM LIMITED  
16 FRANKLIN STREET SOUTH  
KITCHENER, ON N2C1R4  
(519) 893-7500

ATTENTION TO: Ken Hanes

PROJECT: 17KF002

AGAT WORK ORDER: 17W201248

SOIL ANALYSIS REVIEWED BY: Sofka Pehlyova, Senior Analyst

DATE REPORTED: Apr 10, 2017

PAGES (INCLUDING COVER): 5

VERSION\*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

\*NOTES

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.



## Certificate of Analysis

AGAT WORK ORDER: 17W201248

PROJECT: 17KF002

5835 COOPERS AVENUE  
MISSISSAUGA, ONTARIO  
CANADA L4Z 1Y2  
TEL (905)712-5100  
FAX (905)712-5122  
<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY: H. Shinwary

### O. Reg. 153(511) - Metals & Inorganics (Soil)

DATE RECEIVED: 2017-03-30

DATE REPORTED: 2017-04-10

Parameter	Unit	SAMPLE DESCRIPTION:		BH5-SS4	BH6-SS3
		SAMPLE TYPE:		Soil	Soil
		DATE SAMPLED:		2017-03-21	2017-03-21
		G / S	RDL	8288805	8288806
Antimony	µg/g	1.3	0.8	<0.8	<0.8
Arsenic	µg/g	18	1	3	4
Barium	µg/g	220	2	9	13
Beryllium	µg/g	2.5	0.5	<0.5	<0.5
Boron	µg/g	36	5	<5	<5
Boron (Hot Water Soluble)	µg/g	NA	0.10	<0.10	<0.10
Cadmium	µg/g	1.2	0.5	<0.5	0.6
Chromium	µg/g	70	2	5	8
Cobalt	µg/g	21	0.5	1.8	4.0
Copper	µg/g	92	1	8	15
Lead	µg/g	120	1	18	43
Molybdenum	µg/g	2	0.5	<0.5	0.8
Nickel	µg/g	82	1	4	8
Selenium	µg/g	1.5	0.4	<0.4	<0.4
Silver	µg/g	0.5	0.2	<0.2	<0.2
Thallium	µg/g	1	0.4	<0.4	<0.4
Uranium	µg/g	2.5	0.5	<0.5	<0.5
Vanadium	µg/g	86	1	11	19
Zinc	µg/g	290	5	180	370
Chromium VI	µg/g	0.66	0.2	<0.2	<0.2
Cyanide	µg/g	0.051	0.040	<0.040	<0.040
Mercury	µg/g	0.27	0.10	<0.10	<0.10
Electrical Conductivity	mS/cm	0.57	0.005	0.098	0.174
Sodium Adsorption Ratio	NA	2.4	NA	0.303	0.509
pH, 2:1 CaCl <sub>2</sub> Extraction	pH Units		NA	7.94	8.16

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Table 1: Full Depth Background Site Condition Standards - Soil - Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use

8288805-8288806 EC & SAR were determined on the DI water extract obtained from the 2:1 leaching procedure (2 parts DI water:1 part soil). pH was determined on the 0.01M CaCl<sub>2</sub> extract prepared at 2:1 ratio.

Certified By:

*Sofra Pehlyova*





**AGAT** Laboratories

## Guideline Violation

AGAT WORK ORDER: 17W201248

PROJECT: 17KF002

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CANADA L4Z 1Y2  
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FAX (905)712-5122  
<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLEID	SAMPLE TITLE	GUIDELINE	ANALYSIS PACKAGE	PARAMETER	UNIT	GUIDEVALUE	RESULT
8288806	BH6-SS3	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Zinc	µg/g	290	370



## Quality Assurance

CLIENT NAME: PETO MACCALLUM LIMITED

PROJECT: 17KF002

SAMPLING SITE:

AGAT WORK ORDER: 17W201248

ATTENTION TO: Ken Hanes

SAMPLED BY: H. Shinwary

### Soil Analysis

RPT Date: Apr 10, 2017			DUPLICATE			Method Blank	REFERENCE MATERIAL			METHOD BLANK SPIKE			MATRIX SPIKE		
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 153(511) - Metals & Inorganics (Soil)															
Antimony	8287941		<0.8	<0.8	NA	< 0.8	116%	70%	130%	100%	80%	120%	92%	70%	130%
Arsenic	8287941		4	4	NA	< 1	107%	70%	130%	98%	80%	120%	104%	70%	130%
Barium	8287941		48	47	2.6%	< 2	98%	70%	130%	96%	80%	120%	101%	70%	130%
Beryllium	8287941		<0.5	<0.5	NA	< 0.5	78%	70%	130%	108%	80%	120%	89%	70%	130%
Boron	8287941		<5	<5	NA	< 5	89%	70%	130%	108%	80%	120%	91%	70%	130%
Boron (Hot Water Soluble)	8287941		0.34	0.36	NA	< 0.10	112%	60%	140%	100%	70%	130%	101%	60%	140%
Cadmium	8287941		<0.5	<0.5	NA	< 0.5	89%	70%	130%	100%	80%	120%	103%	70%	130%
Chromium	8287941		13	13	0.0%	< 2	95%	70%	130%	106%	80%	120%	120%	70%	130%
Cobalt	8287941		6.0	6.2	3.3%	< 0.5	102%	70%	130%	108%	80%	120%	108%	70%	130%
Copper	8287941		32	33	3.1%	< 1	94%	70%	130%	110%	80%	120%	115%	70%	130%
Lead	8287941		10	10	0.0%	< 1	101%	70%	130%	101%	80%	120%	99%	70%	130%
Molybdenum	8287941		<0.5	<0.5	NA	< 0.5	101%	70%	130%	103%	80%	120%	103%	70%	130%
Nickel	8287941		13	13	0.0%	< 1	105%	70%	130%	107%	80%	120%	108%	70%	130%
Selenium	8287941		<0.4	<0.4	NA	< 0.4	107%	70%	130%	103%	80%	120%	102%	70%	130%
Silver	8287941		<0.2	<0.2	NA	< 0.2	93%	70%	130%	106%	80%	120%	105%	70%	130%
Thallium	8287941		<0.4	<0.4	NA	< 0.4	86%	70%	130%	102%	80%	120%	103%	70%	130%
Uranium	8287941		<0.5	<0.5	NA	< 0.5	90%	70%	130%	92%	80%	120%	95%	70%	130%
Vanadium	8287941		22	22	0.0%	< 1	100%	70%	130%	106%	80%	120%	124%	70%	130%
Zinc	8287941		53	49	7.8%	< 5	103%	70%	130%	118%	80%	120%	116%	70%	130%
Chromium VI	8284952		<0.2	<0.2	NA	< 0.2	92%	70%	130%	96%	80%	120%	98%	70%	130%
Cyanide	8288805	8288805	<0.040	<0.040	NA	< 0.040	102%	70%	130%	103%	80%	120%	104%	70%	130%
Mercury	8287941		<0.10	<0.10	NA	< 0.10	102%	70%	130%	95%	80%	120%	102%	70%	130%
Electrical Conductivity	8291645		0.428	0.431	0.7%	< 0.005	94%	90%	110%	NA			NA		
Sodium Adsorption Ratio	8287941		0.751	0.761	1.3%	NA	NA			NA			NA		
pH, 2:1 CaCl2 Extraction	8285504		7.26	7.23	0.4%	NA	100%	80%	120%	NA			NA		

Comments: NA signifies Not Applicable.

Duplicate Qualifier: As the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Certified By:

*Sofia Pehlyora*

## Method Summary

CLIENT NAME: PETO MACCALLUM LIMITED

PROJECT: 17KF002

SAMPLING SITE:

AGAT WORK ORDER: 17W201248

ATTENTION TO: Ken Hanes

SAMPLED BY: H. Shinwary

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Soil Analysis			
Antimony	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Arsenic	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Barium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Beryllium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron (Hot Water Soluble)	MET-93-6104	EPA SW 846 6010C; MSA, Part 3, Ch.21	ICP/OES
Cadmium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Cobalt	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Copper	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Lead	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Molybdenum	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Nickel	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Selenium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Silver	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Thallium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Uranium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Vanadium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Zinc	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium VI	INOR-93-6029	SM 3500 B; MSA Part 3, Ch. 25	SPECTROPHOTOMETER
Cyanide	INOR-93-6052	MOE CN-3015 & E 3009 A; SM 4500 CN	TECHNICON AUTO ANALYZER
Mercury	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Electrical Conductivity	INOR-93-6036	McKeague 4.12, SM 2510 B	EC METER
Sodium Adsorption Ratio	INOR-93-6007	McKeague 4.12 & 3.26 & EPA SW-846 6010B	ICP/OES
pH, 2:1 CaCl <sub>2</sub> Extraction	INOR-93-6031	MSA part 3 & SM 4500-H+ B	PH METER



**O.Reg. 153/04, As Amended, Table 2 Standards (Soil)**

(Industrial / Commercial / Community Property Use)

CLIENT NAME: PETO MACCALLUM LIMITED  
16 FRANKLIN STREET SOUTH  
KITCHENER, ON N2C1R4  
(519) 893-7500

ATTENTION TO: Ken Hanes

PROJECT: 17KF002

AGAT WORK ORDER: 17T199091

SOIL ANALYSIS REVIEWED BY: Amanjot Bhela, Inorganic Coordinator

TRACE ORGANICS REVIEWED BY: Neli Popnikolova, Senior Chemist

DATE REPORTED: Apr 18, 2017

PAGES (INCLUDING COVER): 6

VERSION\*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

\*NOTES

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.



## Certificate of Analysis

AGAT WORK ORDER: 17T199091

PROJECT: 17KF002

5835 COOPERS AVENUE  
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<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY:

### O. Reg. 153(511) - Metals & Inorganics (Soil)

DATE RECEIVED: 2017-03-23

DATE REPORTED: 2017-04-18

Parameter	Unit	SAMPLE DESCRIPTION:		BH4-SS1	BH5-SS1	BH6-SS1
		SAMPLE TYPE:		Soil	Soil	Soil
		DATE SAMPLED:		2017-03-21	2017-03-21	2017-03-21
		G / S	RDL	8276142	8276150	8276151
Antimony	µg/g	40	0.8	<0.8	<0.8	<0.8
Arsenic	µg/g	18	1	3	5	6
Barium	µg/g	670	2	15	45	48
Beryllium	µg/g	8	0.5	<0.5	<0.5	<0.5
Boron	µg/g	120	5	6	6	<5
Boron (Hot Water Soluble)	µg/g	2	0.10	0.17	0.23	0.27
Cadmium	µg/g	1.9	0.5	<0.5	0.7	0.6
Chromium	µg/g	160	2	8	13	13
Cobalt	µg/g	80	0.5	1.9	4.4	4.9
Copper	µg/g	230	1	8	11	11
Lead	µg/g	120	1	40	62	53
Molybdenum	µg/g	40	0.5	0.6	0.9	0.5
Nickel	µg/g	270	1	5	10	10
Selenium	µg/g	5.5	0.4	<0.4	0.4	0.5
Silver	µg/g	40	0.2	<0.2	<0.2	<0.2
Thallium	µg/g	3.3	0.4	<0.4	<0.4	<0.4
Uranium	µg/g	33	0.5	0.5	0.5	0.5
Vanadium	µg/g	86	1	11	22	24
Zinc	µg/g	340	5	182	313	254
Chromium VI	µg/g	8	0.2	<0.2	<0.2	<0.2
Cyanide	µg/g	0.051	0.040	<0.040	<0.040	<0.040
Mercury	µg/g	3.9	0.10	<0.10	<0.10	<0.10
Electrical Conductivity	mS/cm	1.4	0.005	0.177	0.233	0.173
Sodium Adsorption Ratio	NA	12	NA	0.125	0.142	0.053
pH, 2:1 CaCl2 Extraction	pH Units		NA	6.74	6.90	7.07

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to ON T2 S ICC CT

8276142-8276151 EC & SAR were determined on the DI water extract obtained from the 2:1 leaching procedure (2 parts DI water:1 part soil). pH was determined on the 0.01M CaCl2 extract prepared at 2:1 ratio.

Certified By:

*Amanjot Bhela*



# AGAT Laboratories

## Certificate of Analysis

AGAT WORK ORDER: 17T199091

PROJECT: 17KF002

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<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY:

### O. Reg. 153(511) - OC Pesticides (Soil)

DATE RECEIVED: 2017-03-23

DATE REPORTED: 2017-04-18

		SAMPLE DESCRIPTION:		BH4-SS1	BH6-SS1
		SAMPLE TYPE:		Soil	Soil
		DATE SAMPLED:		2017-03-21	2017-03-21
Parameter	Unit	G / S	RDL	8276142	8276151
Hexachloroethane	µg/g	0.21	0.01	<0.01	<0.01
Gamma-Hexachlorocyclohexane	µg/g	0.056	0.005	<0.005	<0.005
Heptachlor	µg/g	0.19	0.005	<0.005	<0.005
Aldrin	µg/g	0.088	0.005	<0.005	<0.005
Heptachlor Epoxide	µg/g	0.05	0.005	<0.005	<0.005
Endosulfan	µg/g	0.3	0.005	<0.005	<0.005
Chlordane	µg/g	0.05	0.007	<0.007	<0.007
DDE	µg/g	0.52	0.007	<0.007	<0.007
DDD	µg/g	4.6	0.007	<0.007	<0.007
DDT	µg/g	1.4	0.007	<0.007	<0.007
Dieldrin	µg/g	0.088	0.005	<0.005	<0.005
Endrin	µg/g	0.04	0.005	<0.005	<0.005
Methoxychlor	µg/g	1.6	0.005	<0.005	<0.005
Hexachlorobenzene	µg/g	0.66	0.005	<0.005	<0.005
Hexachlorobutadiene	µg/g	0.031	0.01	<0.01	<0.01
Moisture Content	%		0.1	33.0	6.7
Surrogate	Unit	Acceptable Limits			
TCMX	%	50-140		70	66
Decachlorobiphenyl	%	60-130		72	88

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to ON T2 S ICC CT

8276142-8276151 Results are based on the dry weight of the soil.

Note: DDT applies to the total of op'DDT and pp'DDT, DDD applies to the total of op'DDD and pp'DDD and DDE applies to the total of op'DDE and pp'DDE. Endosulfan applies to the total of Endosulfan I and Endosulfan II.

Chlordane applies to the total of Alpha-Chlordane and Gamma-Chlordane.

Certified By:



## Quality Assurance

CLIENT NAME: PETO MACCALLUM LIMITED

PROJECT: 17KF002

SAMPLING SITE:

AGAT WORK ORDER: 17T199091

ATTENTION TO: Ken Hanes

SAMPLED BY:

### Soil Analysis

RPT Date: Apr 18, 2017			DUPLICATE			Method Blank	REFERENCE MATERIAL			METHOD BLANK SPIKE			MATRIX SPIKE		
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 153(511) - Metals & Inorganics (Soil)															
Antimony	8272855		3.6	3.6	NA	< 0.8	126%	70%	130%	105%	80%	120%	96%	70%	130%
Arsenic	8272855		9	7	25.0%	< 1	108%	70%	130%	105%	80%	120%	103%	70%	130%
Barium	8272855		76	75	1.3%	< 2	101%	70%	130%	98%	80%	120%	101%	70%	130%
Beryllium	8272855		<0.5	<0.5	NA	< 0.5	83%	70%	130%	105%	80%	120%	89%	70%	130%
Boron	8272855		6	6	NA	< 5	82%	70%	130%	107%	80%	120%	93%	70%	130%
Boron (Hot Water Soluble)	8272855		0.41	0.42	NA	< 0.10	112%	60%	140%	103%	70%	130%	99%	60%	140%
Cadmium	8272855		0.8	0.8	NA	< 0.5	110%	70%	130%	106%	80%	120%	105%	70%	130%
Chromium	8272855		18	18	0.0%	< 2	96%	70%	130%	114%	80%	120%	112%	70%	130%
Cobalt	8272855		5.5	5.5	0.0%	< 0.5	102%	70%	130%	110%	80%	120%	99%	70%	130%
Copper	8272855		63	62	1.6%	< 1	101%	70%	130%	117%	80%	120%	85%	70%	130%
Lead	8272855		190	197	3.6%	< 1	105%	70%	130%	101%	80%	120%	70%	70%	130%
Molybdenum	8272855		1.3	1.3	NA	< 0.5	107%	70%	130%	103%	80%	120%	105%	70%	130%
Nickel	8272855		24	25	4.1%	< 1	103%	70%	130%	112%	80%	120%	100%	70%	130%
Selenium	8272855		0.9	1.0	NA	< 0.4	128%	70%	130%	99%	80%	120%	106%	70%	130%
Silver	8272855		<0.2	<0.2	NA	< 0.2	98%	70%	130%	115%	80%	120%	110%	70%	130%
Thallium	8272855		<0.4	<0.4	NA	< 0.4	103%	70%	130%	104%	80%	120%	98%	70%	130%
Uranium	8272855		<0.5	<0.5	NA	< 0.5	98%	70%	130%	93%	80%	120%	95%	70%	130%
Vanadium	8272855		20	20	0.0%	< 1	99%	70%	130%	109%	80%	120%	109%	70%	130%
Zinc	8272855		205	199	3.0%	< 5	102%	70%	130%	117%	80%	120%	84%	70%	130%
Chromium VI	8277762		<0.2	<0.2	NA	< 0.2	93%	70%	130%	98%	80%	120%	100%	70%	130%
Cyanide	8278916		<0.040	<0.040	NA	< 0.040	102%	70%	130%	108%	80%	120%	94%	70%	130%
Mercury	8272855		0.15	0.17	NA	< 0.10	100%	70%	130%	88%	80%	120%	93%	70%	130%
Electrical Conductivity	8277893		0.376	0.369	1.9%	< 0.005	93%	90%	110%	NA			NA		
Sodium Adsorption Ratio	8276363		0.057	0.053	7.3%	NA	NA			NA			NA		
pH, 2:1 CaCl2 Extraction	8277854		7.37	7.42	0.7%	NA	101%	80%	120%	NA			NA		

Comments: NA signifies Not Applicable.

Duplicate Qualifier: As the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Certified By:

*Amanjot Bhela*





## Quality Assurance

CLIENT NAME: PETO MACCALLUM LIMITED

PROJECT: 17KF002

SAMPLING SITE:

AGAT WORK ORDER: 17T199091

ATTENTION TO: Ken Hanes

SAMPLED BY:

### Trace Organics Analysis

RPT Date: Apr 18, 2017

RPT Date: Apr 18, 2017			DUPLICATE			Method Blank	REFERENCE MATERIAL		METHOD BLANK SPIKE			MATRIX SPIKE			
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper

#### O. Reg. 153(511) - OC Pesticides (Soil)

Hexachloroethane	8267227		< 0.01	< 0.01	NA	< 0.01	82%	50%	140%	96%	50%	140%	64%	50%	140%
Gamma-Hexachlorocyclohexane	8267227		< 0.005	< 0.005	NA	< 0.005	92%	50%	140%	78%	50%	140%	66%	50%	140%
Heptachlor	8267227		< 0.005	< 0.005	NA	< 0.005	80%	50%	140%	90%	50%	140%	80%	50%	140%
Aldrin	8267227		< 0.005	< 0.005	NA	< 0.005	109%	50%	140%	94%	50%	140%	68%	50%	140%
Heptachlor Epoxide	8267227		< 0.005	< 0.005	NA	< 0.005	90%	50%	140%	96%	50%	140%	82%	50%	140%
Endosulfan	8267227		< 0.005	< 0.005	NA	< 0.005	89%	50%	140%	88%	50%	140%	69%	50%	140%
Chlordane	8267227		< 0.007	< 0.007	NA	< 0.007	87%	50%	140%	91%	50%	140%	78%	50%	140%
DDE	8267227		< 0.007	< 0.007	NA	< 0.007	88%	50%	140%	98%	50%	140%	78%	50%	140%
DDD	8267227		< 0.007	< 0.007	NA	< 0.007	94%	50%	140%	94%	50%	140%	84%	50%	140%
DDT	8267227		< 0.007	< 0.007	NA	< 0.007	88%	50%	140%	87%	50%	140%	78%	50%	140%
Dieldrin	8267227		< 0.005	< 0.005	NA	< 0.005	84%	50%	140%	90%	50%	140%	80%	50%	140%
Endrin	8267227		< 0.005	< 0.005	NA	< 0.005	84%	50%	140%	76%	50%	140%	82%	50%	140%
Methoxychlor	8267227		< 0.005	< 0.005	NA	< 0.005	76%	50%	140%	82%	50%	140%	96%	50%	140%
Hexachlorobenzene	8267227		< 0.005	< 0.005	NA	< 0.005	92%	50%	140%	100%	50%	140%	92%	50%	140%
Hexachlorobutadiene	8267227		< 0.01	< 0.01	NA	< 0.01	93%	50%	140%	100%	50%	140%	68%	50%	140%

Comments: When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

*N Popmukohof*

## Method Summary

CLIENT NAME: PETO MACCALLUM LIMITED

AGAT WORK ORDER: 17T199091

PROJECT: 17KF002

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY:

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Soil Analysis			
Antimony	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Arsenic	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Barium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Beryllium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron (Hot Water Soluble)	MET-93-6104	EPA SW 846 6010C; MSA, Part 3, Ch.21	ICP/OES
Cadmium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Cobalt	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Copper	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Lead	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Molybdenum	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Nickel	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Selenium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Silver	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Thallium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Uranium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Vanadium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Zinc	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium VI	INOR-93-6029	SM 3500 B; MSA Part 3, Ch. 25	SPECTROPHOTOMETER
Cyanide	INOR-93-6052	MOE CN-3015 & E 3009 A; SM 4500 CN	TECHNICON AUTO ANALYZER
Mercury	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Electrical Conductivity	INOR-93-6036	McKeague 4.12, SM 2510 B	EC METER
Sodium Adsorption Ratio	INOR-93-6007	McKeague 4.12 & 3.26 & EPA SW-846 6010B	ICP/OES
pH, 2:1 CaCl <sub>2</sub> Extraction	INOR-93-6031	MSA part 3 & SM 4500-H+ B	PH METER
Trace Organics Analysis			
Hexachloroethane	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Gamma-Hexachlorocyclohexane	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Heptachlor	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Aldrin	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Heptachlor Epoxide	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Endosulfan	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Chlordane	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
DDE	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
DDD	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
DDT	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Dieldrin	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Endrin	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Methoxychlor	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Hexachlorobenzene	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Hexachlorobutadiene	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
TCMX	ORG-91-5112	EPA SW-846 3541,3620 & 8081	GC/ECD
Decachlorobiphenyl	ORG-91-5113	EPA SW-846 3541,3620 & 8081	GC/ECD
Moisture Content		MOE E3139	BALANCE

CLIENT NAME: PETO MACCALLUM LIMITED  
16 FRANKLIN STREET SOUTH  
KITCHENER, ON N2C1R4  
(519) 893-7500

ATTENTION TO: Ken Hanes

PROJECT: 17KF002

AGAT WORK ORDER: 17W201248

SOIL ANALYSIS REVIEWED BY: Sofka Pehlyova, Senior Analyst

DATE REPORTED: Apr 10, 2017

PAGES (INCLUDING COVER): 5

VERSION\*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

\*NOTES

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.



# AGAT Laboratories

## Certificate of Analysis

AGAT WORK ORDER: 17W201248

PROJECT: 17KF002

5835 COOPERS AVENUE  
MISSISSAUGA, ONTARIO  
CANADA L4Z 1Y2  
TEL (905)712-5100  
FAX (905)712-5122  
<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY: H. Shinwary

### O. Reg. 153(511) - Metals & Inorganics (Soil)

DATE RECEIVED: 2017-03-30

DATE REPORTED: 2017-04-10

Parameter	Unit	SAMPLE DESCRIPTION:		BH5-SS4	BH6-SS3
		SAMPLE TYPE:		Soil	Soil
		DATE SAMPLED:		2017-03-21	2017-03-21
		G / S	RDL	8288805	8288806
Antimony	µg/g	40	0.8	<0.8	<0.8
Arsenic	µg/g	18	1	3	4
Barium	µg/g	670	2	9	13
Beryllium	µg/g	8	0.5	<0.5	<0.5
Boron	µg/g	120	5	<5	<5
Boron (Hot Water Soluble)	µg/g	2	0.10	<0.10	<0.10
Cadmium	µg/g	1.9	0.5	<0.5	0.6
Chromium	µg/g	160	2	5	8
Cobalt	µg/g	80	0.5	1.8	4.0
Copper	µg/g	230	1	8	15
Lead	µg/g	120	1	18	43
Molybdenum	µg/g	40	0.5	<0.5	0.8
Nickel	µg/g	270	1	4	8
Selenium	µg/g	5.5	0.4	<0.4	<0.4
Silver	µg/g	40	0.2	<0.2	<0.2
Thallium	µg/g	3.3	0.4	<0.4	<0.4
Uranium	µg/g	33	0.5	<0.5	<0.5
Vanadium	µg/g	86	1	11	19
Zinc	µg/g	340	5	180	370
Chromium VI	µg/g	8	0.2	<0.2	<0.2
Cyanide	µg/g	0.051	0.040	<0.040	<0.040
Mercury	µg/g	3.9	0.10	<0.10	<0.10
Electrical Conductivity	mS/cm	1.4	0.005	0.098	0.174
Sodium Adsorption Ratio	NA	12	NA	0.303	0.509
pH, 2:1 CaCl <sub>2</sub> Extraction	pH Units		NA	7.94	8.16

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to ON T2 S ICC CT

8288805-8288806 EC & SAR were determined on the DI water extract obtained from the 2:1 leaching procedure (2 parts DI water:1 part soil). pH was determined on the 0.01M CaCl<sub>2</sub> extract prepared at 2:1 ratio.

Certified By:

*Sofra Pehlyora*



**AGAT** Laboratories

## Guideline Violation

AGAT WORK ORDER: 17W201248

PROJECT: 17KF002

5835 COOPERS AVENUE  
MISSISSAUGA, ONTARIO  
CANADA L4Z 1Y2  
TEL (905)712-5100  
FAX (905)712-5122  
<http://www.agatlabs.com>

CLIENT NAME: PETO MACCALLUM LIMITED

ATTENTION TO: Ken Hanes

SAMPLEID	SAMPLE TITLE	GUIDELINE	ANALYSIS PACKAGE	PARAMETER	UNIT	GUIDEVALUE	RESULT
8288806	BH6-SS3	ON T2 S ICC CT	O. Reg. 153(511) - Metals & Inorganics (Soil)	Zinc	µg/g	340	370



## Quality Assurance

CLIENT NAME: PETO MACCALLUM LIMITED

PROJECT: 17KF002

SAMPLING SITE:

AGAT WORK ORDER: 17W201248

ATTENTION TO: Ken Hanes

SAMPLED BY: H. Shinwary

### Soil Analysis

RPT Date: Apr 10, 2017			DUPLICATE			Method Blank	REFERENCE MATERIAL			METHOD BLANK SPIKE			MATRIX SPIKE		
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 153(511) - Metals & Inorganics (Soil)															
Antimony	8287941		<0.8	<0.8	NA	< 0.8	116%	70%	130%	100%	80%	120%	92%	70%	130%
Arsenic	8287941		4	4	NA	< 1	107%	70%	130%	98%	80%	120%	104%	70%	130%
Barium	8287941		48	47	2.6%	< 2	98%	70%	130%	96%	80%	120%	101%	70%	130%
Beryllium	8287941		<0.5	<0.5	NA	< 0.5	78%	70%	130%	108%	80%	120%	89%	70%	130%
Boron	8287941		<5	<5	NA	< 5	89%	70%	130%	108%	80%	120%	91%	70%	130%
Boron (Hot Water Soluble)	8287941		0.34	0.36	NA	< 0.10	112%	60%	140%	100%	70%	130%	101%	60%	140%
Cadmium	8287941		<0.5	<0.5	NA	< 0.5	89%	70%	130%	100%	80%	120%	103%	70%	130%
Chromium	8287941		13	13	0.0%	< 2	95%	70%	130%	106%	80%	120%	120%	70%	130%
Cobalt	8287941		6.0	6.2	3.3%	< 0.5	102%	70%	130%	108%	80%	120%	108%	70%	130%
Copper	8287941		32	33	3.1%	< 1	94%	70%	130%	110%	80%	120%	115%	70%	130%
Lead	8287941		10	10	0.0%	< 1	101%	70%	130%	101%	80%	120%	99%	70%	130%
Molybdenum	8287941		<0.5	<0.5	NA	< 0.5	101%	70%	130%	103%	80%	120%	103%	70%	130%
Nickel	8287941		13	13	0.0%	< 1	105%	70%	130%	107%	80%	120%	108%	70%	130%
Selenium	8287941		<0.4	<0.4	NA	< 0.4	107%	70%	130%	103%	80%	120%	102%	70%	130%
Silver	8287941		<0.2	<0.2	NA	< 0.2	93%	70%	130%	106%	80%	120%	105%	70%	130%
Thallium	8287941		<0.4	<0.4	NA	< 0.4	86%	70%	130%	102%	80%	120%	103%	70%	130%
Uranium	8287941		<0.5	<0.5	NA	< 0.5	90%	70%	130%	92%	80%	120%	95%	70%	130%
Vanadium	8287941		22	22	0.0%	< 1	100%	70%	130%	106%	80%	120%	124%	70%	130%
Zinc	8287941		53	49	7.8%	< 5	103%	70%	130%	118%	80%	120%	116%	70%	130%
Chromium VI	8284952		<0.2	<0.2	NA	< 0.2	92%	70%	130%	96%	80%	120%	98%	70%	130%
Cyanide	8288805	8288805	<0.040	<0.040	NA	< 0.040	102%	70%	130%	103%	80%	120%	104%	70%	130%
Mercury	8287941		<0.10	<0.10	NA	< 0.10	102%	70%	130%	95%	80%	120%	102%	70%	130%
Electrical Conductivity	8291645		0.428	0.431	0.7%	< 0.005	94%	90%	110%	NA			NA		
Sodium Adsorption Ratio	8287941		0.751	0.761	1.3%	NA	NA			NA			NA		
pH, 2:1 CaCl2 Extraction	8285504		7.26	7.23	0.4%	NA	100%	80%	120%	NA			NA		

Comments: NA signifies Not Applicable.

Duplicate Qualifier: As the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Certified By:

*Sofia Pehlyora*

## Method Summary

CLIENT NAME: PETO MACCALLUM LIMITED

AGAT WORK ORDER: 17W201248

PROJECT: 17KF002

ATTENTION TO: Ken Hanes

SAMPLING SITE:

SAMPLED BY: H. Shinwary

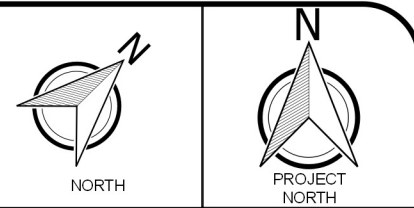
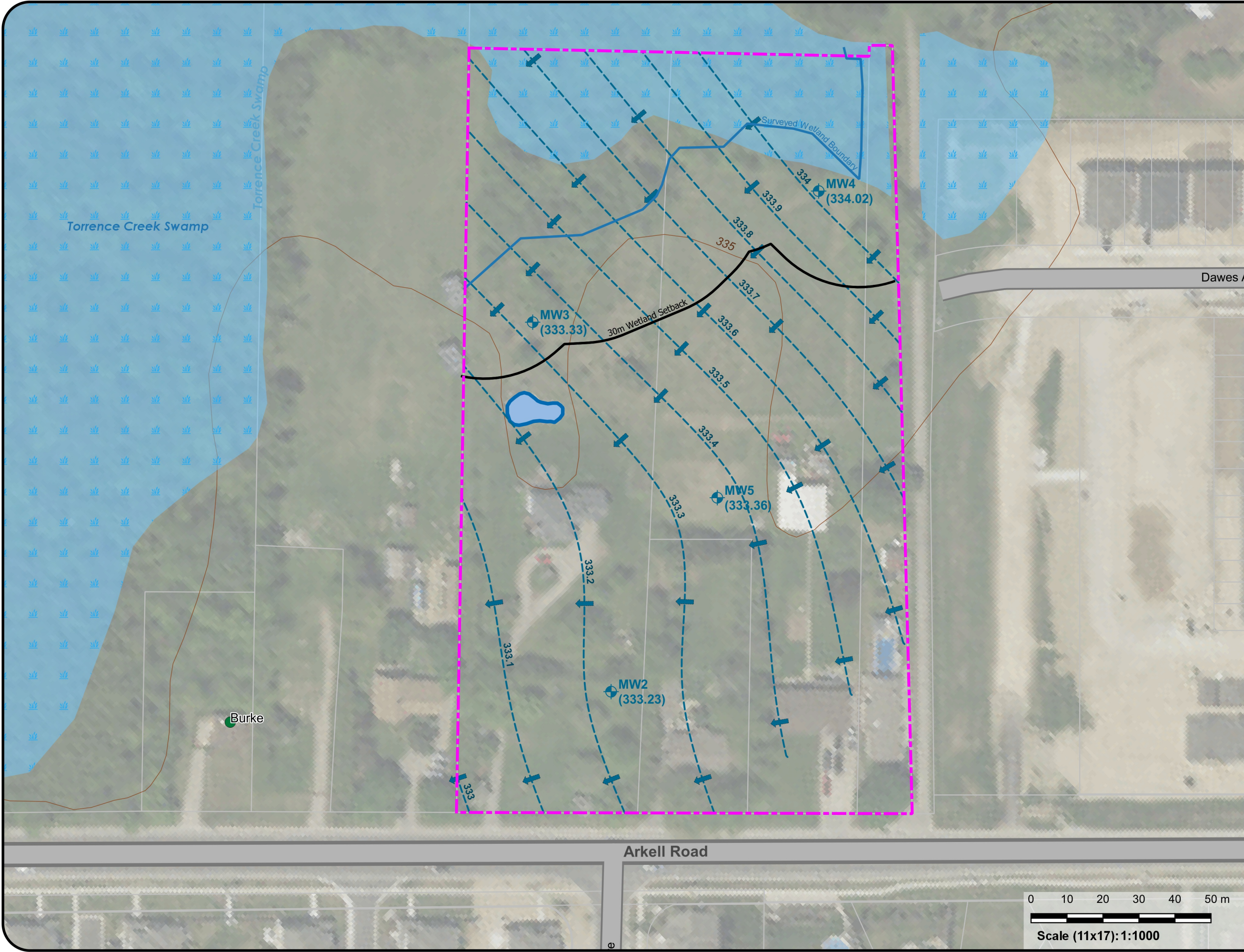
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Soil Analysis			
Antimony	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Arsenic	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Barium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Beryllium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron (Hot Water Soluble)	MET-93-6104	EPA SW 846 6010C; MSA, Part 3, Ch.21	ICP/OES
Cadmium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Cobalt	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Copper	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Lead	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Molybdenum	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Nickel	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Selenium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Silver	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Thallium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Uranium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Vanadium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Zinc	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium VI	INOR-93-6029	SM 3500 B; MSA Part 3, Ch. 25	SPECTROPHOTOMETER
Cyanide	INOR-93-6052	MOE CN-3015 & E 3009 A; SM 4500 CN	TECHNICON AUTO ANALYZER
Mercury	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Electrical Conductivity	INOR-93-6036	McKeague 4.12, SM 2510 B	EC METER
Sodium Adsorption Ratio	INOR-93-6007	McKeague 4.12 & 3.26 & EPA SW-846 6010B	ICP/OES
pH, 2:1 CaCl <sub>2</sub> Extraction	INOR-93-6031	MSA part 3 & SM 4500-H+ B	PH METER

## Appendix G

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### Seasonal High GW





- Legend**
- Subject Lands
  - Study Area (500m)
  - Municipal Well (GRCA)
  - Monitoring Well (Groundwater Elevation (mamsl))
  - Seasonal High Groundwater Elevation Contour (mamsl) & Flow Direction (May 6, 2017)

- Surface Water**
- Surface Water Body
  - Watercourse
  - Provincially Significant Wetland (GRCA)

- Ground Surface Elevation Contour (mamsl)**
- Major Contour (10m)
  - Minor Contour (5m)

Data Sources:

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South Central Ontario Orthophotography Project (2018). Source: Data provided by Ontario Ministry of Natural Resources and Forestry. ©

Project CRS: NAD83 / UTM zone 17N

  
**MTE**  
 Engineers, Scientists, Surveyors  
 Ph. (519) 743-6500

Client

Crescent Homes Ltd.

Project

Hydrogeological Assessment

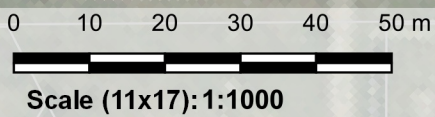
Site

190-216 Arkell Road, Guelph

Title

Seasonal High Groundwater Flow Interpretation (May 6, 2017)

Reviewed By	PAG	Project No	42063-200
Prepared By	KLW	Figure No	9B
Drawn By	KLW		
Date	November 2021		





LEGEND

SITE BOUNDARY

WETLAND BOUNDARY

15m WETLAND SETBACK

SURVEYED DRIPLINE

10m DRIPLINE SETBACK

EXISTING CONTOURS

EXISTING CURB

EXISTING DIRECTION OF DRAINAGE/SWALE

EXISTING BUILDING

EXISTING EMBANKMENT (SLOPE AS NOTED)

EXISTING SANITARY SEWER

EXISTING WATERMAIN

EXISTING STORM SEWER

EXISTING RETAINING WALL

EXISTING FENCE

EXISTING PIEZOMETER

EXISTING BOREHOLE/MONITORING WELL

EXISTING TEST PIT

**GEODETIC BM** ELEV. = 335.455m  
CITY OF GUELPH  
#255 BURKE WELL PUMP HOUSE

**SITE BENCHMARK** ELEV. = 335.455m  
SEE ABOVE

**NOTE TO CONTRACTOR :**  
DO NOT SCALE DRAWINGS.  
CONTRACTORS MUST CHECK AND VERIFY ALL DIMENSIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.  
ALL DRAWINGS REMAIN THE PROPERTY OF THE ENGINEER AND SHALL NOT BE REPRODUCED OR REUSED WITHOUT THE ENGINEER'S WRITTEN PERMISSION.  
THE OWNER/ARCHITECT/CONTRACTOR IS ADVISED THAT M.T.E. CONSULTANTS INC. CANNOT CERTIFY ANY COMPONENT OF THE SITE WORKS NOT INSPECTED DURING CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO NOTIFY M.T.E. CONSULTANTS INC. PRIOR TO COMMENCEMENT OF CONSTRUCTION TO ARRANGE FOR INSPECTION.

8.			
7.			
6.			
5.			
4.			
3.			
2.	REISSUED FOR DRAFT PLAN APPROVAL	DPH	NOV.XX/21
1.	ISSUED FOR DRAFT PLAN APPROVAL	INC	APR.3/20
No.	REVISION	BY	DATE

Engineers, Scientists, Surveyors

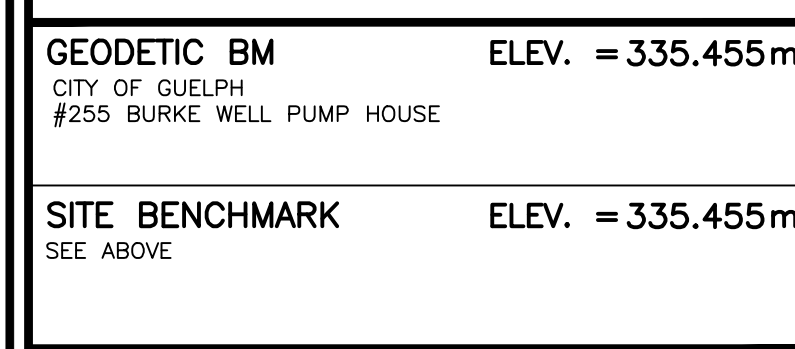
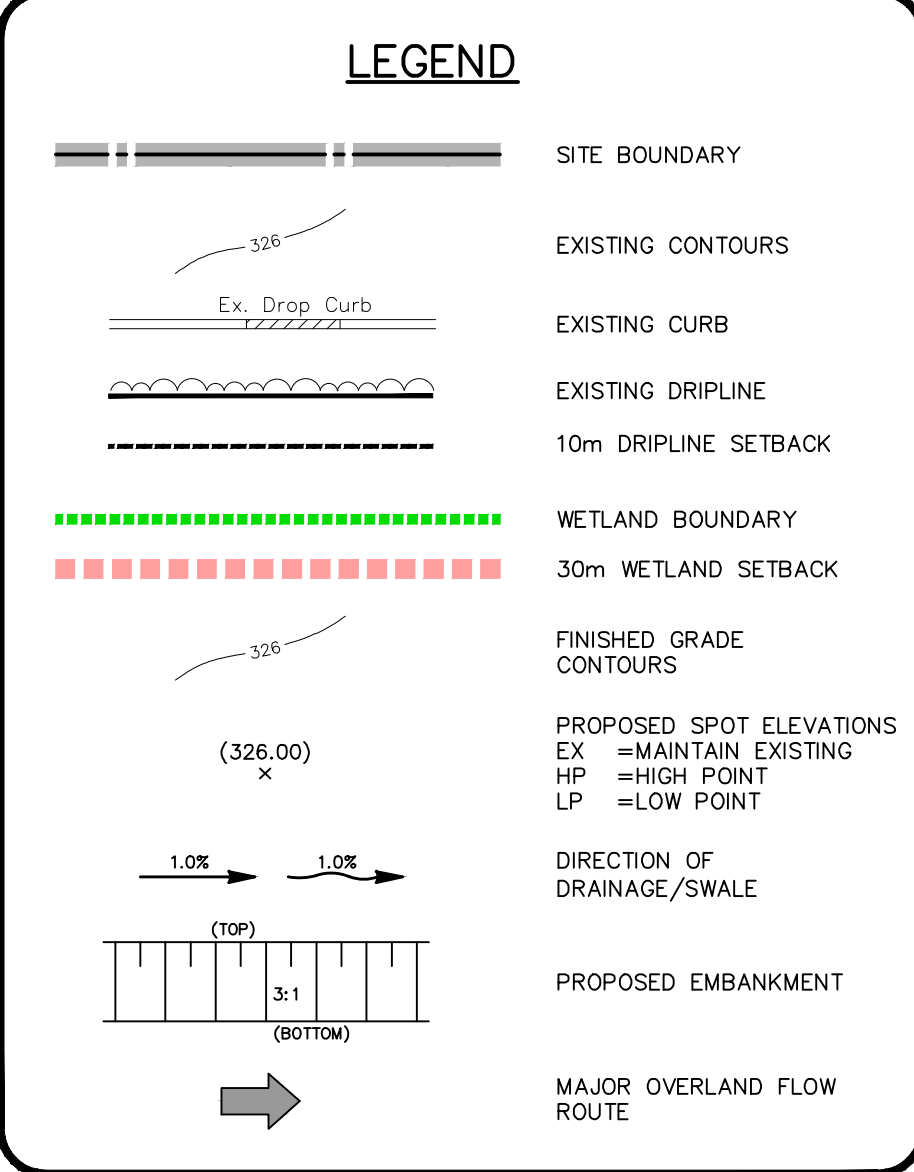
519-743-6500

**OWNER**  
CRESCENT HAVEN HOMES INC.  
180 FROBISHER DRIVE UNIT 3 WATERLOO

**PROJECT**  
ARKELL ROAD PROPERTIES  
216 ARKELL ROAD GUELPH

<b>EXISTING CONDITIONS PLAN</b>	
Project Manager D.HICKS	Project No. 42063-104
Design By CJC/AJC	Checked By VAL
Drawn By SXP/KAT	Checked By AJC
Surveyed By MTE	Drawing No. EC1.1
Date Dec.04/19	
Scale 1:400	Sheet 1 of 1





**NOTE TO CONTRACTOR :**

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ALL DRAWINGS REMAIN THE PROPERTY OF THE ENGINEER AND SHALL NOT BE REPRODUCED OR REUSED WITHOUT THE ENGINEER'S WRITTEN PERMISSION.

THE OWNER/ARCHITECT/CONTRACTOR IS ADVISED THAT THE CONTRACTANTS WILL BE REQUIRED TO OBTAIN ALL COMPONENTS OF THE SITE WORKS NOT INSPECTED DURING CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO NOTIFY THE CONSULTANT PRIOR TO COMMENCEMENT OF CONSTRUCTION TO ARRANGE FOR INSPECTION.

8.			
7.			
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2.	REISSUED FOR DRAFT PLAN APPROVAL	DFH	INV.XX/21
1.	ISSUED FOR DRAFT PLAN APPROVAL	IXC	JAN.10/20
No.	REVISION	BY	DATE



519-743-6500

OWNER	CRESCENT HAVEN HOMES INC.		
180 FROBISHER DRIVE UNIT 3			WATERLOO
PROJECT	ARKELL ROAD PROPERTIES		
216 ARKELL ROAD			GUELPH

AREA  
GRADING  
PLAN

Project Manager	D.HICKS	Project No.	<b>42063-104</b>
Design By	AJC/BDS	Checked By	VAL
Drawn By	SXP/KAT	Checked By	AJC
Surveyed By	MTE	Drawing No.	<b>AG1.1</b>
Date	Mar.30/20		
Scale	1:400	Sheet 1 of 1	